

MONMOUTH COUNTY, NEW JERSEY (ALL JURISDICTIONS)

COMMUNITY NAME ABERDEEN, TOWNSHIP OF ALLENHURST, BOROUGH OF ALLENTOWN, BOROUGH OF ASBURY PARK, CITY OF ATLANTIC HIGHLANDS, BOROUGH OF AVON-BY-THE-SEA, BOROUGH OF BELMAR, BOROUGH OF BRADLEY BEACH, BOROUGH OF BRIELLE, BOROUGH OF COLTS NECK, TOWNSHIP OF DEAL, BOROUGH OF EATONTOWN, BOROUGH OF ENGLISHTOWN, BOROUGH OF FAIR HAVEN, BOROUGH OF FARMINGDALE, BOROUGH OF FREEHOLD, BOROUGH OF FREEHOLD, TOWNSHIP OF HAZLET, TOWNSHIP OF HIGHLANDS, BOROUGH OF HOLMDEL, TOWNSHIP OF HOWELL, TOWNSHIP OF INTERLAKEN, BOROUGH OF KEANSBURG, BOROUGH OF KEYPORT, BOROUGH OF LAKE COMO, BOROUGH OF LITTLE SILVER, BOROUGH OF LOCH ARBOUR, VILLAGE OF LONG BRANCH, CITY OF MANALAPAN, TOWNSHIP OF MANASQUAN, BOROUGH OF MARLBORO, TOWNSHIP OF MATAWAN, BOROUGH OF MIDDLETOWN, TOWNSHIP OF

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COMMUNITY NUMBER

Monmouth County



COMMUNITY NAME MILLSTONE, TOWNSHIP OF	COMMUNITY NUMBER 340314
MONMOUTH BEACH, BOROUGH OF	340315
NEPTUNE CITY, BOROUGH OF	340316
NEPTUNE, TOWNSHIP OF	340317
OCEAN, TOWNSHIP OF	340319
OCEANPORT, BOROUGH OF	340320
RED BANK, BOROUGH OF	340321
ROOSEVELT, BOROUGH OF	340322
RUMSON, BOROUGH OF	345316
SEA BRIGHT, BOROUGH OF	345317
SEA GIRT, BOROUGH OF	340325
SHREWSBURY, BOROUGH OF	340326
SHREWSBURY, TOWNSHIP OF	340002
SPRING LAKE HEIGHTS, BOROUGH OF	340330
SPRING LAKE, BOROUGH OF	340329
TINTON FALLS, BOROUGH OF	340318
UNION BEACH, BOROUGH OF	340331
UPPER FREEHOLD, TOWNSHIP OF	340332
WALL, TOWNSHIP OF	340333
WEST LONG BRANCH, BOROUGH OF	340334

EFFECTIVE: SEPTEMBER 25, 2009



Federal Emergency Management Agency

FLOOD INSURANCE STUDY NUMBER 34025CV001A

NOTICE TO FLOOD INSURANCE STUDY USERS

Communities participating in the National Flood Insurance Program have established repositories of flood hazard data for floodplain management and flood insurance purposes. This Flood Insurance Study (FIS) may not contain all data available within the repository. It is advisable to contact the community repository for any additional data.

Part or all of this FIS may be revised and republished at any time. In addition, part of this FIS may be revised by the Letter of Map Revision process, which does not involve republication or redistribution of the FIS. It is, therefore, the responsibility of the user to consult with community officials and to check the community repository to obtain the most current FIS components.

Initial Countywide FIS Effective Date: September 25, 2009

Revised Countywide FIS Date:

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FLOOD INSURANCE STUDY MONMOUTH COUNTY, NEW JERSEY (ALL JURISDICTIONS)

1.0 INTRODUCTION

1.1 Purpose of Study

This countywide Flood Insurance Study (FIS) investigates the existence and severity of flood hazards in, or revises and updates previous FISs/Flood Insurance Rate Maps (FIRMs) for the geographic area of Monmouth County, New Jersey, including: the Boroughs of Allenhurst, Allentown, Atlantic Highlands, Avon-by-the-Sea, Belmar, Bradley Beach, Brielle, Deal, Eatontown, Englishtown, Fair Haven, Farmingdale, Freehold, Highlands, Interlaken, Keansburg, Keyport, Lake Como, Little Silver, Manasquan, Matawan, Monmouth Beach, Neptune City, Oceanport, Red Bank, Roosevelt, Rumson, Sea Bright, Sea Girt, Shrewsbury, Spring Lake, Spring Lake Heights, Tinton Falls, Union Beach and West Long Branch; the Cities of Asbury Park and Long Branch; the Townships of Aberdeen, Colts Neck, Freehold, Hazlet, Holmdel, Howell, Manalapan, Marlboro, Middletown, Millstone, Neptune, Ocean, Shrewsbury, Upper Freehold and Wall; and the Village of Loch Arbour (hereinafter referred to collectively as Monmouth County). The Borough of Freehold and the Township of Shrewsbury are non-floodprone.

This FIS aids in the administration of the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973. This FIS has developed flood risk data for various areas of the county that will be used to establish actuarial flood insurance rates. This information will also be used by Monmouth County to update existing floodplain regulations as part of the Regular Phase of the National Flood Insurance Program (NFIP), and will also be used by local and regional planners to further promote sound land use and floodplain development. Minimum floodplain management requirements for participation in the NFIP are set forth in the Code of Federal Regulations at 44 CFR, 60.3.

In some States or communities, floodplain management criteria or regulations may exist that are more restrictive or comprehensive than the minimum Federal requirements. In such cases, the more restrictive criteria take precedence and the State (or other jurisdictional agency) will be able to explain them.

1.2 Authority and Acknowledgments

The sources of authority for this FIS are the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973.

This FIS was prepared to include all communities within Monmouth County in a countywide format. Information on the authority and acknowledgments for each jurisdiction included in this countywide FIS, as compiled from their previously printed FIS reports, is shown below.

	Aberdeen, Township of:	the hydrologic and hydraulic analyses in the FIS report dated September 18, 1984, were prepared by Richard Browne Associates, for the Federal Emergency Management Agency (FEMA), under Contract No. H-6809. This work was completed in November 1982. The wave height analysis was performed by Dewberry & Davis for FEMA, under Contract No. EMW-C-0543. This work was completed in July 1983.
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Allenhurst, Borough of: the hydrologic and hydraulic analyses in the FIS report dated September 1978 were performed by Tetra Tech, Inc., for the Federal Insurance Administration (FIA) under Contract No. H-3830. This work was completed in September 1977

Allentown, Borough of: the hydrologic and hydraulic analyses in the FIS report dated March 16, 1981, were conducted by Gannett Fleming Corddry and Carpenter, Inc., under subcontract to the State of New Jersey, Department of Environmental Protection (NJDEP), Division of Water Resources, for the FIA under Contract No. H-4623. This work was completed in March 1980.

Asbury Park, City of: the hydrologic and hydraulic analyses in the FIS report dated March 15, 1983, represent a revision of the original analyses by Tetra Tech, Inc., for FEMA under Contract No. H-3830. The updated analyses were also prepared by Tetra Tech, Inc., under agreement with FEMA. This work was completed in August 1981.

Atlantic Highlands, Borough of: the wave height analysis in the FIS report dated January 5, 1984, was prepared by Dewberry & Davis for FEMA, under Contract No. EMW-C-0543. This work was completed in May 1983.

> the hydrologic and hydraulic analyses in the FIS report dated January 5, 1983, were prepared by Tetra Tech, Inc., for FEMA under Contract No. H-3830. The updated version was prepared by Tetra Tech, Inc. as well, for FEMA under Contract No. H-3830. This work was completed in August 1981.

Belmar, Borough of: the wave height analysis in the FIS report dated September 1, 1983, was prepared by Dewberry &

Avon-by-the-Sea, Borough of:

	Davis for FEMA, under Contract No. EMW-C-0543. This work was completed in December 1982.
Bradley Beach, Borough of:	the hydrologic and hydraulic analyses in the FIS report dated December 15, 1982, represent a revision of the original analyses performed by Tetra Tech, Inc., for FEMA under Contract No. H-3830. The updated version was prepared by Tetra Tech, Inc. for FEMA under Contract No. H-3830. This work was completed in August 1981.
Brielle, Borough of:	the hydrologic and hydraulic analyses in the FIS report dated March 30, 1983, represent a revision of the original analyses performed by Tetra Tech, Inc., for FEMA under Contract No. H-3830. The updated version was also prepared by Tetra Tech, Inc., under agreement with FEMA. This work was completed in August 1981.
Colts Neck, Township of:	the hydrologic and hydraulic analyses in the FIS report dated October 15, 1981, were conducted by Tippetts-Abbett-McCarthy-Stratton under subcontract to NJDEP, Division of Water Resources for FEMA, under Inter-Agency Agreement M-3959. This work was completed in May 1978.
Deal, Borough of:	the hydrologic and hydraulic analyses in the revised FIS report dated August 6, 2002, were prepared by Leonard Jackson Associates, for FEMA, under Contract No. EMN-96-CO-0026. This work was completed in December, 2000. The wave height analysis in the same FIS report was prepared by Dewberry & Davis for FEMA under Contract No. EMW-C-0543. This work was completed in December 1982.
Eatontown, Borough of:	the hydrologic and hydraulic analyses in the FIS report dated March 16, 1981, were prepared by Tippetts-Abbett-McCarthy-Stratton under subcontract to NJDEP, Division of Water Resources for the FIA under Contract No. H-3959. This work was completed in January 1978.
Englishtown, Borough of:	the hydrologic and hydraulic analyses in the FIS report dated September 16, 1980, represent a revision of the original analysis. The analyses for this study were conducted by Justin and Courtney, Inc., under subcontract NJDEP for the FIA under Contract No.

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H-3959. This work was completed in December 1979.

- Fair Haven, Borough of: the hydrologic and hydraulic analyses for the FIS report dated April 1979 were conducted by Tippetts-Abbett-McCarthy-Stratton under subcontract to NJDEP, Division of Water Resources, Bureau of Flood Plain Management, for the FIA under Contract No. H-3959. This work was completed in November 1977.
- Freehold, Township of: the hydrologic and hydraulic analyses for the FIS report dated October 4, 1982, were performed by T & M Associates under subcontract to NJDEP for FEMA, under Contract No. H-4759. This work was completed in August 1980.
- Hazlet, Township of: the hydrologic and hydraulic analyses in the FIS report dated June 1, 1982, represent a revision of the original analyses completed in September 1977 by Tetra Tech, Inc., for FEMA under Contract No. H-3830. This updated version was prepared by Dewberry & Davis under agreement with FEMA, and was completed in October 1981.
- Highlands, Borough of: the hydrologic and hydraulic analyses in the revised FIS report dated December 22, 1998, were prepared by the U.S. Army Corps of Engineers (USACE), Philadelphia District, for FEMA under Inter-Agency Agreement No. EMW-94-E-4432, Project Order Nos. 1 and 1A. This work was completed in April 1996.
- Holmdel, Township of: the hydrologic and hydraulic analyses in the FIS report dated September 1, 1981, were prepared by Tippetts-Abbett-McCarthy-Stratton for FEMA under Contract No. H-3959. This work was completed in May 1978.
- Howell, Township of: the hydrologic and hydraulic analyses in the FIS report dated July 6, 1982, were prepared by T & M Associates under subcontract to NJDEP for FEMA, under Contract No.. A14516. This work was completed in August 1980.
- Keansburg, Borough of: the hydrologic and hydraulic analyses in the FIS report dated November 16, 1982, were prepared by Tetra Tech, Inc., for FEMA under Contract No. H-3830. This work was completed in August 1981.

Keyport, Borough of:	the hydrologic and hydraulic analyses in the FIS report dated October 18, 1982, represents a revision of the original analyses by Tetra Tech, Inc., for FEMA under Contract No. H-3830. The updated version was also prepared by Tetra Tech, Inc., under agreement with FEMA. This work was completed in August 1981.
Little Silver, Borough of:	the wave height analysis in the FIS report dated June 15, 1982, was prepared by Dewberry & Davis for FEMA.
Loch Arbour, Village of:	the hydrologic and hydraulic analyses in the FIS report dated March 15, 1983, represent a revision of the original analyses by Tetra Tech, Inc., for FEMA under Contract No. H-3830. The updated version was also prepared by Tetra Tech, Inc., under agreement with FEMA. This work was completed in August 1981.
Long Branch, City of:	the wave height analysis in the FIS report dated July 5, 1983, was prepared by Dewberry & Davis for FEMA, under Contract No. EMW-C-0543. This work was completed in December 1982.
Manalapan, Township of:	the hydrologic and hydraulic analyses in the FIS report dated September 15, 1977 were performed by Anderson-Nichols & Co., Inc., for the FIA under Contract No. H-3715. The work was completed in May 1976.
Manasquan, Borough of:	the wave height analysis in the FIS report dated June 15, 1983, was prepared by Dewberry & Davis for FEMA under Contract No. EMW-C-0543. This work was completed in December 1982.
Marlboro, Township of:	the hydrologic and hydraulic analyses in the FIS report revised April 1982 was performed by Anderson-Nichols & Co., Inc., for the FIA under Contract No. H-3715. This work was completed in May 1976.
Matawan, Borough of:	the hydrologic and hydraulic analyses in the FIS report dated March 30, 1981, was conducted by T & M Associates under subcontract to NJDEP for the FIA, under Contract No A14516. This work was completed in November 1979.

Middletown, Township of:	the hydrologic and hydraulic analyses in the FIS report dated August 15, 1983, represent a revision of the original analyses performed by Tetra Tech, Inc., for FEMA under Contract No. H-3830. The updated version was prepared by Dewberry & Davis under agreement with FEMA. The wave height analysis was prepared by Tetra Tech, Inc., for FEMA under Contract No. H-3830. This work was completed in August 1981.
Millstone, Township of:	the hydrologic and hydraulic analyses in the FIS report dated July 20, 1981, was conducted by Anderson-Nichols & Co., Inc., under subcontract to NJDEP for FEMA under Contract No. H-4546. This work was completed in February 1979.
Monmouth Beach, Borough of:	the wave height analysis in the FIS report dated October 16, 1984, was prepared by Dewberry & Davis for FEMA under Contract No. EMW-C-0543. This work was completed in January 1982.
Neptune, Township of:	the wave height analysis in the FIS report dated September 1, 1983, was prepared by Dewberry & Davis for FEMA under Contract No. EMW-C-0543. This work was completed in February 1983.
Ocean, Township of:	the hydrologic and hydraulic analyses in the FIS report revised July 2, 2003, represent a revision of the original analyses performed by Edward Schnitzer, Consulting Engineers for USACE for the FIA under Inter-Agency Agreement Nos. IAA-H-2-73 and IAA-H-19-74, Project Order Nos. 14 and 15, respectively. That work was completed in August 1975. The revised analyses were prepared by Leonard Jackson Associates for FEMA under Contract No. EMN-96-CO-0026. This work was completed in December 2000.
Oceanport, Borough of:	the hydrologic and hydraulic analyses in the FIS report dated August 1976 was conducted by Tippetts-Abbett-McCarthy-Stratton for the FIA under Contract No. H-3733.
Red Bank, Borough of:	the hydrologic and hydraulic analyses in the FIS report dated November 19, 1980, were prepared by Tippetts-Abbett-McCarthy-Stratton under subcontract to NJDEP, Division of Water Resources, Bureau of Flood Plain Management

	under Contract No. H-3959. This work was completed in November 1977.
Rumson, Borough of	the wave height analysis in FIS report dated June 15, 1982, was prepared by Dewberry & Davis for FEMA.
Sea Bright, Borough of:	the wave height analysis in the FIS report dated May 16, 1983, was prepared by Dewberry & Davis for FEMA under Contract No. EMW-C-0543. This work was completed in November 1982.
Sea Girt, Borough of:	the wave height analysis in the FIS report dated July 5, 1983, was prepared by Dewberry & Davis for FEMA under Contract No. EMW-C-0543. This work was completed in December 1982.
Shrewsbury, Borough of:	the hydrologic and hydraulic analyses in the FIS report dated February 1979 was performed by NJDEP for the FIA under Contract No. H-3959. This work was completed in March 1978.
Spring Lake, Borough of:	the wave height analysis in the FIS report dated September 1, 1983, was prepared by Dewberry & Davis for FEMA under Contract No. EMW-C-0543. This work was completed in December 1982.
Spring Lake Heights, Borough of:	the hydrologic and hydraulic analyses in the FIS report dated June 15, 1981, was prepared by T & M Associates under subcontract to NJDEP for FEMA under Contract No. H-4546. This work was completed in November 1979.
Tinton Falls, Borough of:	the hydrologic and hydraulic analyses in the FIS report dated October 15, 1981, was conducted by Tippetts-Abbett-McCarthy-Stratton under subcontract to NJDEP, Division of Water Resources under Inter-Agency Agreement No. H- 3959. The hydrologic and hydraulic analyses for the Swimming River were conducted By Dewberry, Nealon & Davis as the Technical Evaluation Contractor (TEC). This work was completed in February 1978.
Union Beach, Borough of:	the hydrologic and hydraulic analyses in the FIS report dated September 2, 1982, represent a revision of the original analyses performed by Tetra Tech, Inc., for FEMA under Contract No. H-3830. The

	updated version was prepared by Tetra Tech, Inc., under agreement with FEMA. This work was completed in August 1981.
Wall, Township of:	the hydrologic and hydraulic analyses in the FIS report dated August 1976 were conducted by Tippetts-Abbett-McCarthy-Stratton for the FIA under Contract No. H-3733.
West Long Branch, Borough of:	the hydrologic and hydraulic analyses in the FIS report dated July 16, 1980, were conducted by Tippetts-Abbett-McCarthy-Stratton under subcontract to NJDEP, Division of Water Resources under Contract No. H-3959. This work was completed in January 1978.

The authority and acknowledgements for the Boroughs of Farmingdale, Interlaken, Lake Como, Neptune City, Roosevelt and the Township of Upper Freehold are not available because no FIS reports were published for those communities

For this countywide FIS, revised hydrologic and hydraulic analyses for Manalapan Brook from the confluence with South River at the county line (Township of Manalapan and Middlesex County) to a location approximately 10 miles upstream at Moonlight Court in the Township of Millstone, were prepared for FEMA by Medina Consultants, P.C. under Contract No. EMN-2003-CO-0005. This work was completed in September 2007.

The remaining flooding sources studied in detail have been redelineated using updated topographic data provided to FEMA by Monmouth County.

Base map information shown on this FIRM was provided in digital format by the State of New Jersey Office of Information Technology. This information was derived from color infrared (CIR) orthophotos produced at a scale of 1:2400 (1"=200') with a 1 foot pixel resolution from photography captured during February-April, 2002. The projection used for the production of this FIRM is New Jersey State Plane, FIPSZONE 2900. The horizontal datum was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection, or State Plane zones used in the production of FIRMs for adjacent counties may result in slight positional differences in map features at the county boundaries. These differences do not affect the accuracy of information shown on the FIRM.

1.3 Coordination

Consultation Coordination Officer's (CCO) meetings may be held for each jurisdiction in this countywide FIS. An initial CCO meeting is held typically with representatives of FEMA, the community, and the study contractor to explain the nature and purpose of a FIS and to identify the streams to be studied by detailed

methods. A final CCO meeting is held typically with representatives of FEMA, the community, and the study contractor to review the results of the study.

The dates of the initial and final CCO meetings held for Monmouth County and the incorporated communities within its boundaries are shown in Table 1, "Initial and Final CCO Meetings."

TABLE 1 - INITIAL AND FINAL CCO MEETINGS

Community	Initial CCO Date	Final CCO Date
Aberdeen, Township of	June 15, 1979	February 29, 1984
Allenhurst, Borough of	*	January 23, 1978
Allentown, Borough of	February, 1978	September 29, 1980
Asbury Park, City of	*	October 28, 1982
Atlantic Highlands, Borough of	*	August 2, 1983
Avon-by-the-Sea, Borough of	*	January 27, 1982
Belmar, Borough of	*	January 21, 1983
Bradley Beach, Borough of	*	February 8, 1982
Brielle, Borough of	*	September 22, 1982
Colts Neck, Township of	May 18, 1976	December 10, 1980
Deal, Borough of	*	January 4, 1983
	*	July 20, 2001
Eatontown, Borough of	May 18, 1976	April 29, 1980
Englishtown, Borough of	March 15, 1976	April 29, 1980
Fair Haven, Borough of	May 18, 1976	November 29, 1978
Freehold, Township of	June 6, 1977	May 20, 1982
Hazlet, Township of	August 26, 1976	January 7, 1982
Highlands, Borough of	August 1, 1996	April 28, 1997
Holmdel, Township of	May 18, 1976	February 26, 1981
Howell, Township of	June 3, 1977	February 2, 1982
Keansburg, Borough of	*	June 30, 1982
Keyport, Borough of	*	April 13, 1982
Little Silver, Borough of	*	October 26, 1981
Loch Arbour, Village of	*	October 28, 1982
Long Branch, City of	*	February 2, 1983
Manalapan, Township of	December 16, 1974	July 20, 1976
Manasquan, Borough of	*	January 21, 1983
Marlboro, Township of	December 17, 1974	September 15, 1976
Matawan, Borough of	*	September 9, 1980
Middletown, Township of	August 26, 1976	April 30, 1982
Millstone, Township of	June 1977	November 25, 1980
Monmouth Beach, Borough of	*	October 26, 1981
Neptune, Township of	*	April 13, 1983
Ocean, Township of	October 2, 1973	January 15, 1976
-	*	October 29, 2001
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TABLE 1 - INITIAL AND FINAL CCO MEETINGS - continued

Community	Initial CCO Date	Final CCO Date
Oceanport, Borough of	*	October 29, 1975
Red Bank, Borough of	May 18, 1976	April 4, 1980
Rumson, Borough of	*	October 27, 1981
Sea Bright, Borough of	*	January 10, 1983
Sea Girt, Borough of	*	January 21, 1983
Shrewsbury, Borough of	May 18, 1976	August 14, 1978
Spring Lake, Borough of	*	January 24, 1983
Spring Lake Heights,	July 14, 1978	December 2, 1980
Borough of	*	*
Tinton Falls, Borough of	May 18, 1976	January 22, 1981
Union Beach, Borough of	*	April 22, 1982
Wall, Township of	May 7, 1975	March 4, 1976
West Long Branch, Borough of	May 18, 1976	January 23, 1980

*Data not available

The initial scoping meetings were held on October 18, 20, and 25, 2005.

The final CCO meeting was held on March 17-19, 2008. Representatives of FEMA, the County of Monmouth, New Jersey Department of Environmental Protection, Dewberry & Davis, and various communities in Monmouth County were present.

2.0 AREA STUDIED

2.1 Scope of Study

This FIS covers the geographic area of Monmouth County, New Jersey.

All or portions of the flooding sources listed in Table 2, "Flooding Sources Studied by Detailed Methods," were studied by detailed methods. Limits of detailed study are indicated on the Flood Profiles (Exhibit 1) and on the FIRM (Exhibit 2).

TABLE 2 - FLOODING SOURCES STUDIED BY DETAILED METHODS

Applegates Creek Ardena Brook Atlantic Ocean Big Brook (Upstream Reach) Bannen Meadow Brook Barclay Brook Barren Neck Creek Betty Brook Big Brook (Downstream Reach) Big Brook Tributary H Burkes Creek Claypit Creek Comptons Creek Cranberry Brook Deal Lake Deal Tributary 1

TABLE 2 - FLOODING SOURCES STUDIED BY DETAILED METHODS - continued

Deal Tributary 2 Deal Tributary 3 Deal Tributary 3A Deal Tributary 4 Deal Tributary 4A **Debois Creek** Debois Creek Tributary Deep Run Deep Run Tributary A Deep Run Tributary B Deep Run Tributary C Doctors Creek East Creek Flat Creek Fletcher Lake Gander Brook Gravelly Brook Gravelly Run Groundhog Brook Hannabrand Brook Haystack Brook Heroys Pond Creek Hockhockson Brook Hog Swamp Brook Hollow Brook Indian Run Judas Creek (Downstream Reach) Judas Creek (Upstream Reach) Jumping Brook 1 Jumping Brook 2 Little Silver Creek Little Silver Creek Tributary 1 Little Silver Creek Tributary 2 Little Silver Creek Tributary 2A Little Silver Creek Tributary 2B Little Silver Creek Tributary A Long Brook Mac's Brook Mahoras Brook Manalapan Brook Manalapan Brook Tributary A Manalapan Brook Tributary B Manasquan River Manasquan River Tributary A Marl Brook Matawan Creek Matchaponix Brook

McClees Creek McGellairds Brook Metedeconk River North Branch Milford Brook Millstone River Mine Brook Miry Bog Brook Mohingson Brook Monascunk Creek Musquash Brook Navesink River Nut Swamp Brook Parkers Creek Parkers Creek North Branch Perrineville Lake Pine Brook 1 Pine Brook 2 Pine Brook Tributary C Poly Pond Brook Polypod Brook Poplar Brook Poplar Brook Tributary 1 Poplar Brook Tributary 2 Poplar Brook Tributary 3 Poricy Brook Ramanessin Brook Raritan Bay Roberts Swamp Brook (Downstream Reach) Roberts Swamp Brook (Upstream Reach) Rocky Brook (Downstream Reach) Rocky Brook (Upstream Reach) Sandy Hook Bay Shark River Shark River Tributary D Shark River Tributary E Shrewsbury River South Shrewsbury River Still House Brook Swimming River Sylvan Lake **Tepehemus Brook** Tepehemus Brook, South Branch Toms River Town Brook Town Neck Creek Turtle Mill Brook

TABLE 2 - FLOODING SOURCES STUDIED BY DETAILED METHODS - continued

Waackaack Creek Wampum Brook Watson Creek Weamaconk Creek Tributary Wells Brook Wemrock Brook Whale Creek Whale Pond Brook Whale Pond Brook Tributary 1 Whale Pond Brook Tributary 2 Willow Brook Willow Brook Tributary F Willow Brook Tributary G Willow Brook East Branch Wreck Pond Brook Wreck Pond Brook, East Branch Yellow Brook Yellow Brook 2 Yellow Brook 2 Tributary Yellow Brook Tributary K Yellow Brook Tributary L

Table 3, "Stream Name Changes," lists streams that have names in this county wide FIS other than those used in previously printed FISs for the communities in which they are located.

TABLE 3 – STREAM NAME CHANGES

Community

<u>Old Name</u>

New Name

East Branch Willow Brook Willow Brook East Branch Township of Holmdel Township of Wall East Branch Wreck Pond Brook Wreck Pond Brook East Branch Township of Aberdeen Gravelly Run Gravelly Brook Gravelly Run Gravelly Brook Borough of Matawan Judas Creek (Upstream Reach) Township of Wall Judas Creek Township of Middletown Jumping Brook Jumping Brook 1 Township of Neptune Jumping Brook Jumping Brook 2 Jumping Brook Jumping Brook 2 **Borough of Tinton Falls** North Branch Metedeconk River Metedeconk River North Branch Township of Howell Borough of Shrewsbury Parkers Creek North Branch North Branch Parkers Creek Township of Colts Neck Pine Brook Pine Brook 1 Township of Manalapan Pine Brook Pine Brook 2 Township of Marlboro Pine Brook Pine Brook 2 Borough of Tinton Falls Pine Brook Pine Brook 1 Borough of Spring Lake Heights Polly Pod Brook **Poly Pond Brook** Township of Wall Polly Pod Brook **Poly Pond Brook** South Branch Tepehemus Brook Tepehemus Brook South Branch Township of Freehold Borough of Brielle **Roberts Swamp Brook Roberts Swamp Brook** (Downstream Reach) Borough of Manasquan **Roberts Swamp Brook Roberts Swamp Brook** (Downstream Reach) **Roberts Swamp Brook** Township of Wall **Roberts Swamp Brook** (Upstream Reach) Tepehemus Brook South Branch Township of Manalapan South Branch Tepehemus Brook

TABLE 3 – STREAM NAME CHANGES - continued

Community

<u>Old Name</u>

New Name

Township of Marlboro Township of Manalapan Township of Marlboro Township of Freehold Township of Manalapan Township of Marlboro Township of Freehold Township of Manalapan Township of Marlboro Township of Freehold Township of Wall Township of Wall Township of Marlboro Township of Freehold Borough of Englishtown Township of Colts Neck Township of Freehold Borough of Manasquan Township of Freehold

South Branch Tepehemus Brook Tributary A Tributary A Tributary A to Manasquan River Tributary B Tributary B Tributary B to Manasquan River Tributary C Tributary C Tributary C to Manasquan River Tributary D Tributary E Tributary F Tributary G Tributary H Tributary K Tributary L Tributary to Debois Creek Tributary to Weamaconk Creek Tributary to Yellow Brook Tributary to Yellow Brook Watson Creek Yellow Brook

Tepehemus Brook South Branch Manalapan Brook Tributary A Deep Run Tributary A Manasquan River Tributary A Manalapan Brook Tributary B Deep Run Tributary B Manasquan River Tributary B Pine Brook 2 Tributary C Deep Run Tributary C Manasquan River Tributary C Shark River Tributary D Shark River Tributary E Willow Brook Tributary F Willow Brook Tributary G Big Brook Tributary H Yellow Brook Tributary K Yellow Brook Tributary L Debois Creek Tributary Weamaconk Creek Tributary Miry Bog Brook Yellow Brook 2 Tributary Mac's Brook Yellow Brook 2

For this countywide FIS, Manalapan Brook has been restudied using detailed methods. The revised hydrologic analysis includes the entire basin area of Manalapan Brook (drainage area 17.53 square miles). The revised hydraulic analysis extends from its confluence with the South River at the border of Monmouth and Middlesex Counties in the Township of Manalapan to Moonlight Court in the Township of Millstone.

The areas studied by detailed methods were selected with priority given to all known flood hazard areas and areas of projected development and proposed construction.

Numerous flooding sources in the country were studied by approximate methods. Approximate analyses were used to study those areas having a low development potential or minimal flood hazards. The scope and methods of study were proposed to, and agreed upon by, FEMA and Monmouth County.

This FIS also incorporates the determinations of letters issued by FEMA resulting in map changes (Letter of Map Revision [LOMR], Letter of Map Revision - based

on Fill [LOMR-F], and Letter of Map Amendment [LOMA], as shown in Table 4, "Letters of Map Correction."

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<u>Community</u>	Flooding Source(s) Project Identifier	Date Issued	<u>Type</u>
Aberdeen, Township of	Raritan Bay / Removal of VE Zone designation	June 11, 1987	LOMR
Aberdeen, Township of	Mohingson Brook / Updated hydrologic analysis	April 17, 1995	LOMR
Colts Neck, Township of	Correction of corporate limits discrepancy	October 7, 1997	LOMR
Freehold, Township of	Correction of corporate limits discrepancy	October 7, 1997	LOMR
Hazlet, Township of	Flat Creek / Correction of street locations	April 12, 1994	LOMR
Manalapan, Township of	Pine Brook / Revised to include updated hydrologic and hydraulic info	June 11, 1987	LOMR
Manalapan, Township of	Still House Brook / Revised to include more detailed topographic data	November 1, 1999	LOMR
Marlboro, Township of	Big Brook Tributaries I and J / Revised to include updated hydrologic info	October 10, 1995	LOMR
Monmouth Beach, Borough of	Atlantic Ocean and Shrewsbury River / Revision of BFE, Designation from VE to AE Zones	May 12, 2005	LOMR
Neptune, Township of	Atlantic Ocean / Revision of Zones based on more detailed topographic data	January 14, 1991	LOMR
Neptune, Township of	Atlantic Ocean/ Revised to include updated hydrologic info	November 28, 2007	LOMR
Sea Bright, Borough of	Atlantic Ocean / Revision of Zones based on more detailed topographic data	January 14, 1991	LOMR

2.2 Community Description

Monmouth County is located in the central part of New Jersey, extending west to east across nearly the entire state, and is the northernmost county along the area known as the Jersey Shore. Monmouth County is bordered by Middlesex County to the north, Ocean County to the south, Burlington County to the southwest, Mercer County to the west and the Atlantic Ocean to the east. Monmouth County also borders the New York City Borough of Staten Island, on the other side of the Raritan Bay. Monmouth County spans 665 square miles and according to the 2000 census, the population in 2000 was 615,301. The County seat is the Borough of Freehold. Several major highways span Monmouth County, including the Garden State Parkway, New Jersey Turnpike, Interstate 195, and State Roads 9, 18, 34, 35, 36 and 70. Monmouth County is also home to the United States Naval Reservation, Naval Weapons Station Earle. Several rail lines and bus lines also pass through the County, including the New Jersey Transit North Jersey Coast Line. The eastern part of Monmouth County is a significant portion of the Jersey Shore, a very popular tourist destination, particularly in summer months. The western part of Monmouth County is far more residential and not as densely populated. The lowest points in Monmouth County occur at the Atlantic Ocean, while the highest point is atop Crawford Hill in the Township of Holmdel, which lies 391 feet above sea level.

Monmouth County has a temperate climate with warm summers and moderate winters. The average annual temperature is approximately 53 degrees Fahrenheit (°F), with January being the coldest month, averaging 31.3°F and July the warmest month averaging 74.5°F. The average annual precipitation is approximately 45 inches (U.S. Department of Commerce, 1961).

2.3 Principal Flood Problems

Flooding in Monmouth County is attributed mainly to tropical storms, extratropical cyclones (also known as northeasters) and to a lesser extent, severe thunderstorms. Near the Atlantic Ocean, Raritan Bay, Navesink River, Sandy Hook Bay, Shark River and Shrewsbury River, serious flooding problems are the result of high tidal surge and associated wave activity caused primarily by tropical storms, especially hurricanes. Usually occurring during late summer and early autumn, these storms can result in severe damage to coastal areas. Although extratropical cyclones can develop at almost any time of the year, they are more likely to occur during winter and spring. Thunderstorms are a common occurrence during the warm summer months.

Other low-lying areas throughout Monmouth County are vulnerable to severe flooding and flood-related damage, due to the periodic flooding caused by the overflow of streams and lakes. Heavy rainfall can result in higher than normal stages of Deal Lake, affecting the Borough of Allenhurst, the City of Asbury Park, the Borough of Deal and the Village of Loch Arbour, which frequently experiences property damage. Additional flooding in the Township of Aberdeen is attributed to tidal inundation and backwater from inadequate culverts. Due to high tidal stages on the Raritan Bay, the northern area of Aberdeen in the tidal plains of Matawan Creek, Mohingson Brook and Whale Creek is prone to flooding that affects Route 35 and properties near the shoreline. Areas adjacent to Mohingson Brook, Gravelly Run and Matawan Creek are prone to flooding due to inadequate culverts.

In the Borough of Deal, the lower portion of Poplar Brook is within the tidal range of the Atlantic Ocean. Runoff from severe rain periodically can cause the upper reach of Poplar Brook to overflow its banks. Residential properties will be affected by flooding on both stretches of Poplar Brook.

In the Borough of Eatontown, at times blockage by debris and refuse on Wampum Brook, Parkers Creek, Whale Pond Brook, Husky Brook, Crystal Book and Turtle Mill Brook can cause severe restrictions of culverts and contribute to local flooding. Most local flooding occurs upstream of State Route 35 on Parkers Creek, upstream of State Route 35 near Clinton Avenue, upstream of State Route 71 on Husky Brook at the twin 48-inch culverts under the Duncan Thecker Associates Service Road, and along the Lewis Street Bridge over Wampum Brook.

In the Township of Freehold, flooding has occurred along Manasquan River Tributary B upstream of Elton Adelphia Road, to a distance of 100 feet beyond normal channel bank. During severe conditions, Coventry Drive, which parallels the stream, has become impassable due to flooding. Debois Creek causes localized flooding where roadways cross the stream. The Strickland Road crossing has been flooded to a depth of two feet above the road surface during severe storms. The adjacent floodplain has been inundated, but with no extensive property damage. Debois Creek Tributary has experienced flooding during storm conditions due to constricted channel areas in the downstream portions of the stream. Extensive erosion in the channel of the tributary has been reported.

In the Township of Holmdel, flooding occurs upstream of State Route 34 and along South Street by Willow Brook, as well as near Middle Road by Waackaack Creek.

In the Township of Howell, localized flooding problems have occurred in the area of Long Brook and Bannen Meadow Brook. Long Brook has caused flooding of adjacent property near Wyckoff Road and the State Route 33 crossing. Howell Road is prone to flooding during severe conditions. Bannen Meadow Brook has caused flooding of adjacent property near Fort Plains Road and Casino Drive. The Fort Plains Road crossing is also flooded during severe flooding conditions.

In the Township of Manalapan considerable flooding occurs along Matchaponix Brook in the area of the corporate limits and at its junction with Pine Brook 2. Flood elevations along the lower reach of Pine Brook 2 are affected by backwater from the main branch of Matchaponix Brook. Flooding occurs along Pension Road near Clarks Mills. The housing development along Birmingham Drive, Terrytown Road and Winthrop Drive is subject to flooding from Pine Brook 2. The area along Pine Brook Road and Pease Road is flooded regularly when Pine Brook 2 Tributary C overflows its banks. Flooding problems also exist along Milford Brook in the area of Commack Lane, Pease Road and Tennant Road. Additional problems along Milford Brook arise during heavy rains in the area of Lafayette Mills and Lafayette Mills Road.

In the Borough of Matawan, flood gates are maintained by the community on Matawan Creek at the Lake Lefferts Dam. At times when the flood gates were not opened quickly enough during severe storm conditions, Ravine Drive was flooded to a depth of eight inches. Gravelly Brook has flooded Mill Road to a depth of six inches. The Municipal Garage, located on the floodplain of Gravelly Brook upstream of Church Street, has been flooded to a depth of eight inches, and the Church Street crossing has been flooded by Gravelly Brook to a depth of four inches. Downstream of the confluence of Gravelly Brook with Matawan Creek, the triple culvert at the Railroad Bridge causes backwater flooding of Aberdeen Road to a depth of five feet.

In the Township of Marlboro, considerable flooding occurs along Deep Run in the area of the corporate limits and Old Texas Road, a relatively flat region. A wide floodplain also occurs at Deep Run's junction with Deep Run Tributary B. Additionally, backwater effects of the culvert on Milford Brook at State Route 18 cause flooding upstream of that structure.

In the Township of Middletown, the bayshore portion of the township lies in a poorly drained floodplain with abundant swamp and marshland. The low banks of the stream and the low relief of the surrounding terrain render this region extremely vulnerable to flooding. During periods of heavy precipitation, the creeks overtop their banks and spread their floodwaters over the broad floodplain.

In the Township of Ocean, inland flow of the ocean tidal surges in restricted by weirs in the streams flowing to the ocean, as well as by lake storage. Flooding in the township is caused mostly by local rainstorms.

In the Borough of Spring Lake Heights, flooding occurs along Wreck Pond Brook, Wreck Pond North Branch and Poly Pond Brook. In general, localized flooding may occur under severe storm conditions due to poor surface drainage.

In the Borough of Tinton Falls, low-lying areas are subject to periodic flooding caused by the overflow of Swimming River, Pine Brook 1 and Jumping Brook 2. The most severe flooding occurs at the junction of Pine Brook 1 and Swimming River.

The Borough of Union Beach lies in a poorly drained floodplain with abundant swamps and marshland. The flat gradient of the streams and low relief of the surrounding terrain makes the area extremely vulnerable to flooding. During periods of heavy rainfall, streams within the Borough can overtop and spread floodwaters across the broad floodplain.

In the Township of Wall, flooding in the eastern section and remaining parts of the Township is caused by streams overflowing their banks. The non-tidal sections of Shark River, Manasquan River and Wreck Pond flow in wide, meandering channels. Urbanization in the areas of Watson Creek, Judas Creek (Upstream Reach), Roberts Swamp Brook (Upstream Reach), Poly Pond Brook and Heroys Pond Brook increase the runoff to these streams. Flooding can be aggravated by the accumulation of debris at culverts and bridges.

Several severe storms have struck Monmouth County in the past. The most severe of these storms are described below.

On September 14-15 of 1944, the entire coast of New Jersey was struck by hurricane-force winds. Wind velocity ranged from 90 miles per hour at Atlantic City to over 100 miles per hour at New York City. The storm produced a maximum tidal elevation of 7.4 feet recorded at a gage in Sandy Hook, located in the Township of Middletown (USACE, 1960; USACE, 1972). On November 25, 1950, a nor'easter brought gale-force winds and more than three inches of rainfall to the entire coastline of Monmouth County. A wind velocity of 70 miles per hour was recorded in the City of Long Branch. The gage at Sandy Hook recorded a maximum tidal elevation of 7.2 feet (USACE, 1960; USACE, 1972).

On November 6-7 of 1953, a strong storm passed through Monmouth County. The City of Long Branch recorded a wind velocity of 78 miles per hour. The gage at Sandy Hook recorded a maximum tidal elevation of 7.9 feet.

On September 12, 1960, Hurricane Donna struck the coast of Monmouth County with wind gusts to nearly 70 miles per hour. The concurrence of the hurricane tidal surge and mean high tide resulted in a maximum tidal elevation of 8.6 feet at the gage at Sandy Hook (U.S. Department of Commerce, 1971).

On March 6-8 of 1962, a strong storm passed through Monmouth County with sustained winds of 45 miles per hour and gusts to 70 miles per hour. The storm remained in the region for 60 hours. This usually long duration coincided with five successive spring high tides. The combination produced a maximum tidal elevation of 7.8 feet at the gage at Sandy Hook (USACE, 1960; USACE, 1972).

On August 26-28 of 1971, Tropical Storm Doria resulted in peak flows greater than any other recorded at 41 streams throughout New Jersey (State of New Jersey, 1972).

On August 9, 1976, Hurricane Belle struck the New Jersey coastline with winds of up to 100 miles per hour. In Asbury Park, 2.56 inches of rain fell in a 24-hour period. At Beach Haven, a tidal surge combined with high tide levels produced a tidal height six feet above normal stage (FEMA, 1985).

2.4 Flood Protection Measures

Development and use of land within floodplains and the floodway are regulated by the New Jersey Department of Environmental Protection, Division of Land Use Regulation. Additionally, several municipalities within Monmouth County have adopted stream cleanup programs, which clear debris near bridges and culverts to prevent backwater flooding during large storm events.

Several structural flood protection measures have been furnished throughout Monmouth County as well. Small dams are located on Conines Mill Pond and Indian Run in the Borough of Allentown, on Swimming River in the Township of Middletown, on Pine Brook near Tinton Avenue in the Borough of Tinton Falls, and scattered elsewhere throughout the County. Small weirs restrict the passage of tidal surges into inland areas on Whale Pond Brook and Poplar Brook in the Township of Ocean, and small erosion control structures have been placed along the streams in the Township of Holmdel. The Township of Wall has also placed small stone wave protection measures near roads and other critical infrastructure. A bulkhead was constructed along Marine Park in the Borough of Red Bank.

3.0 ENGINEERING METHODS

For the flooding sources studied in detail in the county, standard hydrologic and hydraulic study methods were used to determine the flood hazard data required for this FIS. Flood events of a magnitude which are expected to be equaled or exceeded once on the average during any 10-, 50-, 100-, or 500-year period (recurrence interval) have been selected as having special significance for floodplain management and for flood insurance rates. These events, commonly termed the 10-, 50-, 100-, and 500-year floods, have a 10-, 2-, 1-, and 0.2-percent chance, respectively, of being equaled or exceeded during any year. Although the recurrence interval represents the long term average period between floods of a specific magnitude, rare floods could occur at short intervals or even within the same year. The risk of experiencing a rare flood increases when periods greater than 1 year are considered. For example, the risk of having a flood which equals or exceeds the 100-year flood (1-percent chance of annual exceedence) in any 50-year period is approximately 40 percent (4 in 10), and, for any 90-year period, the risk increases to approximately 60 percent (6 in 10). The analyses reported herein reflect flooding potentials based on conditions existing in the county at the time of completion of this FIS. Maps and flood elevations will be amended periodically to reflect future changes.

3.1 Riverine Hydrologic Analyses

Hydrologic analyses were carried out to establish the peak discharge-frequency relationships for the flooding sources studied in detail affecting the county.

Precountywide Analyses

Each incorporated community within, and the unincorporated areas of, Monmouth County, with the exceptions of the Boroughs of Farmingdale, Interlaken, Lake Como, Neptune City and Roosevelt, as well as the Township of Upper Freehold, has a previously printed FIS report. The hydrologic and hydraulic analyses described in those reports have been compiled and are summarized below.

For the Township of Wall, total Tide-Frequency Curves for the Atlantic Ocean, Shark River and Manasquan River were developed by the USACE (U.S. Department of Housing and Urban Development, 1973). In the Boroughs of Oceanport and Shrewsbury, peak storm-tide levels on the Shrewsbury River, South Shrewsbury River and its tributaries, as well as Parkers Creek were previously estimated by the USACE (U.S. Department of Housing and Urban Development: April, 1973; June, 1973; USACE, 1972). These values were adjusted to reflect tide levels in the South Shrewsbury River. Additionally, in the Borough of Highlands, stillwater elevations for Sandy Hook Bay were obtained from a stage-frequency stillwater level graph in a USACE report. For inland areas surrounding Deal Lake, Fletcher Lake and Sylvan Lake, temporary ponding was found to result from heavy rainfall. A hydrologic budget was used to estimate this ponding, and rainfall information for the calculations was obtained from the U.S. Weather Bureau (U.S. Department of Commerce, 1963).

For streams studies by detailed methods in Monmouth County, several methods were used for hydrologic analyses. Special Report 38, developed by the NJDEP in cooperation with the USGS, uses a series of mathematical and graphical relationships to estimate discharge frequency data (U.S. Department of the Interior, 1974). Various parameters such as drainage area, main channel slope, surface storage area and an index of manmade impervious cover based on basin population and development conditions are used in this type of analysis.

A log-Pearson Type III analysis is a statistical technique for fitting frequency distribution data to a curve for the purpose of predicting design floods at a specific site (Water Resources Council, 1976). Probabilities of floods of various sizes can be extracted from this curve. This allows development of a drainage discharge ratio between a known discharge at a known point (gaging station) and the discharge at the area in question. The following equation was used at several streams in Monmouth County to determine this relationship:

$$Q_1 / Q_2 = (A_1 / A_2)^T$$

Where Q_1 and A_1 are the known discharge from a gaging station and the associated drainage area; Q_2 and A_2 are the discharge to be calculated and the associated drainage area, and T is the transfer exponent. This method is outlined by the Water Resources Council in Bulletins 15, 17, 17A and 17B (Water Resource Council, 1977; 1976; 1967).

The last common hydrologic analysis method used in this study is the Rational Method, used for streams with a drainage area less than approximately one square mile. The equation for the Rational Method is:

Q = CIA

Where Q is the discharge to be calculated, C is the runoff coefficient (dependent on land use), I is the rainfall intensity for the design storm and A is the drainage area.

Peak discharges for Big Brook (Downstream Reach) in the Townships of Colts Neck and Big Brook (Upstream Reach) in the Townships of Marlboro were determined using Special Report 38.

Gravelly Brook, in the Township of Aberdeen was studied using a log-Pearson Type III analysis. In the Township of Howell, it was studied by the Rational Method. In the Borough of Matawan and Township of Marlboro, peak discharges for Gravelly Brook were determined using Special Report 38. In the Township of Middletown, Jumping Brook 1 was studied using Special Report 38, and the Borough of Tinton Falls and Township of Wall used drainage area disposition to determine peak flows, then verified the results using Special Report 38.

Peak discharges for Matawan Creek in the Township of Aberdeen were determined using a log-Pearson Type III analysis, and were determined using Special Report 38 in the Township of Marlboro and Borough of Matawan.

In the Borough of Englishtown and Townships of Freehold and Manalapan, peak discharges for McGellairds Brook were determined using Special Report 38.

In the Townships of Manalapan and Marlboro, Milford Brook was studied using Special Report 38.

In the Boroughs of Eatontown, Shrewsbury and Tinton Falls, peak discharges for Parkers Creek were determined using Special Report 38.

In the Township of Colts Neck and the Borough of Tinton Falls, peak discharges for Pine Brook 1 were determined using Special Report 38.

In the Townships of Manalapan and Marlboro, peak discharges for Pine Brook 2 were calculated using Special Report 38.

In the Township of Howell, peak flows for Polypod Brook were determined using the Rational Method.

In the Township of Wall, peak discharges for Poly Pond Brook were determined during an overall basin study using drainage area disposition.

In the Borough of Deal and Township of Ocean, peak discharges for Poplar Brook were determined using a log-Pearson Type III analysis, using gage information obtained from Matawan Creek, and then verified using the Rational Method.

Peak discharges for the Shark River in the Borough of Tinton Falls and Township of Wall were determined using a log-Pearson Type III analysis, for which gage records were obtained for Manasquan River located in Squankum, New Jersey to determine a discharge-frequency relationship (U.S. Geological Survey, published annually).

In the Townships of Colts Neck, Middletown and Ocean, and the Borough of Tinton Falls, peak discharges for the Swimming River were determined using a log-Pearson Type III analysis.

Tepehemus Brook, in the Townships of Manalapan and Marlboro, was studied using Special Report 38.

In the Townships of Colts Neck and Freehold, peak discharges for Yellow Brook 2 Tributary were determined using Special Report 38. In the Townships of Hazlet and Middletown, as well as the Borough of Keansburg and Union Beach, peak discharges for Waackaack Creek were derived from a previous USACE study (USACE, 1968). However, in the Township of Holmdel, Special Report 38 was used to study this stream.

In the Townships of Freehold and Manalapan, as well as the Borough of Englishtown, peak discharges for Weamaconk Creek were determined by Special Report 38. Peak discharges for Wemrock Brook in the Townships of Freehold and Manalapan were determined from Special Report 38 as well.

In the Borough of Eatontown, peak flows for Whale Pond Brook were derived from a USACE study (U.S. Department of Housing and Urban Development, 1977). However, in the Township of Ocean, peak flows were calculated from an overall basin study using drainage area disposition.

In the Townships of Colts Neck, Holmdel and Marlboro, peak discharges for Willow Brook were determined using Special Report 38.

In the Townships of Colts Neck and Freehold, peak discharges for Yellow Brook were determined using Special Report 38.

In the Township of Aberdeen, peak discharges for Mohingson Brook and Whale Creek were determined using a gage analysis based on data obtained from gages on Matawan Creek.

In the Borough of Allentown, peak discharges for Doctors Creek were determined using a gage analysis. Flows for Indian Run were obtained using the Soil Conservation Service (SCS) Computer Program TR20 (U.S. Department of Agriculture, 1976).

In the Township of Colts Neck, peak discharges for Barren Neck Creek, Hockhockson Brook and Marl Brook were obtained using Special Report 38.

In the Township of Freehold, peak discharges for Applegates Creek, Burkes Creek, Debois Creek, Manasquan River Tributary A, Manasquan River Tributary B, Manasquan River Tributary C and Debois Creek Tributary were determined using Special Report 38. Peak flows for Manasquan River were determined using a log-Pearson Type III analysis.

In the Township of Hazlet, peak flows for East Creek, Flat Creek and Monascunk Creek were determined using Special Report 38.

In the Township of Holmdel, peak flows for Willow Brook, East Branch, Mahoras Brook and Ramanessin Brook were determined using Special Report 38.

In the Township of Howell, peak flows for Ardena Brook, Bannen Meadow Brook, Groundhog Brook, Haystack Brook and Long Brook were determined using Special Report 38. Peak flows for Manasquan River and Metedeconk River North Branch were determined using a log-Pearson Type III analysis, while Polypod Brook was studied using the Rational Method.

In the Township of Manalapan, peak discharges for Gander Brook, Matchaponix Brook, Still House Brook, Manalapan Brook Tributary A, Manalapan Brook Tributary B and Pine Brook 2 Tributary C were determined using Special Report 38.

In the Township of Marlboro, Barclay Brook, Deep Run, Deep Run Tributary A, Deep Run Tributary B, Deep Run Tributary C, Willow Brook Tributary F, Willow Brook Tributary G, Big Brook Tributary H, Yellow Brook Tributary K and Yellow Brook Tributary L were determined using Special Report 38.

In the Township of Middletown, peak discharges for Claypit Creek, Comptons Creek, Mahoras Creek, McClees Creek, Nut Swamp Brook, Poricy Brook and Town Brook were determined using Special Report 38.

In the Township of Millstone, peak discharges for Millstone River, Toms River and Rocky Brook (Upstream Reach) were determined using Special Report 38. Flows for Rocky Brook (Downstream Reach) were derived from the Flood Insurance Study for the Township of East Windsor, Middlesex County, New Jersey.

In the Township of Ocean, a basin study was conducted and a drainage area disposition was used to determine the peak discharges for Deal Tributary 1, Deal Tributary 2, Deal Tributary 3, Deal Tributary 3A, Deal Tributary 4, Deal Tributary 4A, Poplar Brook Tributary 1, Poplar Brook Tributary 2, Poplar Brook Tributary 3, Whale Pond Brook Tributary 1 and Whale Brook Tributary 2. Hog Swamp Brook was studied using the Rational Method, and Hollow Brook was studied using Special Report 38.

Peak flows for Turtle Mill Brook in the Borough of Oceanport were derived from a USACE study (USACE, 1969).

Peak flows for Parkers Creek North Branch in the Borough of Shrewsbury were determined using Special Report 38.

Peak flows for Wreck Pond Brook in the Borough of Spring Lake Heights were derived from the FIS for the Township of Wall (U.S. Department of Housing and Urban Development, 1976).

In the Township of Wall, a basin study was conducted and a drainage area disposition was used to determine the peak discharges for Wreck Pond Brook East Branch, Hannabrand Brook, Heroys Pond Creek, Judas Creek (Upstream Reach), Laurel Gully, Poly Pond Brook (Wall), Roberts Swamp Brook (Upstream Reach), Shark River Tributary D, Shark River Tributary E, Watson Creek and Wreck Pond Brook. Calculated flows were verified using Special Report 38.

In the Borough of Little Silver, streams were divided into two basic systems. The first included Little Silver Creek, Little Silver Tributary A and Little Silver

Tributary 1. The second included Little Silver Tributary 2, Little Silver Tributary 2A and Little Silver Tributary 2B. The watershed contributing to each system was defined and converted to a standard model using parameters that allow the selection of a similar gaged watershed, necessary because no gages exist on these streams. For gage data, Swimming River in the Borough of Red Bank was used, with records since 1923. Using a standard log-Pearson Type III analysis, peak discharge-frequency curves were developed for the two watersheds and applied uniformly.

In the Borough of Manasquan peak discharge-drainage area relationships were determined by high water marks for Mac's Brook, Roberts Swamp Brook (Downstream Reach) and Judas Creek (Downstream Reach).

Revised Analyses

Information on the methods used to determine peak discharge-frequency relationships for the stream restudied as part of this countywide FIS is shown below.

All discharges shown below for Manalapan Brook were calculated in accordance with procedures outlined in the publication by the USGS entitled "Nationwide Summary of U.S. Geological Survey Regional Regression Equations for Estimating Magnitude and Frequency of Floods for Ungaged Sites, 1993", also referred to as Water Resources Investigation (WRI) Report 94-4002 (U.S. Department of the Interior, 1982).

Flow locations were selected at various points along the reaches of this brook. Locations were first selected based on prior documented FEMA flow locations for prior studies of the drainage basin (FEMA, 1981; U.S. Department of Housing and Urban Development, 1977) and on USGS gage locations (U.S. Department of the Interior, retrieved 2006: USGS 01405303, 04105310, 01405330, 01405335, 01405400). Additional flow locations were added along the brook to provide a uniform drainage analysis of the study area.

Based on WRI Report 94-4002, the variables governing the peak stream flows for each of the flow locations are Drainage Area, Main Channel Slope, Population Density and Surface Storage Index. With the flow locations selected, the drainage area to each of the locations was delineated based on the Monmouth County GIS 2-foot contour topography (State of New Jersey, Provided 2006).

The Main Channel Slope was measured between points which are 10 percent and 85 percent of the main channel length upstream from the study site. This was also measured based on the Monmouth County GIS 2-foot contour topography. The Population Density was based on Census 2000 Data obtained form the New Jersey Department of Labor and Workforce Development (State of New Jersey, 2001). First the Population density was calculated for each overall community area. A weighted value was then calculated for each incremental drainage area based on estimated community coverage.

The Surface Storage Index is the percentage of the drainage area occupied by lakes and swamps. This was measured based on the New Jersey 2002 digital orthoimagery (New Jersey Image Warehouse, Retrieved 2006). There are two reservoirs, Millhurst Pond and Bulks Lake located along Manalapan Brook. Neither reservoir has significant surcharge capacity or serves as a flood control structure. Therefore, this study neglects any detention effects of these ponds. Their areas, however, are reflected in the Surface Storage Index.

There are four USGS gages located within Manalapan Brook's limit of detailed study. These gages contain no peak flow records; however they are utilized for drainage area checks. USGS 01405400 is located in Middlesex County downstream of the limit of detailed study. This gage contains 49 years of record but does not satisfy the allowable drainage area weighting limit of 50 percent. As such, this gage was not utilized in this study.

A regression analysis was performed at each of the flow locations in accordance with WRI Report 94-4002 to calculate flood discharges. The regression analysis was performed utilizing the National Flood Frequency Program (NFF) (U.S. Department of the Interior, 2002). This program employs the New Jersey regional regression equations established in Special Report 38 (U.S. Department of the Interior, 1974) to calculate discharges for the 2-, 5-, 10-, 25-, 50- and 100year flood. These equations are applicable to rural and urbanized areas because they account for basin development through the Population Density variable. The discharges for the 500-year flood are extrapolated by the NFF. This involves fitting a log-Pearson Type III curve to the 2- and 100-year flood discharges and extrapolates the curve to the 500-year flood discharge.

The governing variables were input into the NFF program and regression flows output by the program were used as the discharge at the selected flow locations.

A summary of the drainage area-peak discharge relationships for all streams studied by detailed methods is shown in Table 5, "Summary of Discharges" and in Figure 1, "Frequency Discharge, Drainage Area Curves."

TABLE 5 - SUMMARY OF DISCHARGES

FLOODING SOURCE	DRAINAGE AREA	PEAK DISCHARGES (cfs)			
AND LOCATION	(sq. miles)	10-PERCENT	2-PERCENT	1-PERCENT	0.2-PERCENT
APPLEGATES CREEK Upstream of confluence with Debois Creek	1.68	246	425	536	872
ARDENA BROOK Upstream of confluence with Manasquan River	1.22	170	293	369	590

FLOODING SOURCE	DRAINAGE AREA PEAK DISCHARGES (cfs)				
AND LOCATION	(sq. miles)	10-PERCENT	<u>2-PERCENT</u>	<u>1-PERCENT</u>	0.2-PERCENT
BANNEN MEADOW BROOK					
Upstream of confluence with Manasquan River	2.00	327	551	690	1,094
BARREN NECK CREEK At limit of detailed study	1.22	230	390	500	800
BIG BROOK (DOWNSTREAM REACH) Upstream of Willow					
Brook At limit of detailed study	10.11 8.22	770 750	1,280 1,250	1,590 1,550	2,550 2,500
BURKES CREEK Upstream of confluence					
with Debois Creek	1.43	175	307	388	631
CLAYPIT CREEK	1.54	205	100	(0 5	1 000
At Locust Avenue At Lakeside Avenue	1.54 0.91	285 185	480 315	605 425	1,000 720
The Lakoblac Tryende	0.51	165	515	120	720
COMPTONS CREEK					
At Campbell Avenue	5.86	682	1,093	1,326	2,120
DEAL LAKE TRIBUTARIES					
*	3.00	330	570	750	*
*	1.00	120	350	520	900
*	0.70	10.70	35	45	90
*	0.01	1.10	4.40	6.50	20.20
DEBOIS CREEK Upstream of confluence					
with Manasquan River Upstream of confluence of Applegates Creek and	7.70	530	910	1,110	1,640
Debois Creek Tributary Approximately 800 feet	5.50	450	745	920	1,370
upstream of Jones Siding Road	2.08	163	282	348	518
DEBOIS CREEK TRIBUTARY Upstream of confluence					
with Debois Creek	1.17	218	366	453	699

FLOODING SOURCE	DRAINAGE AREA		PEAK DISCH	IARGES (cfs)	
AND LOCATION	(sq. miles)	10-PERCENT	2-PERCENT	<u>1-PERCENT</u>	0.2-PERCENT
DOCTORS CREEK					
DOCTORS CREEK At Fowlers Bridge Road At upstream corporate	17.20	1,180	1,830	2,170	3,150
limits	16.30	1,150	1,785	2,115	3,065
EAST CREEK					
At SR 36	2.31	463	771	947	1,385
At Middle Road	1.41	367	608	747	1,090
At SR 35	0.58	213	351	432	630
FLAT CREEK					
At SR 36	2.08	644	1,070	1,320	1,823
Upstream of confluence of				<i></i>	
Monascunk Creek	0.86	334	559	692	910
Upstream of SR 36	0.67	198	325	401	575
GRAVELLY RUN					
At Lake Matawan Dam	2.57	589	1,107	1,413	2,405
Upstream of confluence	2.37	505	1,107	1,115	2,100
with Metedeconk River					·
North Branch	0.92	113	199	250	403
North Draiton	0.72	115	177	200	100
GROUNDHOG BROOK					
Upstream of confluence					
with Haystack Brook	1.56	178	306	382	606
Upstream of confluence of					
Polypod Brook	0.75	140	220	300	500
HAYSTACK BROOK					
Upstream of confluence	11.40	485	830	1,020	1,550
with the Metedeconk					
River North Branch	()	100	(D C		1 000
Approximately 3,000 feet	6.30	402	675	832	1,292
upstream of Ramtown					
Greenville Road	1 2 4	249	597	725	1,135
Approximately 1,700 feet	4.34	348	587	123	1,155
from the Glenn Road					
crossing					
HOCKHOCKSON BROOK					
Upstream of junction with					
Pine Brook 1	6.86	590	990	1,230	2,100
Approximately 700 feet				-	
upstream of Water					
Street	4.88	540	910	1,140	1,500
Upstream of Roller Road	0.52	303	385	443	516
-					

FLOODING SOURCE	DRAINAGE AREA					
AND LOCATION	(sq. miles)	10-PERCENT	2-PERCENT	<u>1-PERCENT</u>	0.2-PERCENT	
HOLLOW BROOK						
*	1.00	480	560	650	900	
*	0.70	55	85	90	160	
*	0.10	65	95	140	240	
*	0.01	14	24	32	40	
INDIAN RUN						
At confluence with						
Doctors Creek	1.80	630	900	1,045	1,550	
At upstream corporate						
limits	1.60	615	875	1,025	1,500	
JUMPING BROOK 1						
At Neptune corporate						
limits	3.70	970	1,500	1,760	2,490	
Upstream of tributary, 800 feet upstream of						
corporate limits	2.47	680	1,090	1,270	1,800	
At bridge at Shadow Lake	2.11	425	685	835	1,300	
At Middletown-Lincroft					-)	
Road	1.10	280	455	555	820	
LONG BROOK						
Upstream of confluence						
with Manasquan River	2.40	281	476	596	951	
Upstream of Adelphia-						
Farmingdale Road	1.46	242	413	517	829	
Upstream of Varnderveer						
Road	0.96	167	289	363	588	
MAHORAS BROOK						
At downstream corporate						
limits	3.06	645	965	1,115	1,505	
At NY & Long Branch			004		1 2 2 2	
Railroad	2.74	597	896	1,035	1,393	
At Holland Road	1.37	366	549	635	855	
Upstream of confluence	1.20	005	1.045	1 405	1.000	
with Waackaack Creek	1.30	825	1,245	1,425	1,920	
MANASQUAN RIVER						
Downstream of Southard						
Avenue	33.46	1,380	1,963	2,239	2,932	
Approximately 3,500 feet		-				
downstream of West						
Farms Road	28.65	1,200	1,736	1,980	2,590	

FLOODING SOURCE	DRAINAGE AREA		ARGES (cfs)		
AND LOCATION	(sq. miles)	10-PERCENT	<u>2-PERCENT</u>	1-PERCENT	0.2-PERCENT
MANASQUAN RIVER (continued) Upstream of confluence of					
Ardena Brook Upstream of confluence of	25.18	1,099	1,566	1,786	2,335
Barren Meadow Brook Upstream of confluence of	21.80	980	1,395	1,591	2,081
Long Brook Upstream of confluence of	19.40	893	1,271	1,450	1,896
Debois Creek	10.41	544	774	883	1,155
Upstream of Jackson Mills Road Upstream of confluence of Manasquan River	8.46	459	653	745	975
Tributary B	6.87	391	557	635	830
MANASQUAN RIVER TRIBUTARY A Upstream of confluence with Manasquan River	2.10	325	545	685	1,085
MANASQUAN RIVER TRIBUTARY B Upstream of confluence with Manasquan River	1.42	214	366	458	727
MANASQUAN RIVER TRIBUTARY C Upstream of confluence with Manasquan River	1.80	312	527	657	1,043
MARL BROOK Upstream of Mine Brook At limit of detailed study	1.54 1.04	270 210	460 370	580 470	980 800
MATAWAN CREEK At mouth with Raritan					
Bay Unstroom of Mahingson	13.19	1,850	3,480	4,440	7,560
Upstream of Mohingson Brook At USGS gaging station at	9.94	1,520	2,855	3,640	6,200
Lake Lefferts Dam	6.11	1,080	2,030	2,590	4,410
At New Brunswick Road At upstream corporate	4.18	830	1,555	1,990	3,380
limits	1.16	340	635	810	1,380

FLOODING SOURCE	DRAINAGE AREA	PEAK DISCHARGES (cfs)				
AND LOCATION	(sq. miles)	10-PERCENT	2-PERCENT	1-PERCENT	0.2-PERCENT	
MCCLEES CREEK At Navesink River Road	4.56	690	1,100	1,380	1,900	
At Whipporwill Valley Road	2.95	430	700	865	1,280	
At Sleepy Hollow Road	1.56	260	430	535	790	
MCGELLAIRDS BROOK At downstream corporate limits Upstream of Freehold/ Manalapan corporate	14.99	760	1,240	1,510	2,490	
limits	2.46	143	272	334	601	
METEDECONK RIVER NORTH BRANCH Upstream of Howell- Lakewood corporate						
limits Upstream of confluence of	34.90	671	998	1,160	1,612	
Gravelly Run	32.79	658	979	1,137	1,581	
Upstream of confluence of Haystack Brook Upstream of Ramtown-	21.39	535	797	926	1,287	
Greenville Road Upstream of Lakewood-	20.78	528	786	913	1,235	
Farmingdale Road Approximately 4,500 feet upstream of Lakewood-	20.30	522	777	902	1,254	
Farmingdale Road Approximately 755 feet	19.69	514	765	889	1,235	
upstream of Kent Road	18.08	490	730	847	1,178	
Upstream of Church Road Approximately 600 feet downstream of Aldrich	15.40	452	672	781	1,085	
Road Upstream of Farmingdale	11.26	387	576	669	930	
Road Approximately 1,800 feet upstream of Hulse's	10.80	348	518	602	837	
Road	7.20	323	480	558	775	
MILLSTONE RIVER At Millstone/Monroe						
corporate limits	7.47	705	1,095	1,305	1,870	
At Sweetmans Lane	2.52	249	441	561	730	

FLOODING SOURCE	DRAINAGE AREA	PEAK DISCHARGES (cfs)				
AND LOCATION	(sq. miles)	10-PERCENT	2-PERCENT	<u>1-PERCENT</u>	0.2-PERCENT	
MINE BROOK Upstream of Yellow						
Brook Downstream of Marl	5.63	740	1,230	1,540	2,500	
Brook	5.28	670	1,120	1,400	2,300	
Upstream of Marl Brook	3.74	500	850	1,060	1,800	
At limit of detailed study	2.40	350	600	760	1,220	
MIRY BOG BROOK						
Upstream of Yellow	1.00	170	200	280	625	
Brook	1.08	170	300	380	625	
MOHINGSON BROOK At confluence with						
Matawan Creek	2.44	570	1,070	1,360	2,320	
Upstream of GSP	2.17	525	985	1,255	2,140	
Upstream of Church						
Street	1.74	450	845	1,075	1,830	
At upstream corporate						
limits	0.65	225	425	540	920	
MONASCUNK CREEK At confluence with Flat						
Creek	0.96	285	469	577	835	
At SR 35	0.75	255	416	511	730	
NUT SWAMP BROOK						
At bridge at Shadow Lake	2.93	425	690	840	1,310	
At GSP	2.40	375	620	760	1,130	
At Middletown-Lincroft	2.10	0,0	020	,	-,	
Road	1.55	255	425	525	785	
PARKERS CREEK At Sunnybank Drive	6.12	700	1,110	1,350	2,050	
Downstream of confluences with Parkers Creek North	0.12	700	1,110	1,550	2,050	
Branch and Wampum Brook Upstream of confluences with Parkers Creek	5.47	740	1,190	1,440	2,300	
North Branch and	1.50	200	400	500	0.20	
Wampum Brook	1.58	290	480	580	920	
At Main Street	1.24	250	410	500	810	
At Railroad crossing	1.24	250	410	500	810	

FLOODING SOURCE	DRAINAGE AREA	PEAK DISCHARGES (cfs)				
AND LOCATION	(sq. miles)	10-PERCENT	2-PERCENT	<u>1-PERCENT</u>	0.2-PERCENT	
PARKERS CREEK NORTH BRANCH Upstream of confluence with Parkers Creek At limit of detailed study	1.03 0.83	180 150	310 250	380 310	600 510	
PINE BROOK 1 Upstream of confluence with Swimming River Upstream of Tinton	13.20	900	1,480	1,830	2,900	
Avenue	11.04	950	1,560	1,930	3,100	
Upstream of junction with Hockhockson Brook	4.18	430	730	900	1,950	
POLYPOD BROOK Upstream of confluence with Groundhog Brook	0.72	174	220	255	413	
POPLAR BROOK At confluence with Atlantic Ocean Upstream of Monmouth	3.74	593	1,044	1,292	1,990	
Road Upstream of confluence with Poplar Brook	2.93	441	777	962	1,481	
Tributary 2 Upstream of Willow	1.70	531	676	762	864	
Drive	1.15	405	515	589	685	
PORICY BROOK At confluence with						
Navesink River	2.86	425	690	845	1,330	
At Normandy Road At Middletown-Lincroft	1.31	290	480	590	900	
Road	1.21	275	460	570	845	
RAMANESSIN BROOK Upstream of confluence with Willow Brook Approximately 1,200 feet	6.49	1,000	1,700	200	2,900	
northwest of Middletown and Stillwell Avenue, downstream Approximately 1,200 feet northwest of Middletown and	5.06	950	1,620	1,900	2,700	
Stillwell Avenue, upstream	3.83	760	1,399	1,599	2,199	

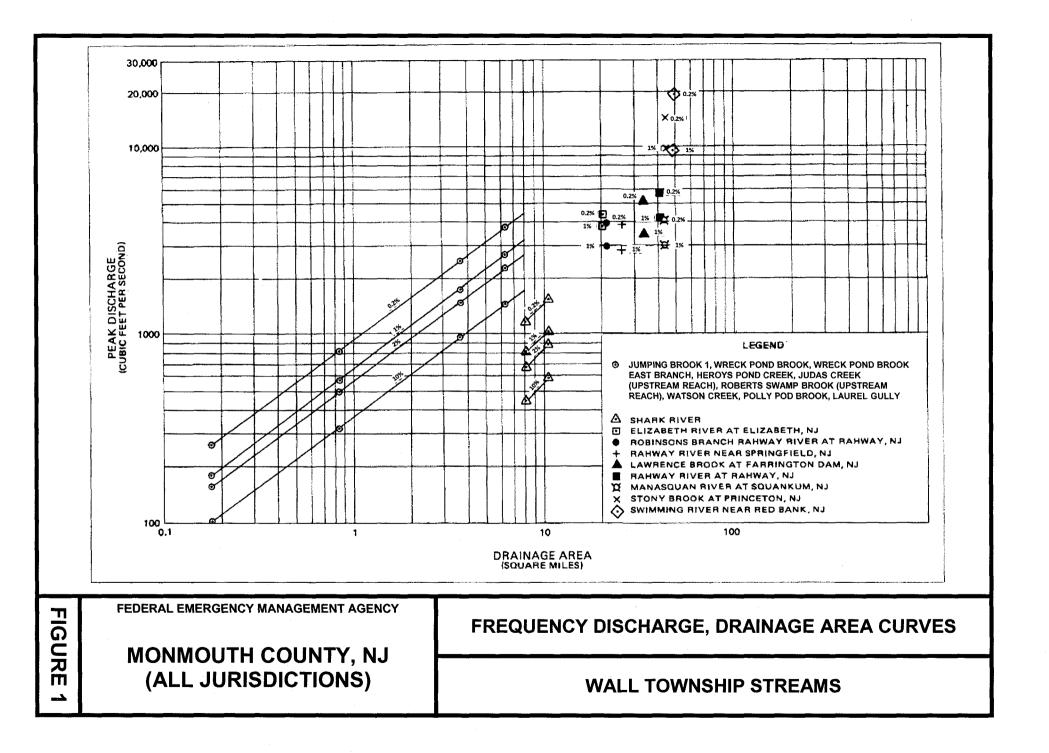
FLOODING SOURCE	DRAINAGE AREA	PEAK DISCHARGES (cfs)				
AND LOCATION	(sq. miles)	10-PERCENT	2-PERCENT	<u>1-PERCENT</u>	0.2-PERCENT	
RAMANESSIN BROOK (continued) Approximately 1,400 feet downstream of Robert						
Road	2.43	510	830	1,000	1,460	
Upstream of Longstreet Road	1.03	300	500	620	960	
ROCKY BROOK (DOWNSTREAM REACH) At Disbrow Road	6.10	410	690	870	1,310	
ROCKY BROOK (UPSTREAM REACH) Approximately 2,240 feet downstream of Perrineville Road	3.19	327	562	705	1,128	
SHARK RIVER At Neptune corporate limits	7.99	440	670	790	1,140	
At Shark River Road	2.37	130	200	230	330	
SWIMMING RIVER	61.70	3,844	0 35 2	11 176	21,970	
At Hubbards Avenue USGS gage near Red	61.70	3,044	8,253	11,176	21,970	
Bank	48.50	3,280	7,060	9,630	18,000	
At Swimming River Road	48.50	3,248	6,973	9,443	18,563	
TEPEHEMUS BROOK SOUTH BRANCH Upstream of Freehold/ Manalapan corporate						
limits	1.74	200	340	430	790	
TOMS RIVER At Millstone-Jackson corporate limits	3.39	191	342	433	717	
At upstream study limit	2.92	219	390	495	808	
•						
TOWN BROOK At Park Avenue	2.69	385	629	769	1,220	
At SR 35	1.33	219	366	450	715	
			-			
TURTLE MILL BROOK *	4.20	600	900	1,100	2,600	
*	0.64	200	250	320	650	

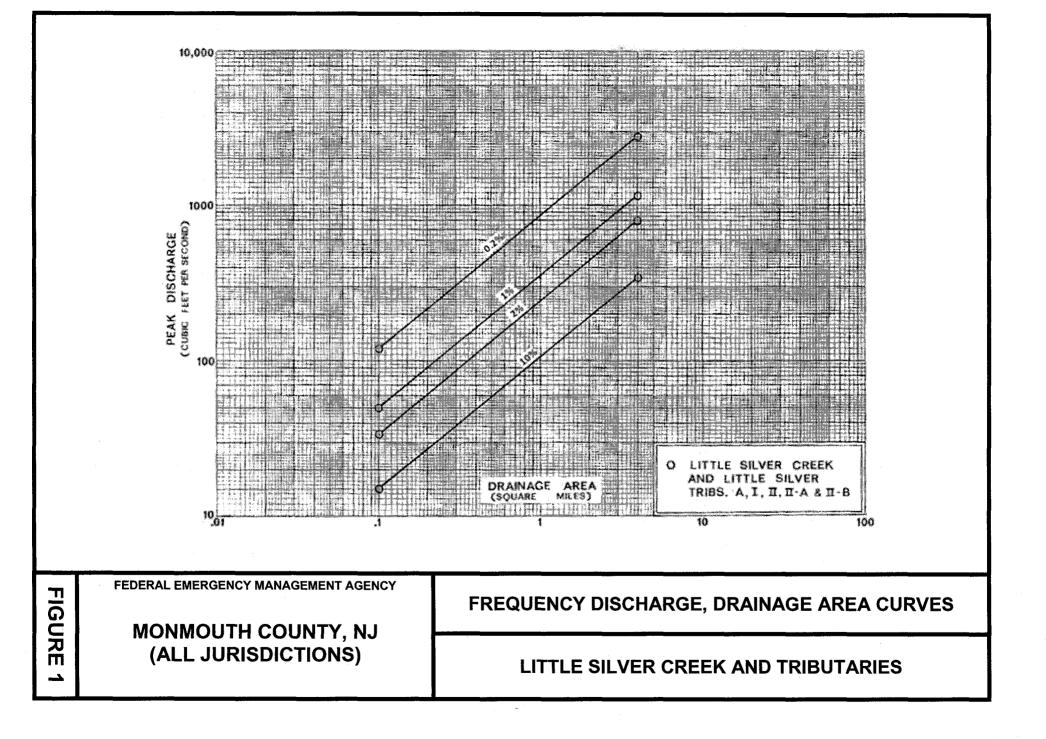
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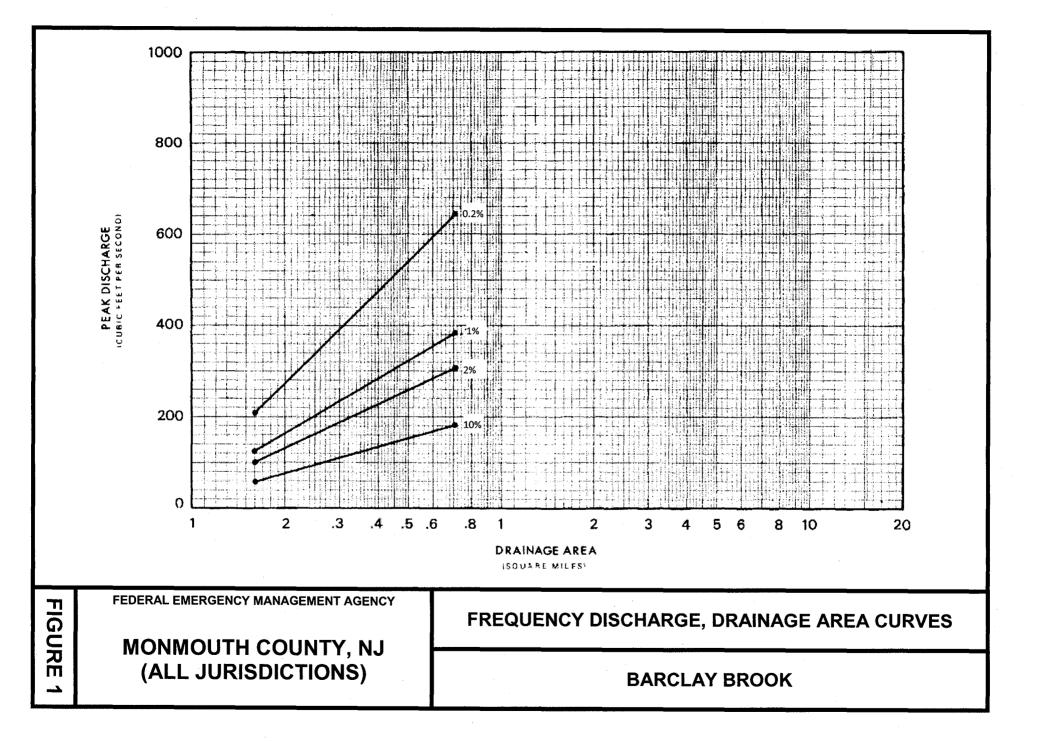
FLOODING SOURCE	DRAINAGE AREA				
AND LOCATION	<u>(sq. miles)</u>	10-PERCENT	<u>2-PERCENT</u>	<u>1-PERCENT</u>	0.2-PERCENT
WAACKAACK CREEK At downstream corporate					
limits	7.44	1,200	1,800	2,080	2,800
At SR 36	6.77	1,125	1,685	1,950	2,620
At downstream corporate					
limits	5.64	1,125	1,685	1,950	2,620
Upstream of confluence					
with Mahoras Brook	4.34	355	530	615	825
WAMPUM BROOK					
At Main Street	2.86	390	630	760	1,200
At Lewis Street	2.01	360	580	710	1,110
		200	200	, 10	.,
WEAMACONK CREEK					
At downstream corporate					
limits	7.66	410	650	790	1,360
Upstream of Freehold/					
Manalapan corporate					
limits	1.49	146	230	300	494
Upstream of US Route 9	1.02	106	167	219	360
WEAMACONK CREEK TRIBUTARY At confluence with Weamaconk Creek At upstream corporate limits	0.34	120	185	220	320
limits	0.19	90	135	160	215
WEMROCK BROOK (within the Township of Freehold) Upstream of Freehold/ Manalapan corporate limits	2.58	215	401	423	694
WHALE POND BROOK At Whale Pond Road Downstream of Neptune	3.94	320	730	1,080	2,700
Highway Upstream of Neptune	2.64	240	560	850	2,000
Highway	1.81	180	410	680	1,500
WHALE POND BROOK AND TRIBUTARIES					
*	4.50	480	875	1,450	4,200
*	0.70	75	260	350	610
*	0.10	10	34	48	140

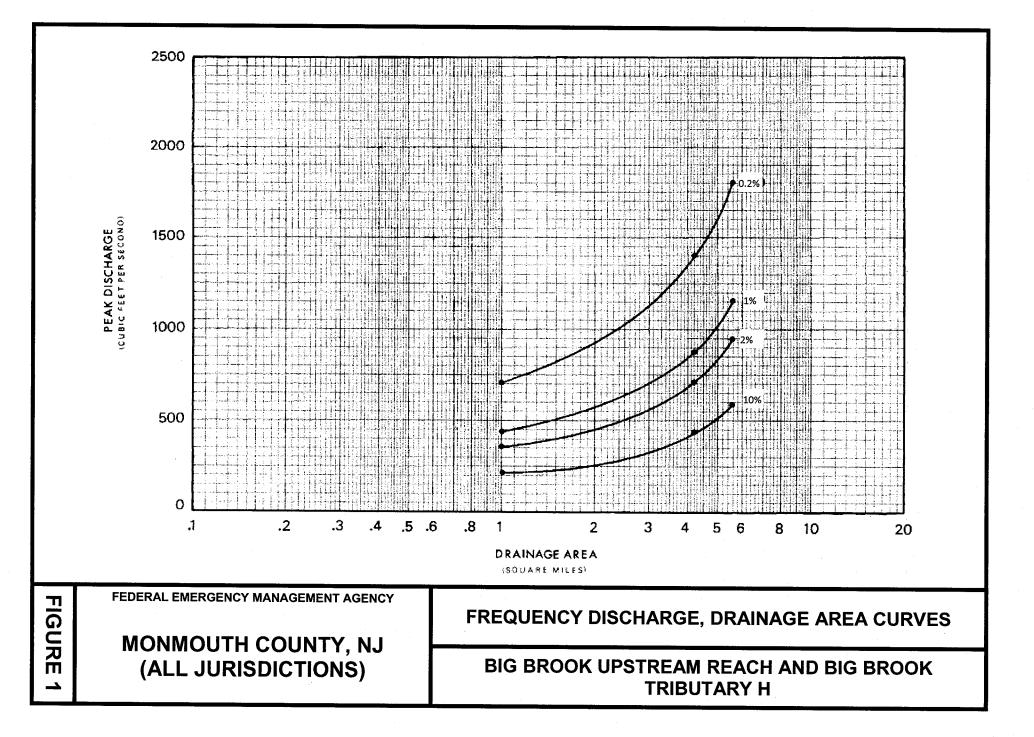
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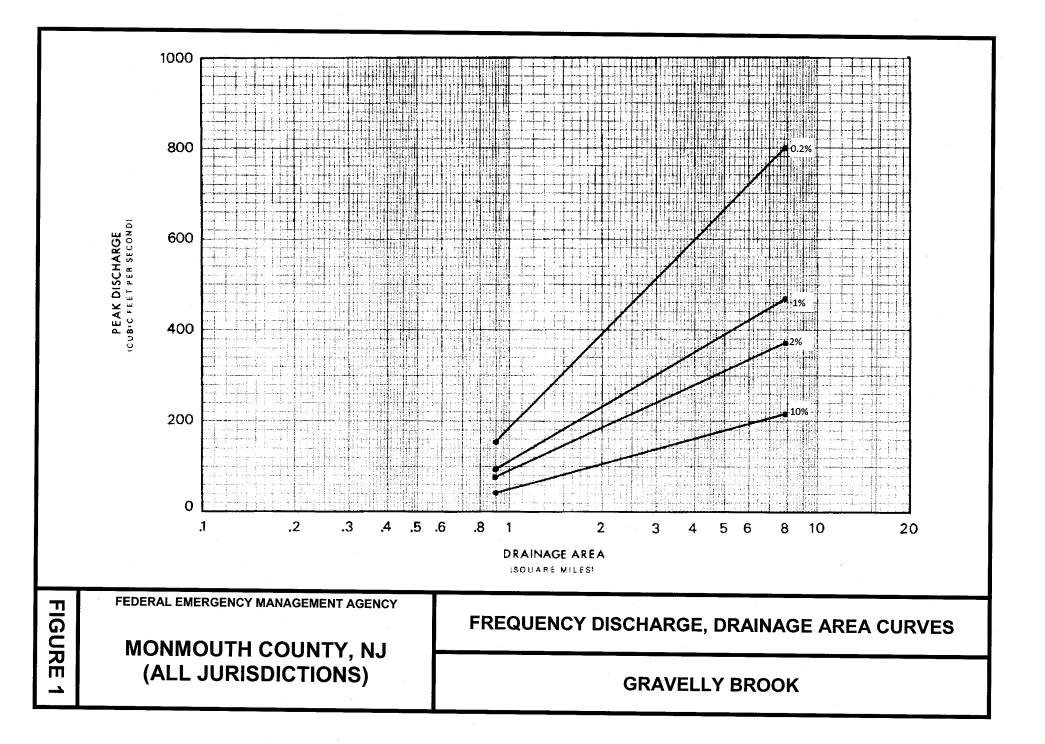
FLOODING SOURCE	DRAINAGE AREA	PEAK DISCHARGES (cfs)				
AND LOCATION	(sq. miles)	10-PERCENT	<u>2-PERCENT</u>	<u>1-PERCENT</u>	0.2-PERCENT	
WILLOW BROOK Downstream of confluence of	-					
Ramanessin Brook Upstream of confluence of	14.07	1,700	2,800	3,400	5,200	
Ramanessin Brook	7.58	1,065	1,790	2,735	3,275	
Downstream of CR 520	5.00	1,100	1,850	2,250	3,100	
Upstream of CR 520 Upstream of confluence of East Branch Willow	2.25	700	1,200	1,500	2,400	
Brook	1.75	400	700	900	1,450	
At limit of detailed study	1.30	320	550	700	1,100	
WILLOW BROOK, EAST BRANCH At confluence with						
Willow Brook	0.50	130	210	250	400	
WRECK POND BROOK Upstream of Old Mill						
Road	7.20	1,600	2,450	2,940	4,200	
YELLOW BROOK Downstream of Mine						
Brook	15.45	1,580	2,580	3,180	5,000	
Upstream of Mine Brook Downstream of Tributary	9.82	1,020	1,680	2,090	3,350	
to Yellow Brook Upstream of Tributary to	8.80	920	1,520	1,880	3,000	
Yellow Brook Junction North and South	7.72	820	1,380	1,710	2,800	
Branch	6.03	790	1,330	1,650	2,640	
Upstream of South Branch	2.02	370	630	790	1,300	
At limit of detailed study Upstream of confluence of Tributary to Yellow	1.76	350	590	750	1,220	
Brook	1.18	258	438	549	873	
YELLOW BROOK 2 Upstream of the Township of Freehold/Township of						
Colts Neck corporate limits Upstream of the confluence of Tributary to Yellow	1.76	360	610	770	1,220	
Brook	1.18	258	438	549	873	
YELLOW BROOK 2 TRIBUTARY Upstream of confluence with Yellow Brook 2	0.66	176	301	377	599	

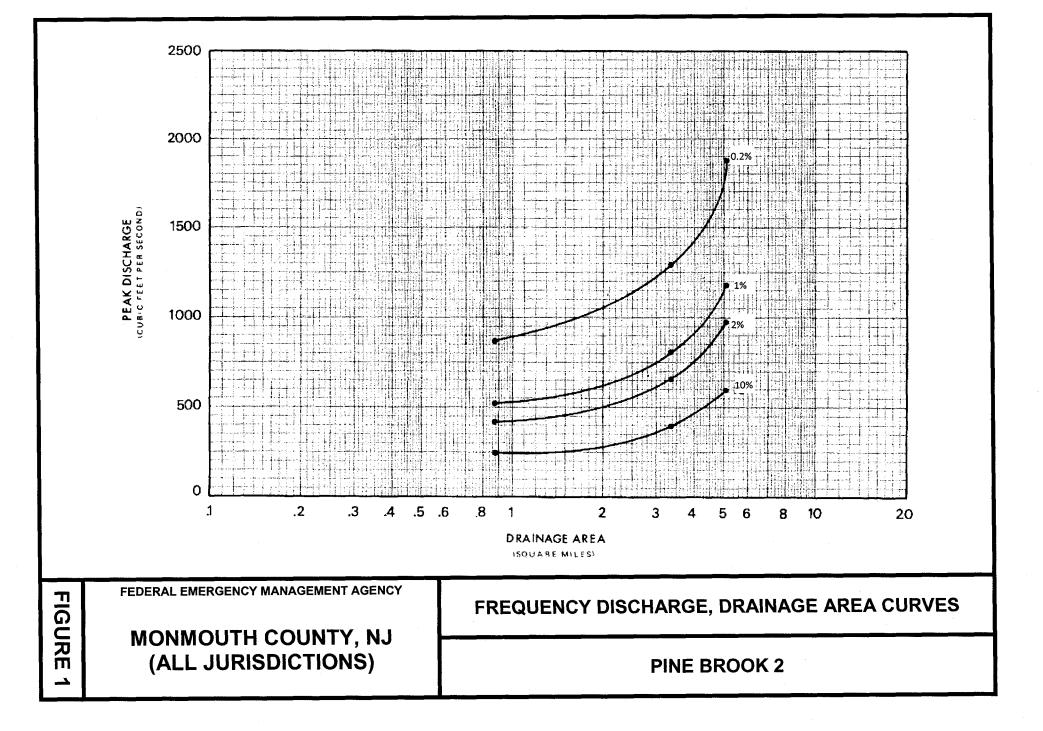


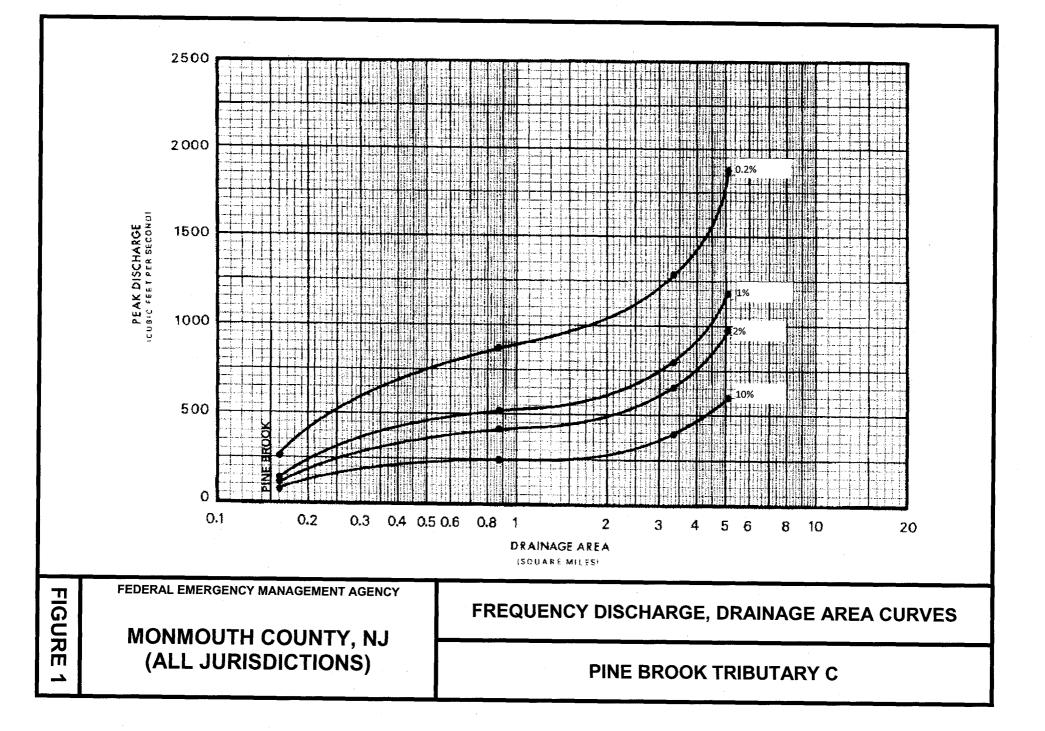


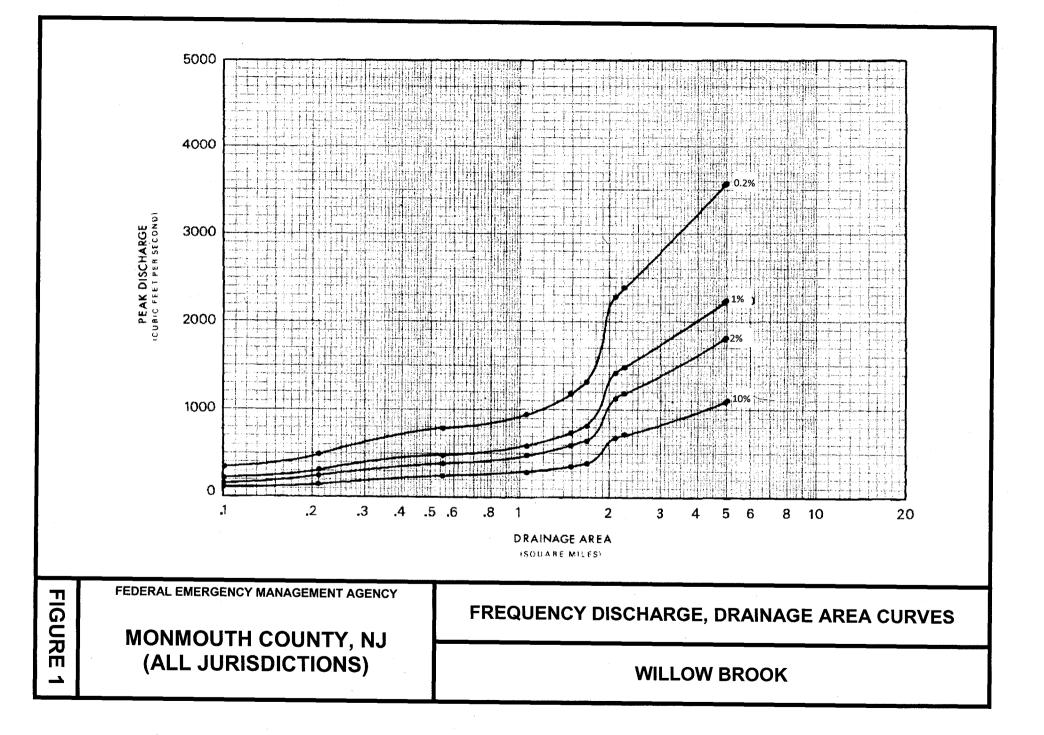


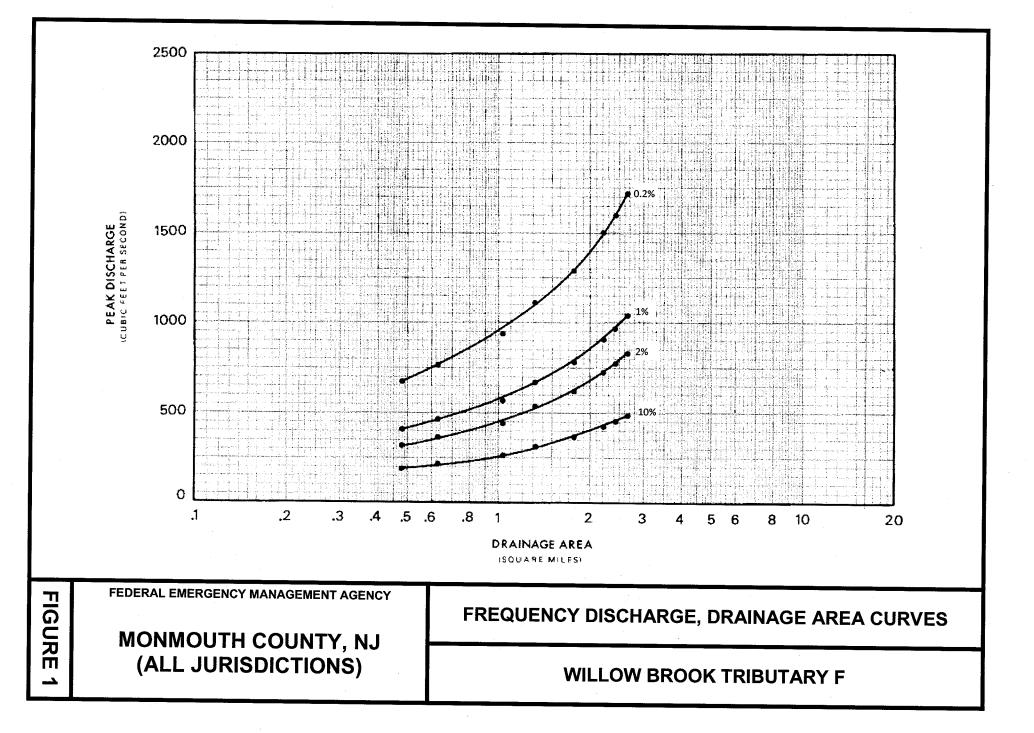


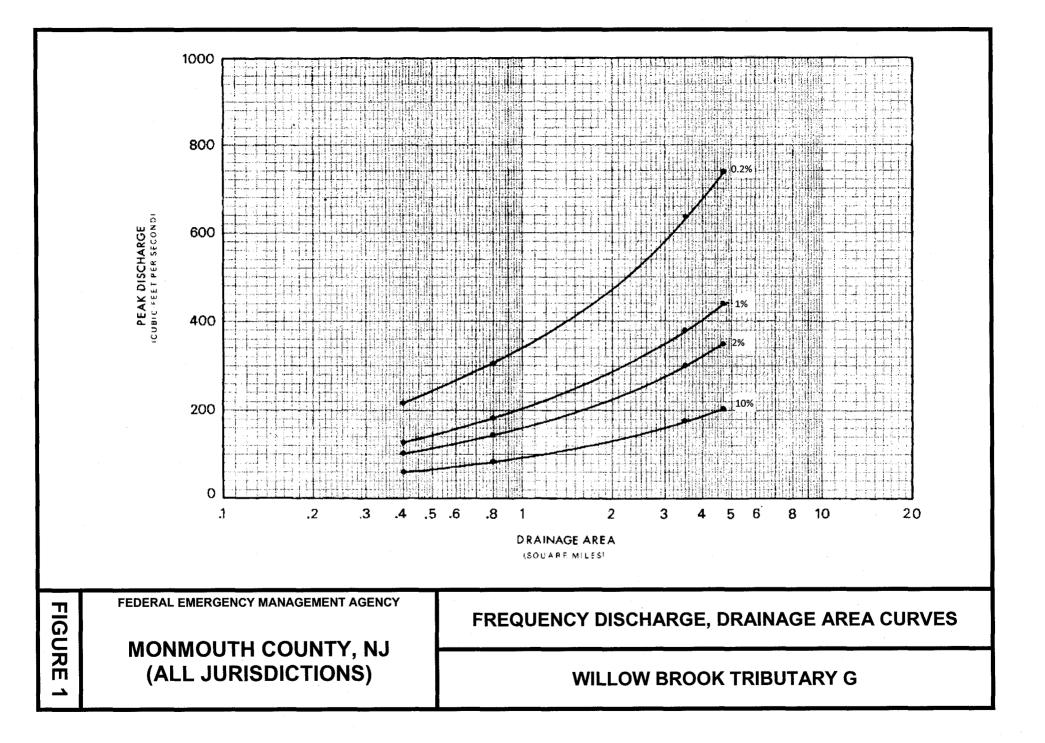


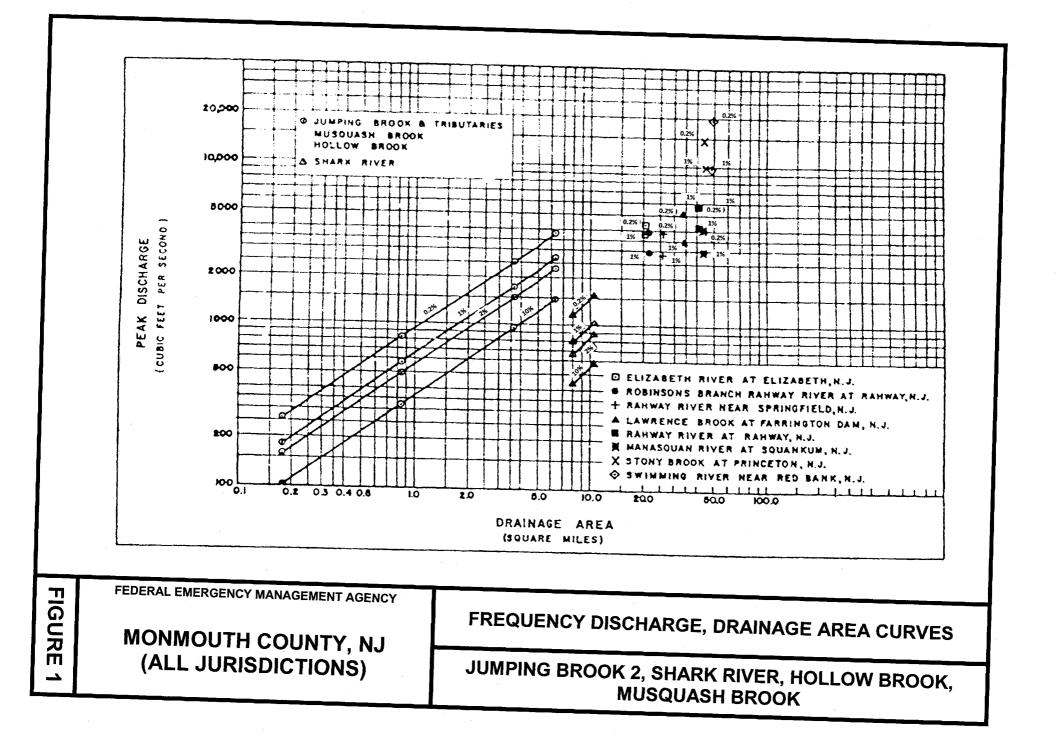


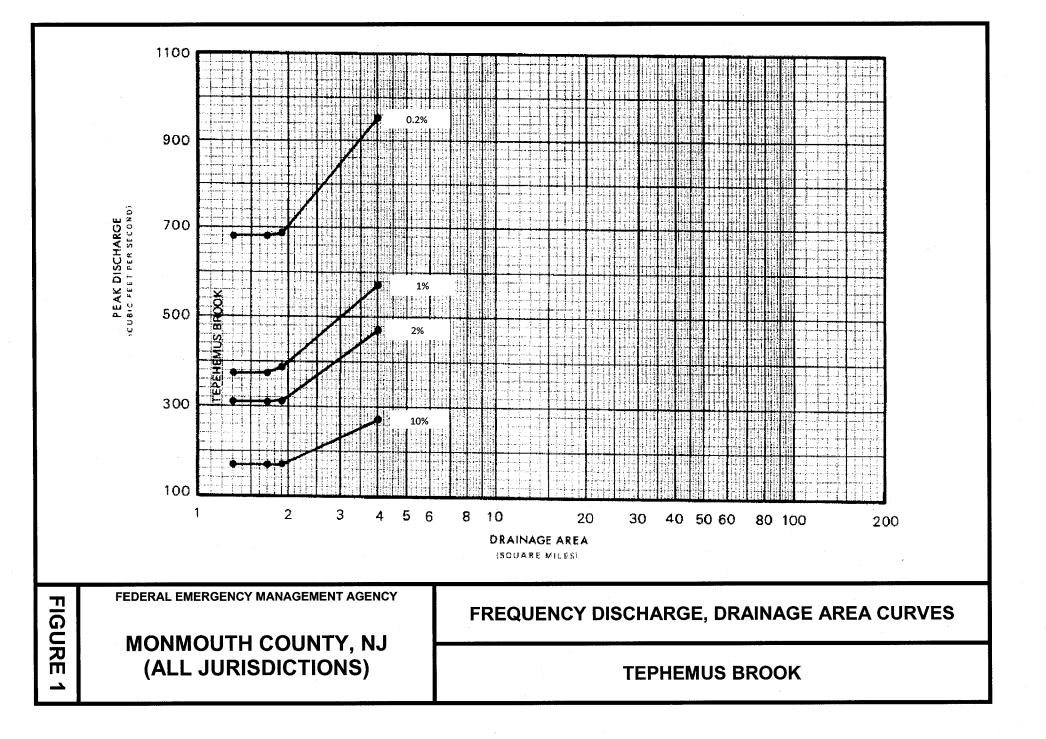


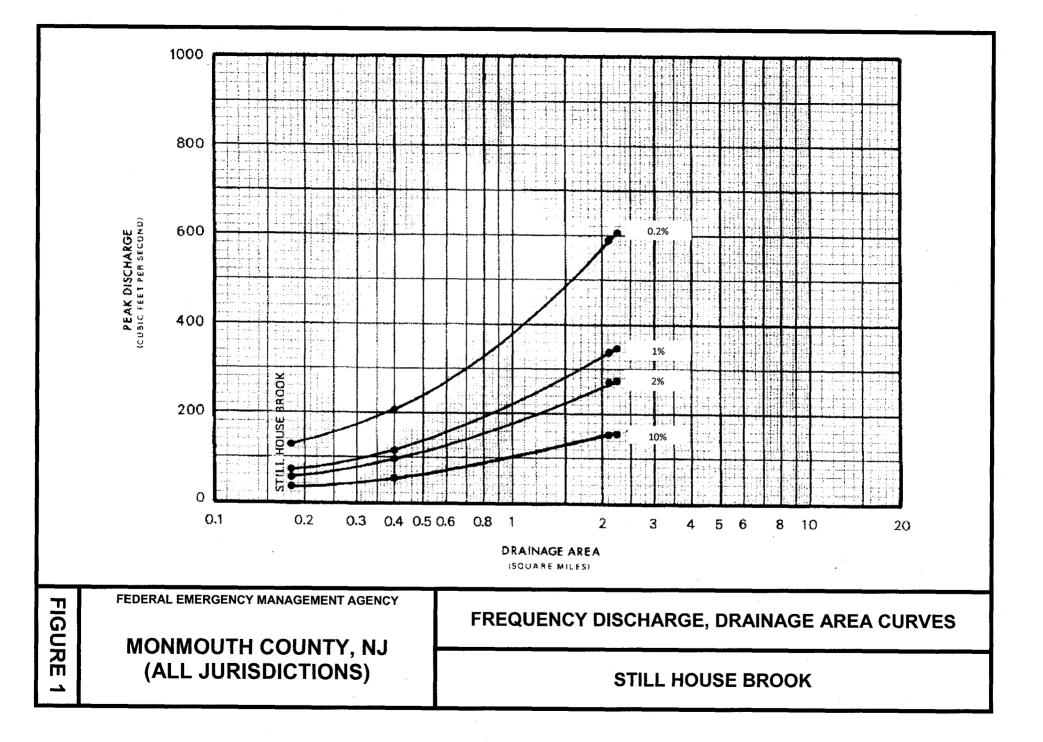


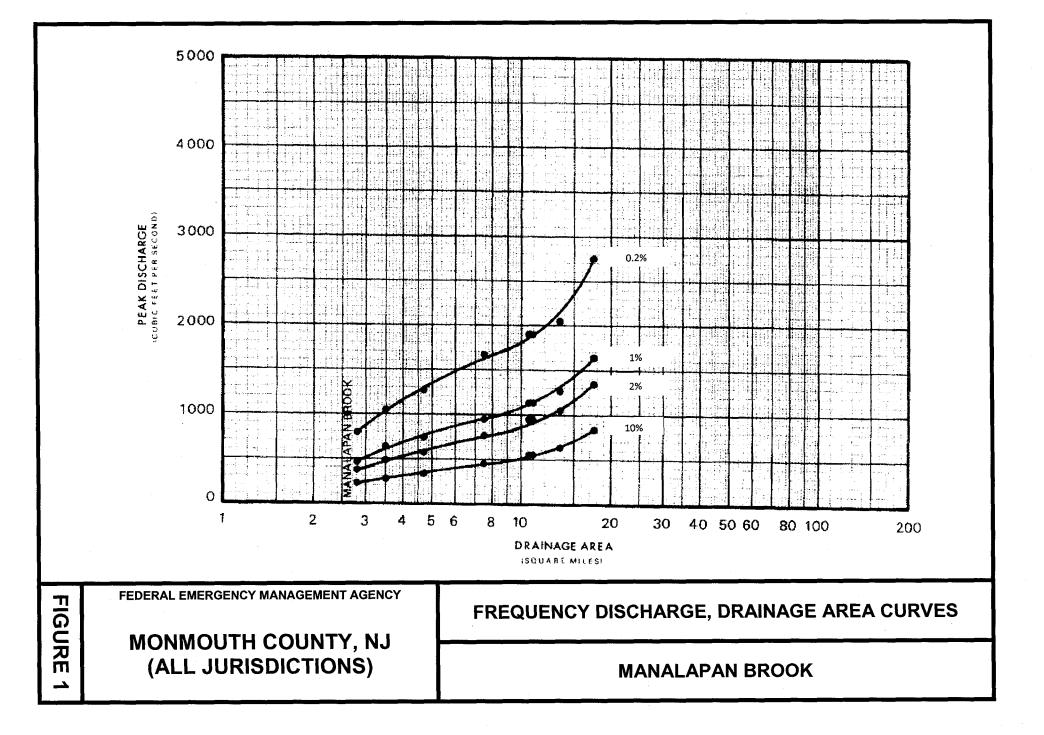


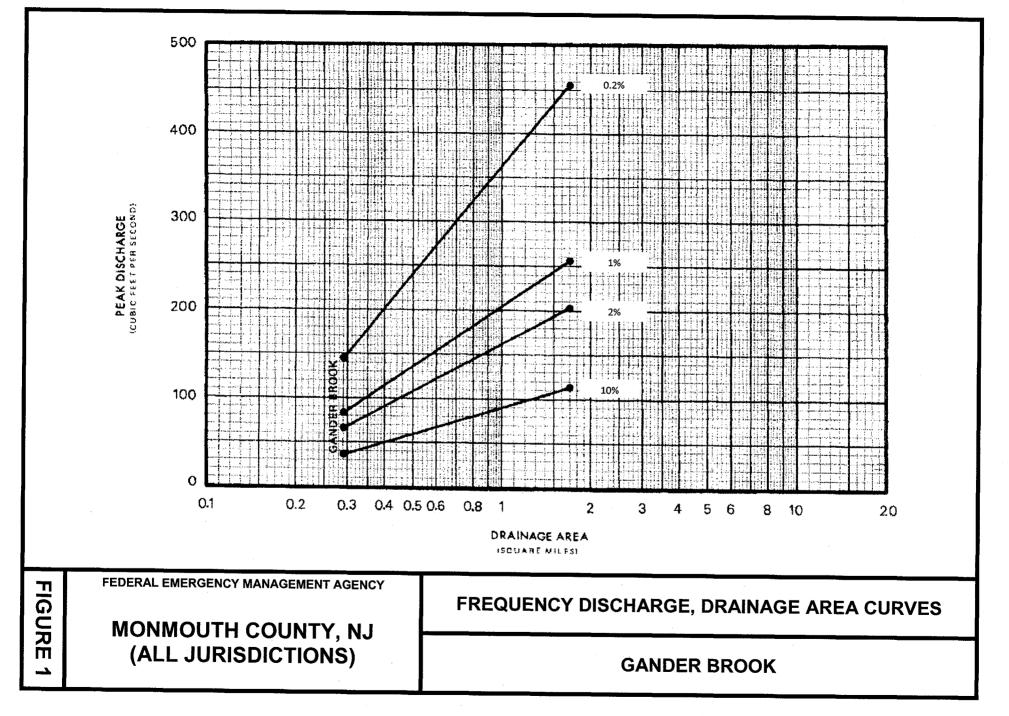


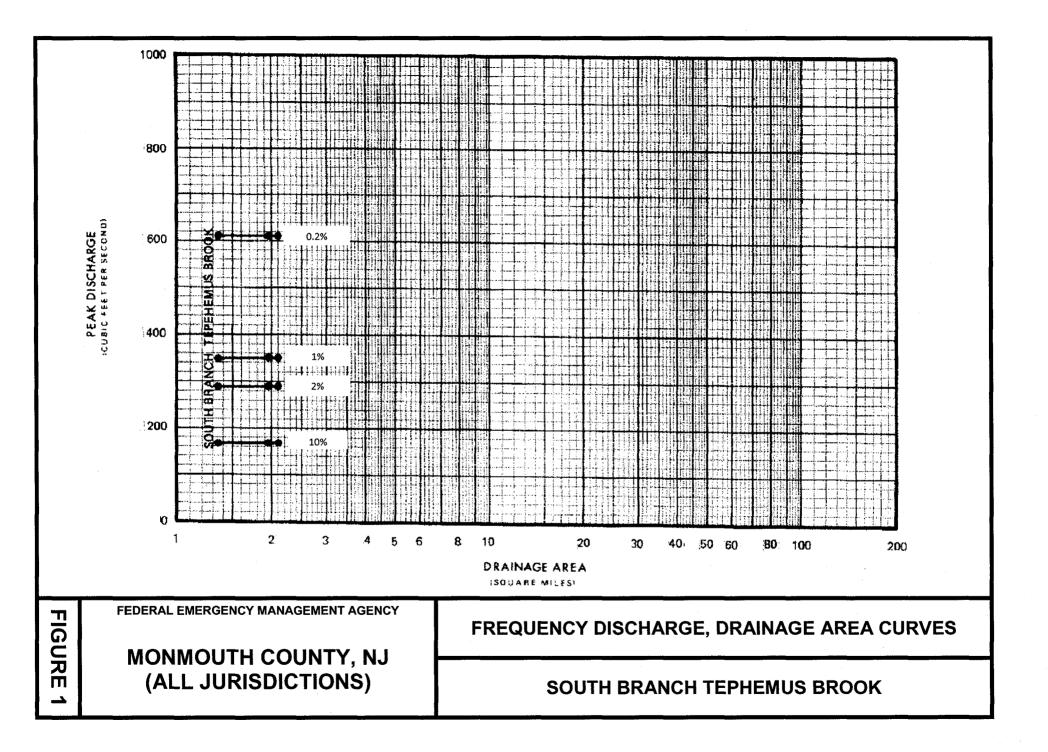


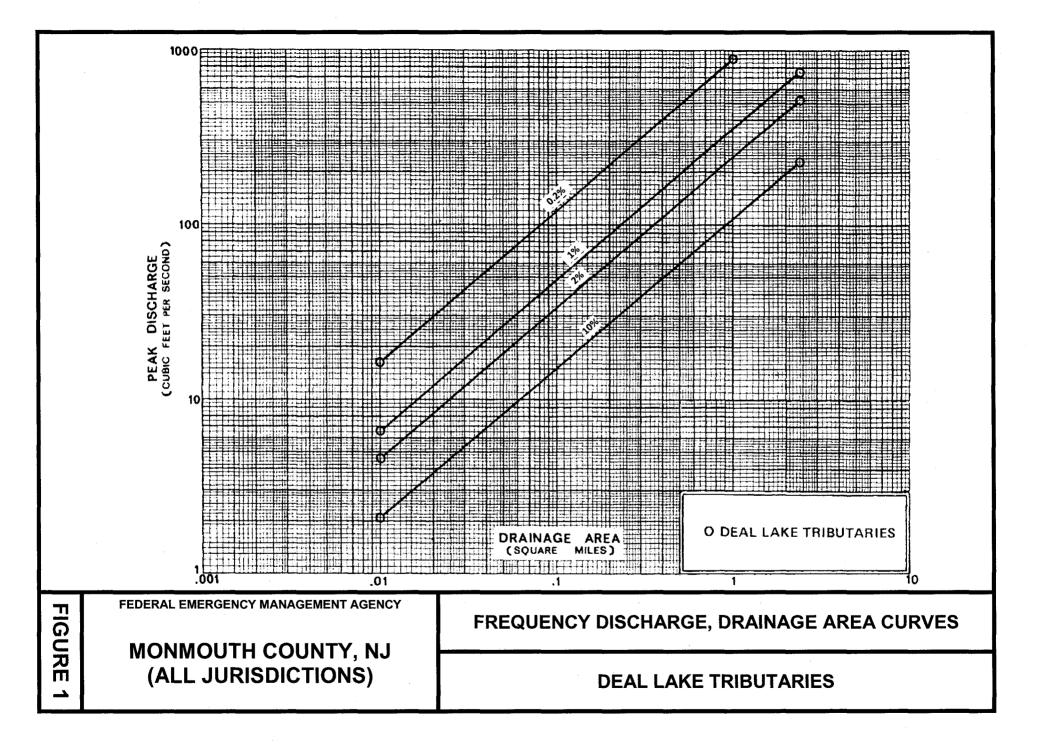


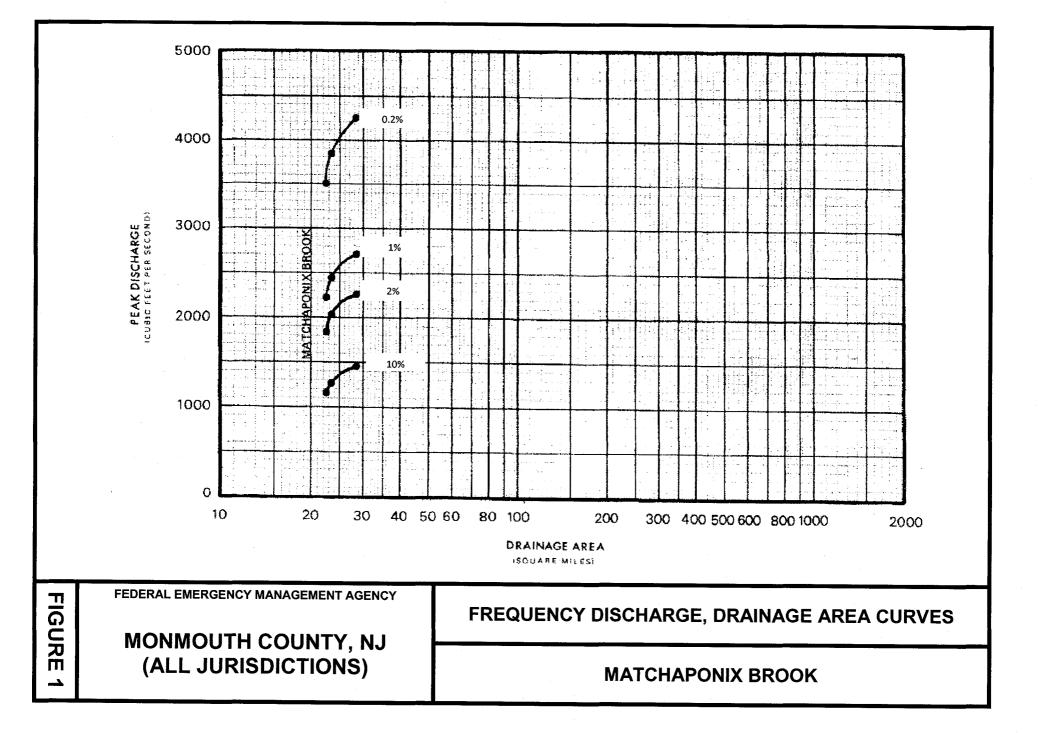


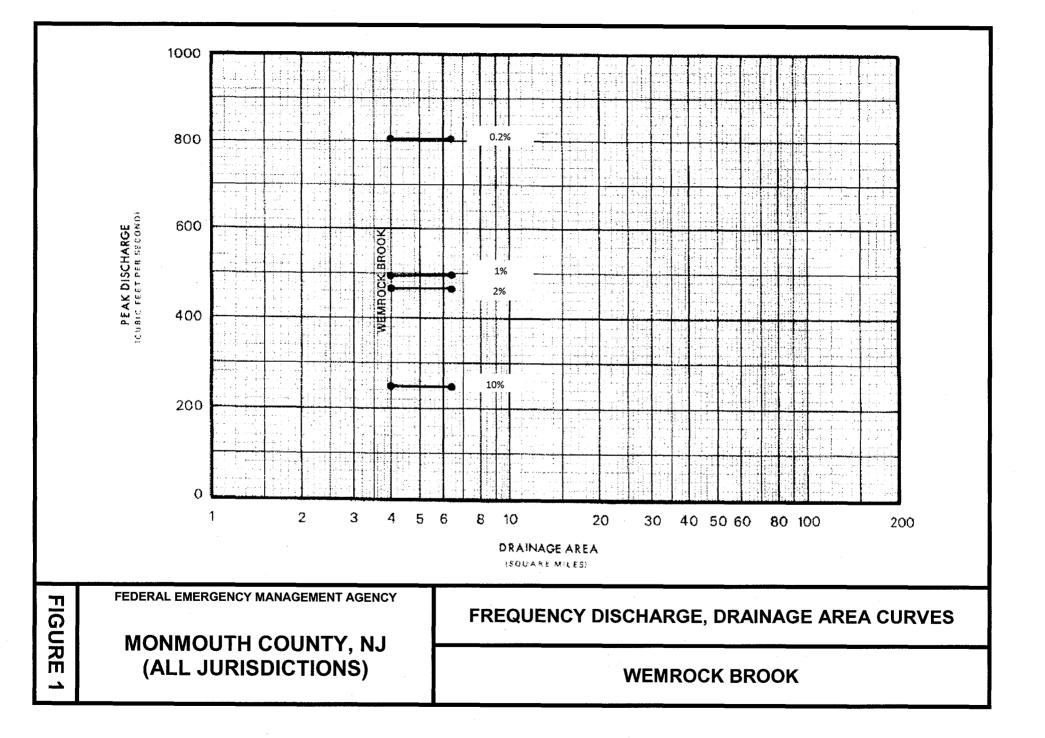


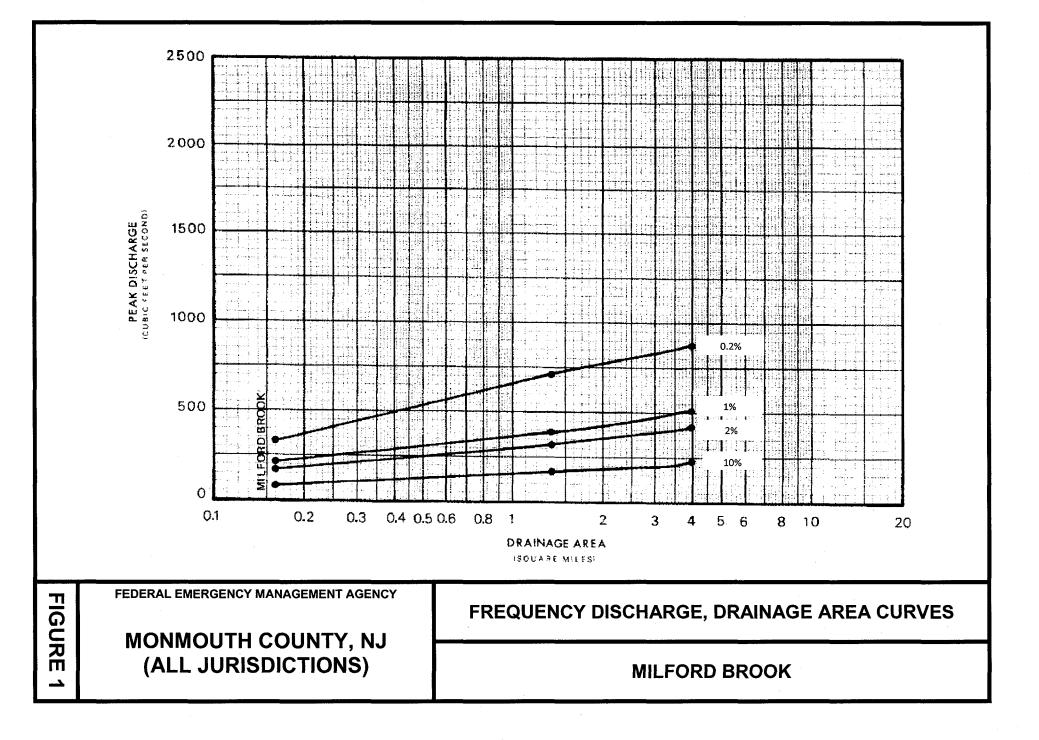


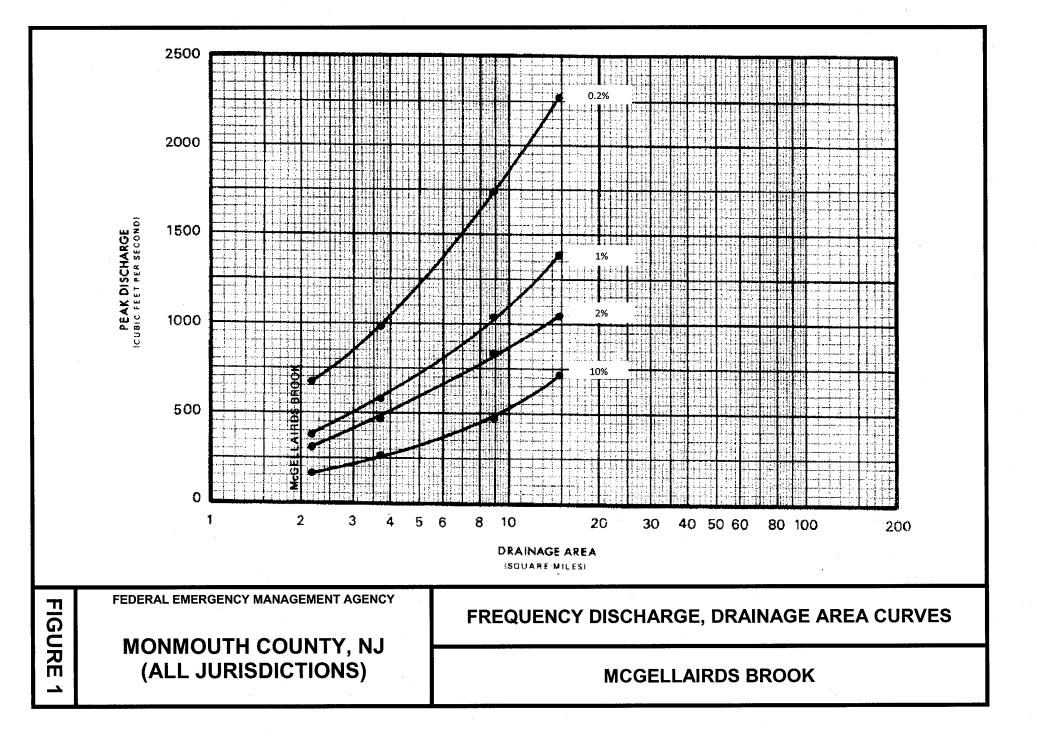


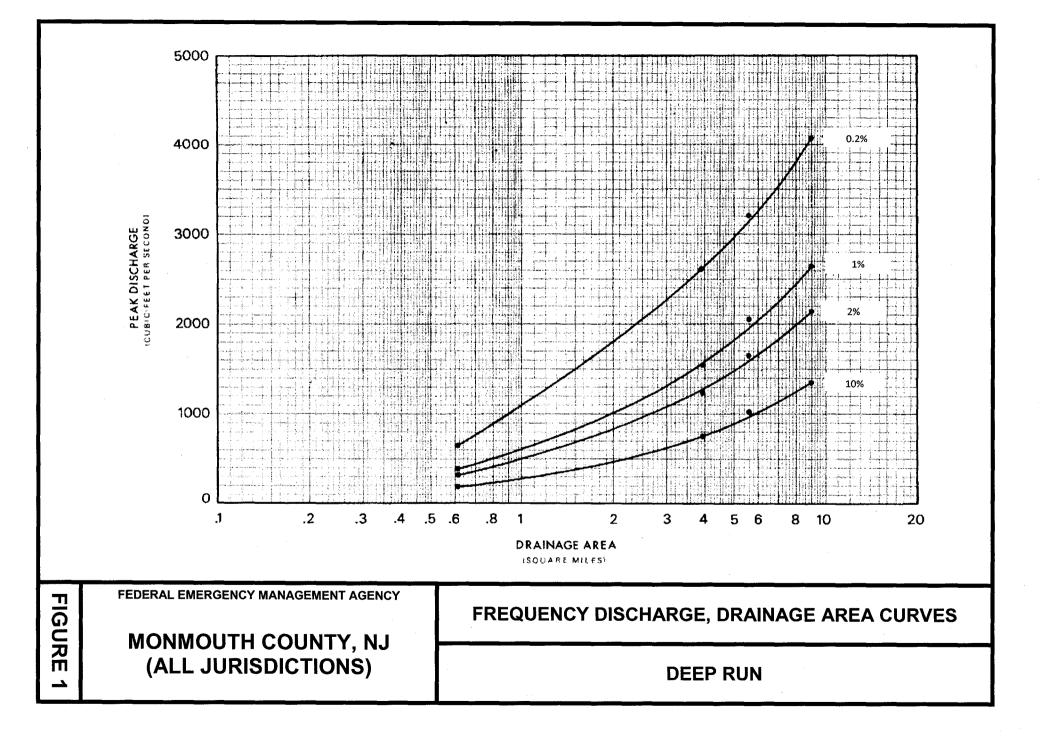


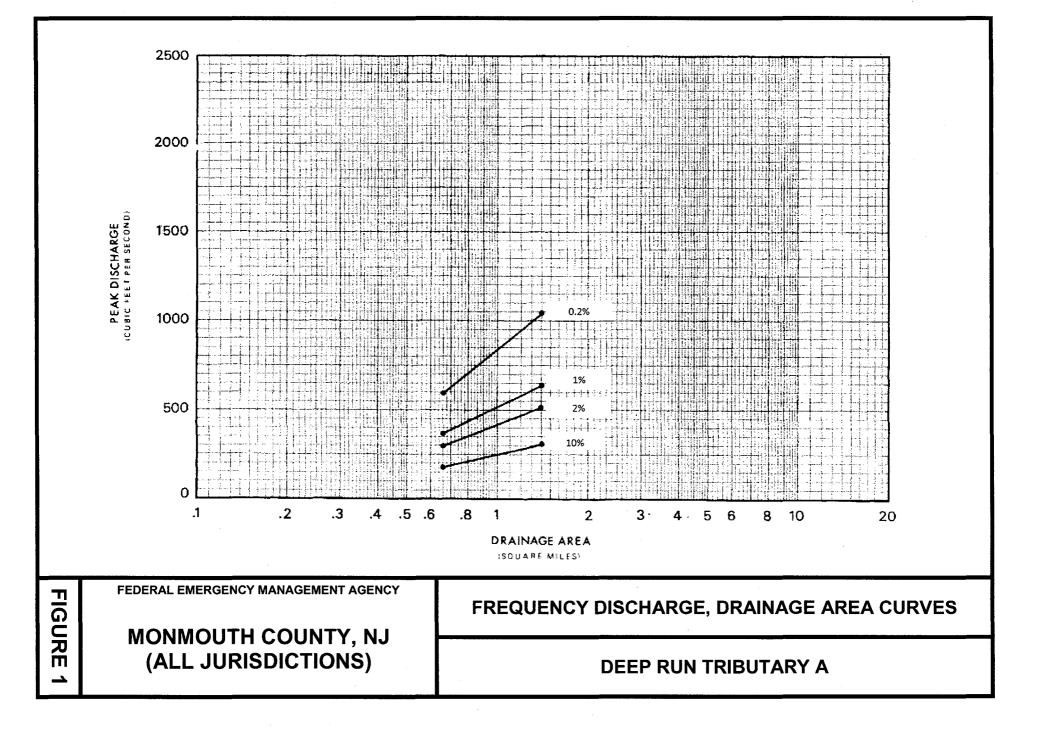


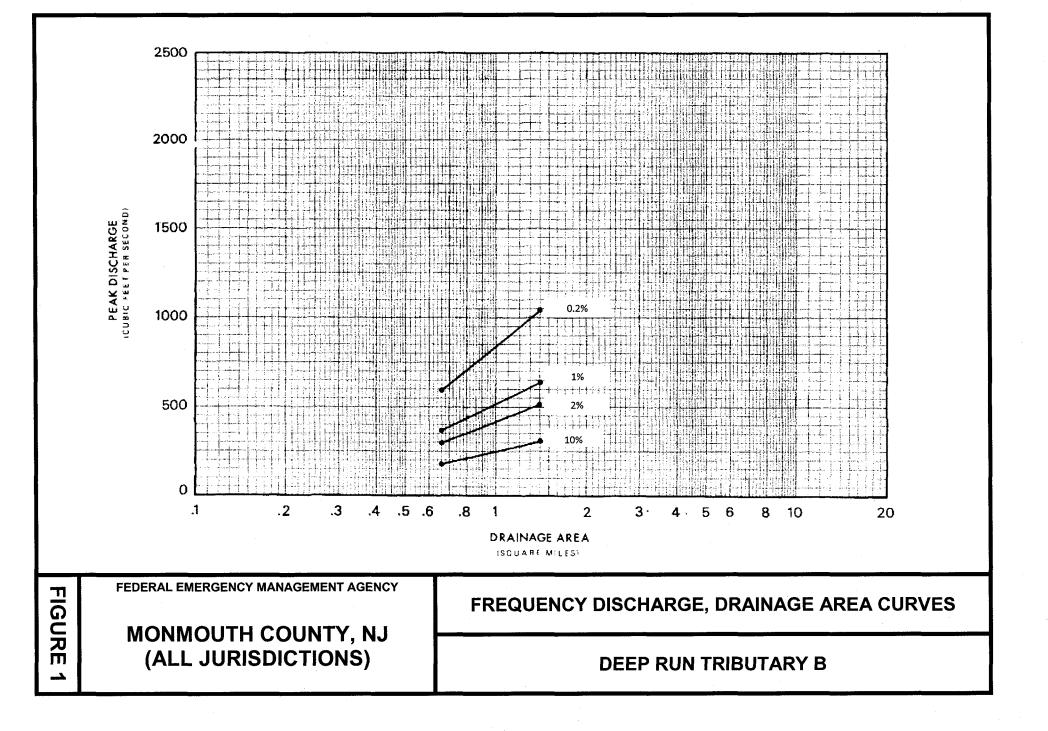


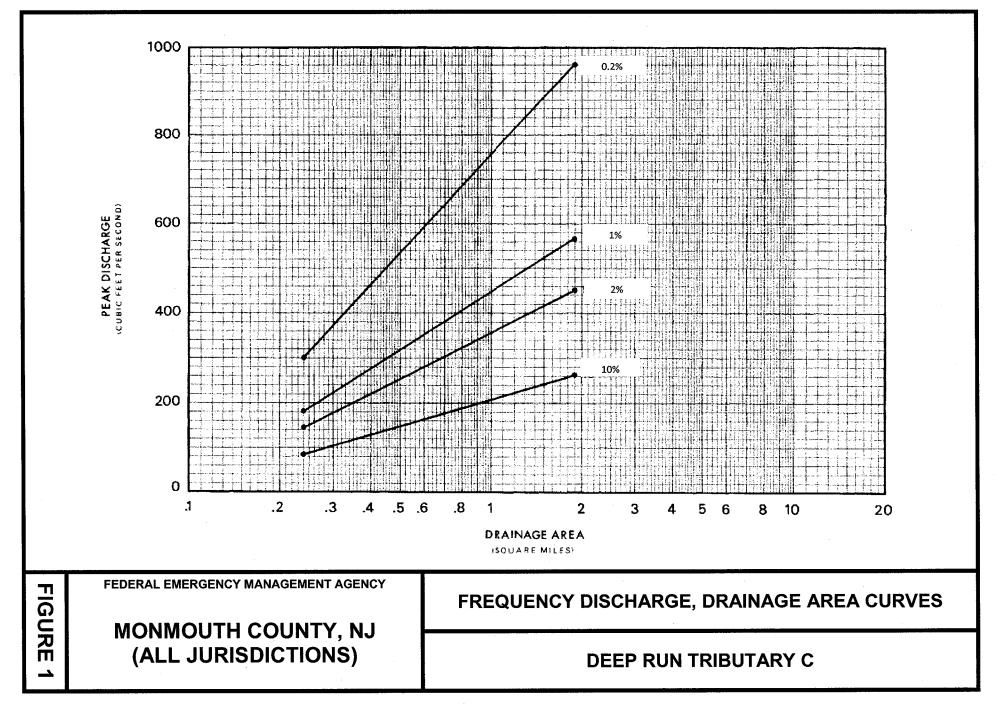




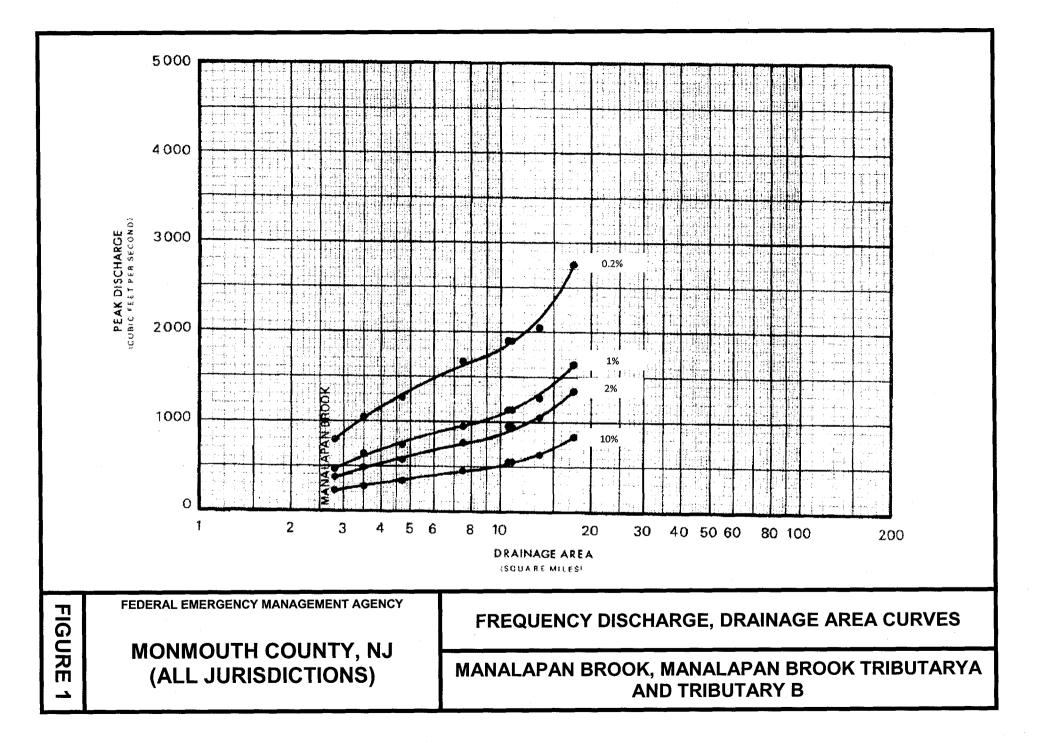








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The stillwater elevations have been determined for the 10-, 2-, 1-, and 0.2-percent annual chance floods for the flooding sources studied by detailed methods and are summarized in Table 6, "Summary of Stillwater Elevations."

TABLE 6 - SUMMARY OF STILLWATER ELEVATIONS

FLOODING SOURCE AND LOCATION	STILLWATER ELEVATION 10-PERCENT 2-PERCENT 1-PERCENT			et NAVD) 0.2-PERCENT
PARKERS CREEK				
Shoreline within Borough of Shrewsbury	5.1	6.9	7.9	12.4
PERRINEVILLE LAKE Entire shoreline	163.9	165.1	165.7	167.3

3.2 Riverine Hydraulic Analyses

Analyses of the hydraulic characteristics of flooding from the source studied were carried out to provide estimates of the elevations of floods of the selected recurrence intervals. Users should be aware that flood elevations shown on the FIRM represent rounded whole-foot elevations and may not exactly reflect the elevations shown on the Flood Profiles or in the Floodway Data Tables in the FIS report. For construction and/or floodplain management purposes, users are encouraged to use the flood elevation data presented in this FIS in conjunction with the data shown on the FIRM.

Precountywide Analyses

Each incorporated community within, and the unincorporated areas of, Monmouth County, with the exceptions of the Borough of Freehold, Borough of Lake Como and Borough of Neptune City, has a previously printed FIS report. The hydraulic analyses described in those reports have been compiled and are summarized below.

For streams studied by detailed methods, water-surface elevations of floods of the selected recurrence intervals were predominantly computed through use of the USACE HEC-2 step-backwater program (USACE, 1976; 1974). Cross sections for the backwater analyses of the streams studied in detail were field-surveyed and located at close intervals above and below bridges and culverts, in order to compute the significant backwater effects of these structures in highly urbanized areas.

Water surface elevations of floods of the selected recurrence intervals were calculated by a flood/rainfall routing analysis for Deal Lake. Indian Run, in the Borough of Allentown, was studied using the SCS WSP-2 program (U.S. Department of Agriculture, 1976).

In the Township of Aberdeen, starting water surface elevations for Matawan Creek and Whale Creek were derived from the mean high tide. Starting water surface elevations were determined using the slope/area method for Mohingson Brook, and were derived from the FIS for the Borough of Matawan (FEMA, 1981) for Gravelly Brook.

In the Borough of Allentown, starting water surface elevations for Doctors Creek were taken from the FIS for the Township of Hamilton, Mercer County, New Jersey (FEMA, 1984). For Indian Run, starting water surface elevations were calculated using the slope/area method.

In the Borough of Brielle, starting water surface elevations for Roberts Swamp Brook (Downstream Reach) were obtained from the FIS for the Borough of Manasquan (U.S. Department of Housing and Urban Development, 1971).

In the Township of Colts Neck, starting water surface elevations were obtained from the FIS for the township of Middletown (FEMA, 1983) for Barren Neck Creek, Big Brook (Downstream Reach), Willow Brook and Yellow Brook. Starting elevations for Pine Brook 1 were obtained from the FIS for the Borough of Tinton Falls (FEMA, 1981). Starting elevations for Hockhockson Brook were derived from the Pine Brook 1 profiles, while starting water surface elevations for Marl Brook were derived from the Mine Brook Profiles. Starting water surface elevations for Mine Brook and Miry Bog Brook were obtained from the Yellow Brook profiles.

In the Borough of Deal, starting water surface elevations for Poplar Brook were taken from known tidal elevations.

In the Borough of Eatontown, starting water surface elevations for Wampum Brook were derived from a discharge rating curve. Starting water surface elevations for Parkers Creek were obtained from the FIS for the Borough of Oceanport (U.S. Department of Housing and Urban Development, 1976), and starting elevations for Whale Pond Brook were obtained from the FIS for the Borough of West Long Branch (FEMA, 1981).

In the Borough of Englishtown, starting water surface elevations for McGellairds Brook and Weamaconk Creek were obtained from the FIS for the Township of Manalapan (U.S. Department of Housing and Urban Development, 1977). Starting elevations for Weamaconk Creek Tributary were derived from the profiles for Weamaconk Creek.

In the Township of Freehold, starting water surface elevations for Applegates Creek, Burkes Creek, Debois Creek, Debois Creek Tributary, Manasquan River Tributary A, Manasquan River Tributary B, Manasquan River Tributary C, Tepehemus Brook South Branch, Yellow Brook and Yellow Brook Tributary were calculated using the slope/area method. Starting elevations for Manasquan River were obtained from the FIS for the Township of Howell (FEMA, 1982). Starting elevations for McGellairds Brook, Weamaconk Creek and Wemrock

Brook were obtained from the FIS for the Township of Manalapan (U.S Department of Housing and Urban Development, 1977).

In the Township of Hazlet, starting water surface elevations for East Creek and Flat Creek were calculated using the slope/area method. Starting elevations for Monascunk Creek were derived from the Flat Creek profiles. Starting elevations for Waackaack Creek were obtained from the FIS for the Borough of Keansburg (FEMA, 1982).

In the Township of Holmdel, starting water surface elevations for Waackaack Creek were obtained from the FIS for the Township of Hazlet (FEMA, 1982), and starting elevations for Mahoras Brook were derived from the Waackaack Creek profiles. Starting water surface elevations for Willow Brook were obtained from the FISs for the Townships of Colts Neck and Marlboro (FEMA, 1981 & 1982), and the starting elevations for Ramanessin Brook and Willow Brook East Branch were derived from the Willow Brook profiles.

In the Township of Howell, starting water surface elevations for Ardena Brook, Bannen Meadow Brook, Gravelly Run, Groundhog Brook, Haystack Brook, Long Brook and Manasquan River were calculated using the slope/area method. Starting elevations for Metedeconk River North Branch were obtained from the FIS for the Township of Lakewood, Ocean County, New Jersey (FEMA, 1976). Starting elevations for Polypod Brook were derived from the Groundhog Brook profiles.

In the Borough of Keansburg, starting water surface elevations for Waackaack Creek were taken from the Spring High Tide for Raritan Bay.

In the Borough of Matawan, starting water surface elevations for Gravelly Brook and Matawan Creek were calculated using spillway rating curves.

In the Township of Middletown, starting water surface elevations for Claypit Creek, McClees Creek, Poricy Brook and Swimming River were obtained from the Mean Low Tide of Navesink River. Starting elevations for Comptons Creek were calculated using the slope/area method. Starting elevations for Mahoras Brook and Waackaack Creek were obtained from the FIS for the Township of Holmdel (FEMA, 1981). Starting elevations for Jumping Brook 1 and Nut Swamp Brook were derived from the profiles for Shadow Lake. Starting elevations for Town Brook were derived from the profiles for Comptons Creek.

In the Township of Millstone, starting water surface elevations for Rocky Brook (Upstream Reach) and Toms River were calculated using the slope/area method. Starting elevations for Millstone River were obtained from the FIS for the Township of Monroe, Middlesex County, New Jersey (FEMA, unpublished). Starting elevations for Rocky Brook (Downstream Reach) were derived from the profiles for Millstone River.

In the Township of Ocean, starting water surface elevations for Poplar Brook and Whale Pond Brook were obtained from the FIS for the Borough of Deal (FEMA, 1976). Starting elevations for Deal Tributary 1, Deal Tributary 2, Deal Tributary 3, Deal Tributary 3A, Deal Tributary 4, Deal Tributary 4A, Hog Swamp Brook and Hollow Brook were derived from the profiles for Deal Lake. Starting elevations for Poplar Brook Tributary 1, Poplar Brook Tributary 2 and Poplar Brook Tributary 3 were derived from the profiles for Poplar Brook.

In the Borough of Shrewsbury, starting water surface elevations for Parkers Creek were obtained from the FIS for the Boroughs of Oceanport and Little Silver (FEMA, 1976 & 1977). Starting elevations for Parkers Creek North Branch, were derived from the profiles for Parkers Creek.

In the Borough of Spring Lake Heights, starting water surface elevations for Wreck Pond Brook were obtained from the FIS for the Township of Wall (U.S. Department of Housing and Urban Development, 1976).

In the Borough of Tinton Falls, starting water surface elevations for Pine Brook 1 were calculated using the slope/area method. Starting elevations for Jumping Brook 2 were obtained from the FIS for the Borough of Neptune (U.S. Department of Housing and Urban Development, 1978). Starting elevations for Parkers Creek were obtained from the FIS for the Borough of Eatontown (FEMA, 1981). Starting elevations for Shark River were obtained from the FIS for the Township of Wall (U.S. Department of Housing and Urban Development, 1976). Starting elevations for Swimming River were obtained from the FIS for the Township of Middletown (FEMA, 1983).

In the Borough of Union Beach, starting water surface elevations for Waackaack Creek were obtained from the FIS for the Borough of Keansburg (FEMA, 1982).

In the Borough of Little Silver, water-surface profiles for floods of the selected recurrence intervals were computed using HEC-2. Starting water surface elevations for Little Silver Creek, Little Silver Tributary 1 and Little Silver Tributary 2 were obtained from a USACE tidal frequency study. Starting water surface elevations for the remaining streams were determined using the profiles of the downstream stream at the confluence.

Revised Analyses

Information on the methods used to determine peak discharge-frequency relationships for Manalapan Brook, restudied as part of this countywide FIS, is shown below.

The study area on Manalapan Brook extends from the downstream corporate limit of Township of Manalapan to Moonlight Court in the Township of Millstone. This stream contains 13 distinct bridges or other structures as it traverses a suburban area. Using aerial photographs, cross-section locations were identified for use in the modeling program. These locations were then surveyed to obtain accurate information on the river channel and bank configurations. The surveyors also obtained the necessary dimensions of crossing structures and overlying streets. Of the structures, ten (10) are bridges/culverts for roads or pedestrian paths. One surveyed bridge is a home-built stream crossing made of tree trunks and planks. Upon close inspection it was determined that this structure will likely not hold up in severe floods; therefore it was not included in the hydraulic model. The other two structures are small dams located at the downstream ends of Millhurst Pond and Bulks Lake. Neither reservoir has significant surcharge capacity nor serves as a flood control structure. Therefore, this study neglects any detention effects of these ponds.

Using the HEC-RAS 3.1.3 computer model (USACE, 2005) with RiverCAD software (Boss, 2007), a backwater hydraulic model of the river was developed. The study includes approximately 52,000 feet (9.8 miles) of river. The hydraulic analysis was performed using 95 river cross-sections.

Based on the information obtained in the survey and site inspections, several roughness coefficients (Manning's "n" values) are used for the overbank areas, as follows:

- 0.100 Trees: heavy stand of timber, few down trees.
- 0.053 Development areas (Sub-Urban)
- 0.050 Brush and heavy weeds
- 0.035 Pasture (native Grass)

The bottom of the channel varies greatly in elevation, from 66 feet in the northern portion to 180 feet in the southern portion. The channel has small stones and some small pools. Banks are mildly sloped in the northern portion and fairly steep in the southern portion and they are lined with trees and brush along the channel. Based on site inspections, Manning's "n" values of 0.045 is assigned to the channel.

Water surface elevations for design floods at the selected cross sections were computed through use of the HEC-RAS 3.1.3 computer program. The downstream beginning water surface elevations are taken from the profiles for Manalapan Brook found in the FIS for Monroe Township, Middlesex County, New Jersey.

Flood profiles were drawn showing computed water surface elevations for floods of the selected recurrence intervals.

Channel roughness factors (Manning's "n") for these hydraulic computations were assigned on the basis of field inspection of floodplain areas and the study of past floods. Channel roughness factors for streams studied by detailed methods are listed in Table 7, "Manning's "n" Values."

TABLE 7 – MANNING'S "n" VALUES

Stream	Channel "n"	Overbank "n"
Applegates Creek	0.030-0.035	0.060-0.080
Ardena Brook	0.030-0.040	0.060-0.070
Bannen Meadow Brook	0.025-0.035	0.060-0.080
Barclay Brook	*	*
Barren Neck Creek	0.020-0.035	0.055
Betty Brook	*	*
Big Brook	0.025-0.035	0.050-0.055
Big Brook Tributary H	*	*
Burkes Creek	0.025-0.040	0.050-0.100
Claypit Creek	0.016-0.050	0.080
Comptons Creek	0.018-0.060	0.080-0.100
Cranberry Brook	*	*
Deal Tributary 1	0.035-0.040	0.040-0.060
Deal Tributary 2	0.035-0.040	0.040-0.060
Deal Tributary 3	0.035-0.040	0.040-0.060
Deal Tributary 3A	0.035-0.040	0.040-0.060
Deal Tributary 4	0.035-0.040	0.040-0.060
Deal Tributary 4A	0.035-0.040	0.040-0.060
Debois Creek	0.025-0.035	0.040-0.080
Debois Creek Tributary	0.025-0.040	0.060-0.080
Deep Run	*	*
Deep Run Tributary A	*	*
Deep Run Tributary B	*	*
Deep Run Tributary C	*	*
Doctors Creek	0.030-0.050	0.050-0.080
East Creek	0.050-0.100	0.060-0.140
Flat Creek	0.014-0.060	0.070-0.100
Gander Brook	*	*
Gravelly Brook	*	*
Gravelly Run	0.018-0.040	0.040-0.100
Groundhog Brook	0.030-0.050	0.045-0.060
Hannabrand Brook	*	*
Haystack Brook	0.025-0.040	0.050-0.080
Heroys Pond Creek	*	*
Hockhockson Brook	0.025-0.035	0.055
Hog Swamp Brook	0.035-0.040	0.040-0.060
Hollow Brook	0.035-0.040	0.040-0.060
Indian Run	0.060-0.070	0.100-0.170
Judas Creek (Upstream Reach)	*	*
Jumping Brook 1	0.012-0.070	0.055-0.100
Jumping Brook 2	0.015-0.035	0.055
Little Silver Creek	*	*

*Data not available

TABLE 7 - MANNING'S "n" VALUES - continued

	<u>k "n"</u>
Little Silver Creek Tributary A * *	
Little Silver Creek Tributary I * *	
Little Silver Creek Tributary II * *	
Little Silver Creek Tributary II-A * *	
Little Silver Creek Tributary II-B * *	
Long Brook 0.025-0.045 0.050-0	.080
Mac's Brook * *	
Mahoras Brook 0.013-0.070 0.070-0	.100
Manalapan Brook 0.035-0.053 0.035-0	
Manalapan Brook Tributary A * *	
Manalapan Brook Tributary B * *	
Manasquan River 0.030 0.060-0	.150
Manasquan River Tributary A 0.030-0.045 0.050-0	.150
Manasquan River Tributary B 0.030-0.040 0.050-0	.100
Manasquan River Tributary C 0.030-0.045 0.050-0	.150
Marl Brook 0.035 0.05	5
Matawan Creek 0.018-0.040 0.040-0	.100
Matchaponix Brook * *	
McClees Creek 0.018-0.040 0.050-0	.100
McGellairds Brook 0.030-0.080 0.040-0	.100
Metedeconk River North Branch 0.045-0.060 0.050-0	.070
Milford Brook * *	
Millstone River 0.045 0.10	0
Mine Brook 0.013-0.035 0.05	5
Miry Bog Brook 0.015-0.035 0.045-0	.055
Mohingson Brook 0.018-0.040 0.060-0	.100
Monascunk Creek 0.013-0.070 0.060-0	.100
Musquash Brook * *	
Navesink River * *	
Nut Swamp Brook 0.012-0.060 0.070-0	
Parkers Creek 0.015-0.040 0.050-0	
Parkers Creek, North Branch 0.015-0.040 0.050-0	.065
Pine Brook 1 0.015-0.040 0.05	5
Pine Brook 2 * *	
Pine Brook Tributary C * *	
Polypod Brook 0.030 0.06	0
Poly Pond Brook (Wall Township) * *	
Poly Pond Brook (Spring Lake Heights) * *	
Poplar Brook 0.035-0.050 0.040-0	
Poplar Brook Tributary 1 0.035-0.040 0.040-0	
Poplar Brook Tributary 2 0.035-0.040 0.040-0	
Poplar Brook Tributary 3 0.035-0.040 0.040-0	.060

*Data not available

TABLE 7 – MANNING'S "n" VALUES

Stream	Channel "n"	Overbank "n"
Poricy Brook	0.014-0.040	0.060-0.150
Ramanessin Brook	0.035-0.040	0.055-0.065
Roberts Swamp Brook	*	*
Rocky Brook (Downstream Reach)	0.035-0.045	0.070-0.170
Rocky Brook (Upstream Reach)	0.030-0.040	0.060-0.090
Shark River	0.035	0.035
Shark River Tributary D	*	*
Shark River Tributary E	*	*
Shrewsberry River	*	*
Still House Brook	*	*
Swimming River	0.016-0.040	0.070-0.090
Tepehemus Brook	*	*
Tepehemus Brook South Branch	0.035-0.048	0.040-0.120
Toms River	0.035-0.050	0.060-0.110
Town Brook	0.013-0.060	0.050-0.090
Town Neck Creek	*	*
Turtle Mill Brook	*	*
Upper Yellow Brook	*	*
Waackaack Creek	0.014-0.080	0.013-0.100
Wampum Brook	0.015-0.040	0.050-0.065
Watson Creek	*	*
Weamaconk Creek	0.040-0.050	0.045-0.120
Weamaconk Creek Tributary	0.013-0.035	0.060
Wells Brook	*	*
Wemrock Brook	0.045-0.052	0.090
Whale Creek	0.018-0.040	0.060-0.100
Whale Pond Brook	0.015-0.075	0.030-0.070
Whale Pond Tributary 1	*	*
Whale Pond Tributary 2	*	*
Willow Brook	0.015-0.040	0.050-0.065
Willow Brook Tributary F	*	*
Willow Brook Tributary G	*	*
Willow Brook, East Branch	0.040-0.045	0.070
Wreck Pond East Branch	*	*
Wreck Pond Brook	*	*
Yellow Brook	0.015-0.040	0.045-0.080
Yellow Brook 2	0.030-0.040	0.050-0.080
Yellow Brook Tributary 2	0.030	0.040-0.060
Yellow Brook Tributary K	*	*
Yellow Brook Tributary L	*	*

*Data not available

Locations of selected cross sections used in the hydraulic analyses are shown on the Flood Profiles (Exhibit 1). For stream segments for which a floodway was computed (Section 4.2), selected cross sections are also shown on the FIRM (Exhibit 2).

All elevations are referenced to the North American Vertical Datum of 1988 (NAVD 88).

All qualifying bench marks within a given jurisdiction that are cataloged by the National Geodetic Survey (NGS) and entered into the National Spatial Reference System (NSRS) as First or Second Order Vertical and have a vertical stability classification of A, B, or C are shown and labeled on the FIRM with their 6-character NSRS Permanent Identifier.

Bench marks cataloged by the NGS and entered into the NSRS vary widely in vertical stability classification. NSRS vertical stability classifications are as follows:

- Stability A: Monuments of the most reliable nature, expected to hold position/elevation well (e.g., mounted in bedrock)
- Stability B: Monuments which generally hold their position/elevation well (e.g., concrete bridge abutment)
- Stability C: Monuments which may be affected by surface ground movements (e.g., concrete monument below frost line)
- Stability D: Mark of questionable or unknown vertical stability (e.g., concrete monument above frost line, or steel witness post)

In addition to NSRS bench marks, the FIRM may also show vertical control monuments established by a local jurisdiction; these monuments will be shown on the FIRM with the appropriate designations. Local monuments will only be placed on the FIRM if the community has requested that they be included, and if the monuments meet the aforementioned NSRS inclusion criteria.

To obtain current elevation, description, and/or location information for bench marks shown on the FIRM for this jurisdiction, please contact the Information Services Branch of the NGS at (301) 713-3242, or visit their Web site at www.ngs.noaa.gov.

It is important to note that temporary vertical monuments are often established during the preparation of a flood hazard analysis for the purpose of establishing local vertical control. Although these monuments are not shown on the FIRM, they may be found in the Technical Support Data Notebook associated with this FIS and FIRM. Interested individuals may contact FEMA to access this data.

3.3 Coastal Hydrologic Analyses

For all other municipalities bordering the Atlantic Ocean, and for the Raritan Bay, Navesink River and Shrewsbury River, the determination of coastal inundation caused by the passage of a hurricane storm surge was approached by the joint probability method (U.S. Department of Commerce, 1970). The storm populations were described by probability distributions of five parameters which influence surge heights. These were central pressure depression (which measures the intensity of the storm), radius to maximum winds, forward speed of the storm, shoreline crossing point and crossing angle. These characteristics were described statistically, based upon an analysis of observed storms in the vicinity of New Jersey. Several primary sources of data were researched for this information (U.S. Department of Commerce: 1975, 1970, 1965, 1957; National Hurricane Research Project No. 5, 1957). The storm parameters adopted for New Jersey are shown in Table 8, "Parameter Values for Surge Evaluation."

A numerical hydrodynamic model of the region was used to simulate the coastal surge generated by any chosen storm (any combination of the five storm parameters defined previously). Performing such simulations for a large number of storms – each of known total probability – permits the establishment of frequency distribution of surge heights at a coastal location. The astronomic tide for the region is then statistically combined with the computed storm surge to yield recurrence intervals of total water level. This procedure is detailed in the Coastal Flooding Handbook (Tetra Tech, 1977).

Wave heights and corresponding wave crest elevations were determined using the National Academy of Sciences (NAS) methodology (1977). This methodology considers maximum conditions associated with the 1% annual chance flood, and uses transects which are oriented perpendicular to the mean sea level shoreline to deduce wave crest elevations. The stillwater elevations and the maximum wave crest elevations of the selected recurrence intervals are shown in Table 9, "Summary of Stillwater Elevations."

CENTRAL PRESSURE (INCHES HG)	27.39	27.68	27.97	28.26	28.55	28.84	29.12	29.40	29.70
ASSIGNED PROBABILITIES [:] STORMS OVER LAND STORMS OVER SEA	0.00 0.00	0.00	0.000 0.055	0.000	0.000 0.145	0.00 0.15	0.80	0.125 0.125	0.075 0.075
STORM RADIUS (NAUTICAL MILES [NM])					37.5				
ASSIGNED PROBABILITY					1.0				
FORWARD SPEED (KNOTS)	2	20		30 40					
ASSIGNED PROBABILITIES:					·				
STORMS OVER LAND	0.	76			0.15		0.	09	
STORMS OVER SEA	0.	56	<u> </u>	·	0.44		0.	00	
DIRECTION (DEGREE)			-11				20		
ASSIGNED PROBABILITY:				<u> </u>		<u> </u>			<u> </u>
STORMS OVER LAND			0.32				0.68		
STORMS OVER SEA			0.06		·····		0.94		
SPATIAL OCCURRENCE RATE	1.22	X 10 ⁻³		Sto	orms Over L	and			
STORMS/NM YEAR		X 10 ⁻³		Sto	orms Over S	Sea			
	· · · · · · · · · · · · · · · · · · ·				<u> </u>		<u> </u>		

 Image: Second system
 Federal EMERGENCY MANAGEMENT AGENCY

 Image: Second system
 MONMOUTH COUNTY, NJ

 Image: Second system
 PARAMETER VALUES FOR SURGE ELEVATIONS

 Image: Second system
 PARAMETER VALUES FOR SURGE ELEVATIONS

		STILLWATER	ELEVATION (fe	et NAVD)
FLOODING SOURCE AND LOCATION	10-PERCENT	2-PERCENT	1-PERCENT	0.2-PERCENT
ATLANTIC OCEAN				
Shoreline within Borough of	·	()		0.0
Allenhurst	5.5	6.8	7.4	9.0
Shoreline within City of Asbury Park	5.5	6.8	7.4 / 11.9*	9.0
Shoreline within Borough of Avon-by- the-Sea	5.5	6.8	7.4 / 11.9*	9.0
Shoreline within Borough of Bradley Beach	5.5	6.8	7.4 / 11.9*	9.0
Shoreline within Borough of Brielle	5.5	6.7	7.3 / 8.9*	8.8
Shoreline within Village of Loch Arbour	5.5	6.8	7.4	9.0
Shoreline within Borough of Deal	5.5	6.9	7.5	9.1
Shoreline within Township of Wall	6.0	7.9	8.9	12.9
NAVESINK RIVER	5.9	7.3	8.1	10.4
Shoreline within Borough of Fair Haven Shoreline within Township of Middletown	5.9	7.3	8.1 / 10.9*	10.4
2	5.9 5.9	7.3	8.1	10.4
Shoreline within Borough of Red Bank	5.9	7.5	8.1	10.4
RARITAN BAY				
Shoreline within Borough of Keansburg	6.1	9.1	10.5 / 16.9*	13.4
Shoreline within Township of Middletown	6.1	9.0	10.5 / 16.9*	12.9
Shoreline within Borough of Matawan	6.1	9.3	10.5	13.6
At confluence of Matawan Creek with	6.3	9.5	10.7 / 16.9*	13.8
Raritan Bay	()	0.0	109/100*	14.1
At western corporate limits of Township of Aberdeen	6.3	9.6	10.8 / 16.9*	14.1
Shoreline within Township of Hazlet	6.3	9.5	10.7	13.8
Shoreline within Borough of Keyport	6.3	9.5	10.7 / 15.9*	13.8
Shoreline within Borough of Union Beach	6.3	9.5	5.5	13.8
SANDY HOOK BAY				
Shoreline within Borough of Highlands	6.3	8.8	9.7	11.8
SHARK RIVER				
Shoreline within Township of Wall	6.0	7.9	8.9	12.9
SHREWSBURY RIVER				
Shoreline within Township of Middletown	5.9	7.3	8.1 / 10.9*	10.4

TABLE 9 - SUMMARY OF COASTAL STILLWATER ELEVATIONS

*Stillwater Elevation / Maximum Wave Crest Elevation

	STILLWATER ELEVATION (feet NAVD)			
FLOODING SOURCE AND LOCATION	10-PERCENT	2-PERCENT	1-PERCENT	0.2-PERCENT
SOUTH SHREWSBURY RIVER Shoreline within Borough of Oceanport	5.4	6.9	8.0	12.9
WRECK POND BROOK Shoreline within Borough of Spring Lake Heights	7.4	9.0	9.4	13.7

TABLE 9 - SUMMARY OF COASTAL STILLWATER ELEVATIONS - continued

*Stillwater Elevation / Maximum Wave Crest Elevation

3.4 Coastal Hydraulic Analyses

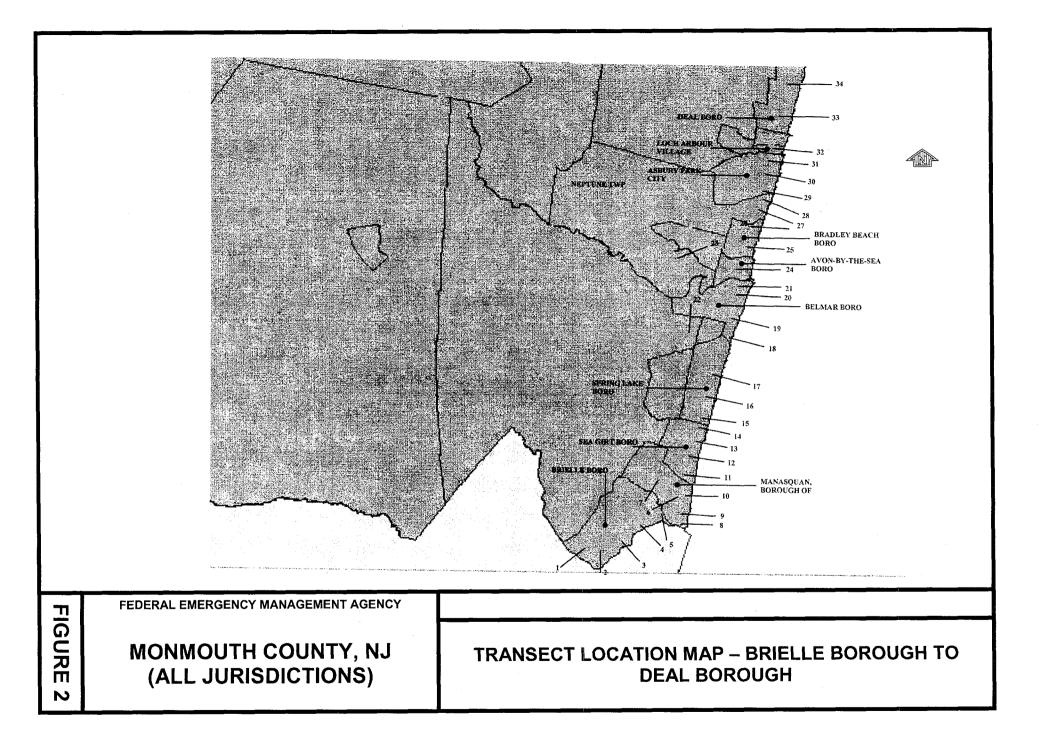
Areas of coastline subject to significant wave attack are referred to as coastal high hazard zones. The USACE has established the 3-foot breaking wave as the criterion for identifying the limit of coastal high hazard zones. The 3-foot wave has been determined as the minimum size wave capable of causing major damage to conventional wood frame and brick veneer structures.

The methodology for analyzing the effects of wave heights associated with coastal storm surge flooding is described in the NAS report (National Academy of Sciences, 1977). This method is based on three major concepts. First, depth-limited waves in the shallow water reach a maximum breaking height that is equal to 0.78 times the stillwater depth, and the wave crest is 70 percent of the total wave height above the stillwater level. Second, wave height may be diminished by the dissipation of energy due to the presence of obstructions such as sand dunes, dikes, sea walls, buildings and vegetation. The amount of energy dissipation is a function of the physical characteristics of the obstructions and is determined by procedures outlined in the <u>User's Manual for Wave Height Analysis</u> (FEMA, 1981). Third, wave height can be regenerated in open fetch areas due to the transfer of wind energy to the water. This added energy is related to fetch length and depth.

As of 1989, FEMA defines a "coastal high hazard area" as an area of special flood hazards extending from offshore to the inland limit of a primary frontal dune along an open coast or any other area subject to high velocity wave actions (i.e., wave heights greater than or equal to 3 feet) from storms or seismic sources. The "primary frontal dune" is defined as a continuous or nearly continuous mound or ridge of sand with relatively steep seaward and landward slopes immediately landward and adjacent to the beach and subject to erosion and overtopping from high tides and waves during major coastal storms. The inland limit of the primary frontal dune occurs at the point where there is a distinct change from relatively steep slope to a relatively mild slope.

Wave heights were computed along transects (cross-section lines) located along the coastal areas, as illustrated in Figure 3 in accordance with the <u>User's Manual</u> for <u>Wave Height Analysis</u> (FEMA, 1981). The transects were located perpendicular to the shoreline, representing sections of similar characteristics. Transects were spaced close together in areas of complex topography and dense development. In areas having more uniform characteristics, they were spaced at larger intervals. It was also necessary to locate transects in areas where unique flooding existed and in areas where computed wave heights varied significantly between adjacent transects.

Figure 2, "Transect Location Map," illustrates the locations of transects, and Table 10, "Transect Descriptions," provides a list of the transect locations and stillwater elevations, as well as initial wave crest elevations. In addition, Table 9 provides the 1-annual chance stillwater and maximum wave crest elevations for each transect in the County.



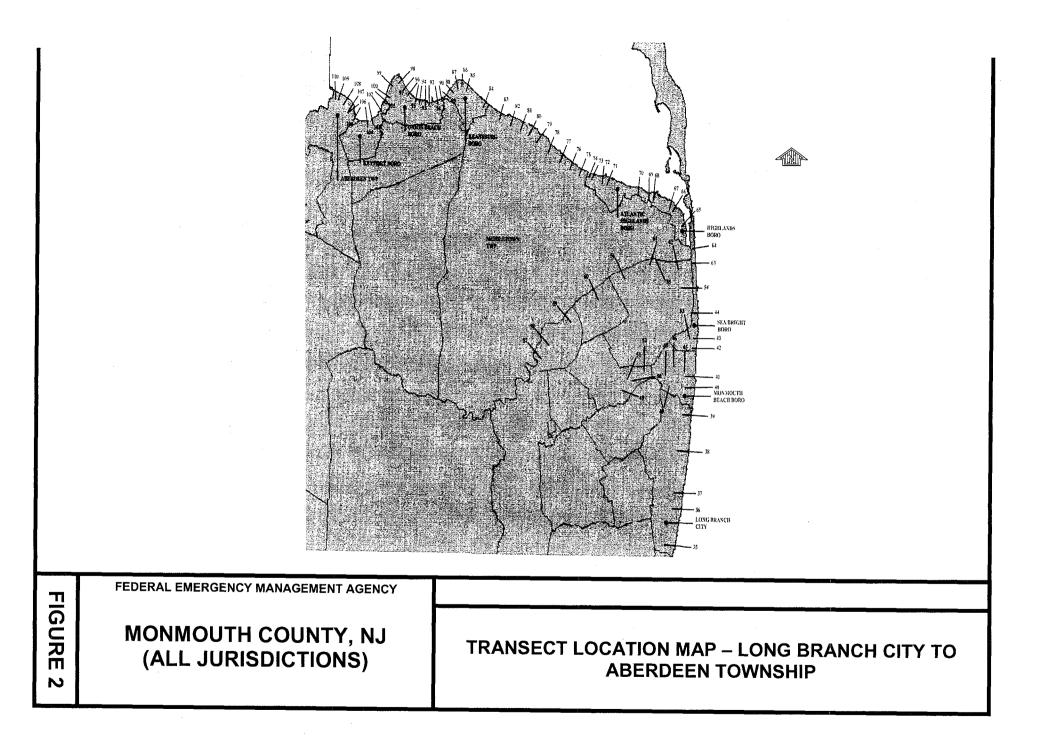


TABLE 10 – TRANSECT DESCRIPTIONS

		ELEVATION (feet NAV	
			MAXIMUM
TRANSECT	LOCATION	1-PERCENT	WAVE CREST ¹
1	*	7.3	8.9
2	*	7.3	8.9
3	*	7.3	8.9
4	*	7.3	8.9
5	*	7.3	8.9
6	*	7.3	8.9
7	*	7.3	8.9
8	From Whiting Avenue, extended, to the	7.3	11.9
0	southern corporate limits of the Borough	1.5	11.7
	of Manasquan		
9	From Brielle Road, extended, to Whiting	7.4	11.9
10	Avenue, extended	77 A	11.0
10	From Ocean Avenue, extended, to Brielle Road, extended	7.4	11.9
11	From northern corporate limits of the	7.4	11.9
	Borough of Manasquan to Ocean Avenue,		
	extended		
12	From Boston Boulevard, extended, to Sea	7.4	11.9
13	Girt Avenue, extended	7.4	11.9
15	From the Terrace, extended, to Boston Boulevard, extended	/.4	11.9
14	From the northern corporate limits of the	7.4	11.9
	Borough of Sea Girt to the Terrace,		
	extended		
15	From the southern corporate limits of the	7.4	11.9
	Borough of Spring Lake to Union Avenue, extended		
16	From Union Avenue, extended, to Morris	7.4	11.9
	Avenue, extended		
17	From Morris Avenue, extended, to Pitney	7.4	11.9
18	Avenue, extended From 18 th Avenue, extended, to the	7 4	11.9
18	southern corporate limits of the Borough	7.4	11.9
	of Belmar		
19	From 6 th Avenue, extended, to 18 th	7.4	11.9
	Avenue, extended		
20	From 5 th Avenue, extended, to 6 th Avenue,	7.4	11.9
21	extended From the northern corporate limits of the	7.4	11.9
<u>~ 1</u>	Borough of Belmar to 5 th Avenue,	/.4	11.7
	extended		

extended ¹ Because of map scale limitations, the maximum wave elevation may not be shown on the FIRM *Data not available

		ELEVATION (feet NAVD)	
<u>TRANSECT</u>	LOCATION	1-PERCENT	MAXIMUM WAVE CREST ¹
22	The entire Shark River shoreline within the Borough of Belmar	7.4	8.9
23	From the corporate boundary of the Boroughs of Fair Haven and Rumson to Oceanic Bridge	7.4	8.9
24	From Sylvan Lake to the confluence of Shark River	7.4	12.0
25	From the southern limits of the Borough of Bradley Beach to Brinley Avenue	7.4	12.0
26	From Brinley Avenue to the northern corporate limits of the Borough of Bradley Beach	7.4	12.0
27	From Pitman Avenue, extended, to the southern corporate limits of the Township of Neptune	7.4	11.9
28	From the northern corporate limits of the Township of Neptune Avenue to Pitman Avenue, extended	7.4	11.9
29	*	7.4	11.9
30	*	7.4	11.9
31	*	7.4	11.9
32	*	7.4	11.9
33	From the southern corporate limits of the Borough of Deal to Parker Avenue, extended	7.5	11.9
34	From Parker Avenue, extended, to the northern corporate limits of the Borough of Deal	7.5	11.9
35	From the southern corporate limits of the City of Long Branch to Pullman Avenue, extended	7.5	11.9
36	From Pullman Avenue, extended, to Sternberger Avenue, extended	7.6	11.9
37	From Sternburger Avenue, extended, to Brighton Avenue, extended	7.6	11.9
38	From Brighton Avenue, extended, to Sea View Avenue, extended	7.6	11.9
39	From Sea View Avenue, extended, to the northern corporate limits of the City of Long Branch	7.7	12.9

Long Branch¹ Because of map scale limitations, the maximum wave elevation may not be shown on the FIRM *Data not available

		<u>ELEVATION (feet NAVD)</u> MAXIM	
TRANSECT	LOCATION	1-PERCENT	WAVE CREST ¹
40	From Cottage Street, extended, to the southern corporate limits of the Borough of Monmouth Beach	9	14.9
41	From the northern corporate limits of the Borough of Monmouth Beach, to Cottage Street, extended	9	14.9
42	The South Shrewsbury River shoreline within the Borough of Sea Bright	8	10.9
43	From the southern corporate limits of the Borough of Sea Bright to approximately 1,000' north of Imbrie Place, extended	7.8	12.9
44	From approximately 1,000' north of Imbrie Place, extended, to approximately 2,800' north of Rumson Road, extended	7.8	12.9
45	From the northern corporate limits of the Borough of Monmouth Beach to Monmouth Parkway, extended	8.0	10.9
46	From Monmouth Parkway, extended, to Raccoon Island	8.0	10.9
47	From Raccoon Island to Manahassett Creek	8.0	10.9
48	The entire Branchport Creek and Manahassett Creek shorelines within the City of Long Branch	8.0	9.9
49	From the south side of Paag Court	8.0	10.9
50	From the south side of Little Silver Point Road	8.0	10.9
51	From the north side of Little Silver Point Road	8.0	10.9
52	From the eastern shore of Oyster Bay to the corporate limits of the Boroughs of Little Silver and Rumson	8.0	10.9
53	From Rumson Road to the eastern shore of Oyster Bay	8.0	11.9
54	From approximately 1,000' above Hartshorne Lane, extended, to Rumson Road	8.0	7.9
55	From Oceanic Bridge to approximately 1,000' above Hartshorne Lane, extended	8.1	10.9
56	Entire Shark River shoreline within the Township of Neptune	8.1	10.9

¹ Because of map scale limitations, the maximum wave elevation may not be shown on the FIRM

		ELEVATION (feet NAVD)	
			MAXIMUM
TRANSECT	LOCATION	1-PERCENT	WAVE CREST ¹
57	From Conover Lane, extended, to	8.1	9.9
5 0	Hubbards Bridge	0.1	0.0
58	From Hubbards Bridge to Newman	8.1	9.9
59	Spring Road From Jones Point to Conover Lane,	8.1	10.9
59	extended	0.1	10.9
60	From Claypit Creek to Jones Point	8.1	10.9
61	From Tan Vat Road, extended, to Claypit	8.1	10.9
01	Creek	0.1	10.9
62	From the eastern corporate limits of the	8.1	10.9
02	Township of Middletown to Tan Vat	0.11	1012
	Road, extended		
63	From approximately 2,800' north of	7.9	12.9
	Rumson Road, extended, to the northern		
	corporate limits of the Borough of Sea		
	Bright		
64	The entire Shrewsbury River shoreline	8.1	10.9
	within the Borough of Sea Bright	·	
65	From Shrewsbury River, approximately	9.7	15.7
	150' west of the intersection of 5 th Street		
((and Valley Avenue	07	15 7
66	From Sandy Hook Bay, approximately 250' northwest of the intersection of	9.7	15.7
	Central Avenue and Shore Drive		
67	From Sandy Hook Bay, approximately	9.7	15.7
07	800' northwest of the intersection of	7.1	13.7
	Central Avenue and Shore Drive		
68	From Beverout Place, extended, to the	10.5	15.9
	eastern corporate limits of the Borough of		
	Highlands		
69	From Cedar Place, extended, to Beverout	10.5	15.9
	Place, extended		
70	From Ballinswood Road, extended, to	10.5	15.9
	Cedar Place, extended	40.0	15.0
71	From 1 st Avenue, extended, to	10.5	15.9
70	Ballinswood Road, extended	10.5	15.0
72	From Avenue A, extended, to 1 st Avenue, extended	10.5	15.9
73	From Bowne Avenue, extended, to	10.5	15.9
15	Avenue A, extended	10.5	1.5.7
	11101100/11, 07101000		

¹ Because of map scale limitations, the maximum wave elevation may not be shown on the FIRM

		ELEVATION (feet NAVD)	
TRANSECT	LOCATION	1-PERCENT	<u>MAXIMUM</u> WAVE CREST ¹
74	From the western corporate limits of the Borough of Atlantic Highlands to Bowne Avenue, extended	10.5	16.9
75	From Brevent Avenue, extended, to the eastern corporate limits of the Township of Middletown	10.5	16.9
76	From the State of New Jersey Marina to Brevent Avenue, extended	10.5	16.9
77	From Cedar Avenue, extended, to the State of New Jersey Marina	10.5	16.9
78	From the mouth of Ware Creek to Cedar Avenue, extended	10.5	16.9
79	From the mouth of Comptons Creek to the mouth of Ware Creek	10.5	16.9
80	From approximately 3,000' east of Wilson Avenue, extended, to the mouth of Comptons Creek	10.5	16.9
81	From Wilson Avenue, extended, to approximately 3,000' east of Wilson Avenue	10.5	16.9
82	From the mouth of Pews Creek to Wilson Avenue, extended	10.5	16.9
83	From the western corporate limits of the Township of Middletown to the mouth of Pews Creek	10.5	16.9
84	*	10.5	16.9
85	*	10.5	16.9
86	*	10.5	16.9
87	*	10.5	16.9
88	*	10.5	16.9
89	*	10.5	16.9
90	*	10.5	16.9
91	*	10.5	16.9
92	*	10.7	16.9
93	*	10.7	16.9
94	*	10.7	16.9
95	*	10.7	16.9

¹ Because of map scale limitations, the maximum wave elevation may not be shown on the FIRM *Data not available

		ELEVATION (feet NAVD)
<u>TRANSECT</u>	LOCATION	1-PERCENT	MAXIMUM WAVE CREST ¹
95	*	10.7	16.9
97	*	10.7	16.9
98	*	10.7	16.9
99	. *	10.7	16.9
100	*	10.7	16.9
101	*	10.7	16.9
102	*	10.7	15.9
103	*	10.7	16.9
104	*	10.7	15.9
105	From the confluence of Matawan Creek with Raritan Bay (Keyport Harbor)	10.7	15.9
106	From Woodmere Road, extended, to South Concourse Drive, extended	10.7	15.9
107	From South Concourse Drive, extended, to Woodlane Drive, extended	10.7	16.9
108	From Woodlane Drive, extended, to Lakeshore Drive, extended	10.7	16.9
109	From Lakeshore Drive, extended, to the confluence of Whale Creek with Raritan Bay (Keyport Harbor)	10.7	15.9
110	From the confluence of Whale Creek with Raritan Bay (Keyport Harbor) to the northern corporate limits of the Township of Aberdeen	10.8	16.9

¹ Because of map scale limitations, the maximum wave elevation may not be shown on the FIRM *Data not available

Each transect was taken perpendicular to the shoreline and extended inland to a point where the wave action ceased. Along each transect, wave heights and elevations were computed considering the combined effects of changes in ground elevation, vegetation, and physical or cultural features. The stillwater elevations for the 1-annual chance flood were used as starting elevations for these computations. Wave heights were calculated to the nearest 0.1 foot, and wave elevations were determined at whole-foot increments along the transects. The locations of the 3-foot breaking wave for determining the terminus of the V Zone (area with velocity wave actions) was also computed at each transect. The results of this analysis are summarized in Table 11, "Transect Data."

TABLE 11 - TRANSECT DATA

	<u>STIL</u>	LWATER ELE	VATION (feet N	AVD)		E FLOOD EVATION
FLOODING SOURCE	10-PERCENT	2-PERCENT	1-PERCENT	0.2-PERCENT	<u>ZONE</u>	(feet NAVD)
ATLANTIC OCEAN Transect 8	5.5	6.7	7.3	8.9	AE	7-10
Transect o	5.5	0.7	7.5	0.7	AE VE	10-12
Transects 9-14	5.5	6.7	7.4	8.9	AE	7-10
					VE	10-12
Transects 15-21	5.5	6.8	7.4	9.0	AE	7-10
					VE	10-12
Transects 24-26	5.5	6.7	7.4	9.0	AE	7-10
					VE	10-12
Transects 27-32	5.5	6.8	7.4	9.0	AE	7-10
					VE	10-12
Transects 33-35	5.5	6.9	7.5	9.1	AE	8-11
					VE	11-12
Transect 36	5.6	6.9	7.6	9.2	AE	8-11
					VE	11-12
Transect 37	5.6	7.0	7.6	9.3	AE	8-11
					VE	11-12
Transect 38	5.6	7.1	7.6	9.4	AE	8-11
					VE	11-12
Transect 39	5.7	7.1	7.7	9.5	AE	8-11
					VE	11-13
Transect 40-41	6.0	7.8	9.0	13.6	AE	9-12
					VE	12-15
Transect 43	5.8	7.1	7.8	9.6	AE	8-11
					VE	11-13
Transect 44	5.9	7.2	7.8	9.8	AE	8-11
					VE	11-13
Transect 54	5.0	7.0	8.0	12.6	AE	8-11
					VE	11-12
Transect 63	5.9	7.3	7.9	9.9	AE	8-11
					VE	11-13
GLIMMER GLASS HARBOR						
Transects 6-7	5.5	6.7	7.3	8.8	AE	7-9

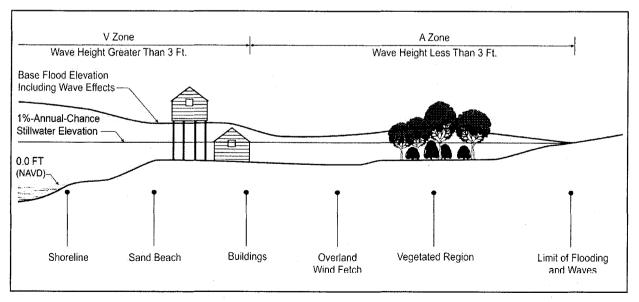
TABLE 11 - TRANSECT DATA - continued

	STIL	LWATER ELE	VATION (feet N	AVD)		E FLOOD EVATION
FLOODING SOURCE	10-PERCENT	2-PERCENT	1-PERCENT	0.2-PERCENT	<u>ZONE</u>	(feet NAVD)
MANASQUAN RIVER						
Transects 1-5	5.5	6.7	7.3	8.8	AE	7-9
NAVESINK RIVER						
Transects 55-56	5.9	7.3	8.1	10.4	AE	8-11
Transects 57-58	5.9	7.3	8.1	10.4	AE	8-10
Transects 59-62	5.9	7.3	8.1	10.4	AE	8-11
RARITAN BAY						
Transects 75-83	6.1	9.0	10.5	12.9	AE	11-14
					VE	14-17
Transects 84-89	6.1	9.1	10.5	13.4	AE	11-14
					VE	14-17
Transects 90-102	6.3	9.5	10.7	13.8	AE	11-14
					VE	14-17
Transects 103-106	6.3	9.5	10.7	13.8	AE	11-14
					VE	14-16
Transects 107-108	6.3	9.5	10.7	13.8	AE	11-14
					VE	14-17
Transect 109	6.3	9.5	10.7	13.8	AE	11-14
					VE	14-16
Transect 110	6.3	9.5	10.7	13.8	AE	11-14
SANDY HOOK BAY					VE	14-17
Transects 65-67	6.3	8.8	9.7	11.8	AE	10-13
	0.5	0.0	2.1	11.0	VE	13-16
Transects 68-73	6.1	9.0	10.5	12.9	AE	11-14
					VE	14-16
Transect 74	6.1	9.0	10.5	12.9	AE	11-14
					VE	14-17
SHARK RIVER						
Transects 22-23	5.5	6.8	7.4	9.0	AE	7-9

TABLE 11 - TRANSECT DATA - continued

	<u>STII</u>	BASE FLOOD ELEVATION				
FLOODING SOURCE	10-PERCENT	2-PERCENT	1-PERCENT	0.2-PERCENT	ZONE	(feet NAVD)
SHREWSBURY RIVER Transects 45-47 Transect 48 Transects 49-53 Transect 64	5.0 5.0 5.0 5.9	6.8 6.8 7.0 7.3	8.0 8.0 8.0 8.1	12.7 12.7 12.6 12.6	AE AE AE AE	8-11 8-10 8-11 8-11
SOUTH SHREWSBURY RIVER Transect 42	5.0	7.3	8.0	12.6	AE	8-11

Figure 3 is a profile for a typical transect illustrating the effects of energy dissipation and regeneration on a wave as it moves inland. This figure shows the wave elevations being decrease by obstructions, such as buildings, vegetation and rising ground elevations, and being increased by open, unobstructed wind fetches. Actual wave conditions in the County may not include all situations illustrated in Figure 3.



TRANSECT SCHEMATIC

Figure 3

3.5 Vertical Datum

All FISs and FIRMs are referenced to a specific vertical datum. The vertical datum provides a starting point against which flood, ground, and structure elevations can be referenced and compared. Until recently, the standard vertical datum in use for newly created or revised FISs and FIRMs was the National Geodetic Vertical Datum of 1929 (NGVD 29). With the finalization of the North American Vertical Datum of 1988 (NAVD 88), many FIS reports and FIRMs are being prepared using NAVD 88 as the referenced vertical datum.

All flood elevations shown in this FIS report and on the FIRM are referenced to NAVD 88. Structure and ground elevations in the community must, therefore, be referenced to NAVD 88. It is important to note that adjacent communities may be referenced to NGVD 29. This may result in differences in base flood elevations across the corporate limits between the communities.

Prior versions of the FIS report and FIRM were referenced to NGVD 29. When a datum conversion is effected for an FIS report and FIRM, the Flood Profiles, base flood elevations (BFEs) and ERMs reflect the new datum values. To compare structure and ground elevations to 1-percent annual chance flood elevations shown in the FIS and on the FIRM, the subject structure and ground elevations must be referenced to the new datum values.

As noted above, the elevations shown in the FIS report and on the FIRM for Monmouth County are referenced to NAVD 88. Ground, structure, and flood elevations may be compared and/or referenced to NGVD 29 by applying a standard conversion factor. The conversion factor to NGVD 29 is +1.1. The conversion between the datums may be expressed as an equation:

The BFEs shown on the FIRM represent whole-foot rounded values. For example, a BFE of 102.4 will appear as 102 on the FIRM and 102.6 will appear as 103. Therefore, users that wish to convert the elevations in this FIS to NGVD 29 should apply the stated conversion factor(s) to elevations shown on the Flood Profiles and supporting data tables in the FIS report, which are shown at a minimum to the nearest 0.1 foot.

For more information on NAVD 88, see <u>Converting the National Flood Insurance</u> <u>Program to the North American Vertical Datum of 1988</u>, FEMA Publication FIA-20/June 1992, or contact the Spatial Reference System Division, National Geodetic Survey, NOAA, Silver Spring Metro Center, 1315 East-West Highway, Silver Spring, Maryland 20910 (Internet address http://www.ngs.noaa.gov).

4.0 FLOODPLAIN MANAGEMENT APPLICATIONS

The NFIP encourages State and local governments to adopt sound floodplain management programs. To assist in this endeavor, each FIS provides 1-percent annual chance floodplain data, which may include a combination of the following: 10-, 2-, 1-, and 0.2-percent annual chance flood elevations; delineations of the 1- and 0.2-percent annual chance floodplains; and 1-percent annual chance floodway. This information is presented on the FIRM and in many components of the FIS, including Flood Profiles, Floodway Data tables, and Summary of Stillwater Elevation tables. Users should reference the data presented in the FIS as well as additional information that may be available at the local community map repository before making flood elevation and/or floodplain boundary determinations.

4.1 Floodplain Boundaries

To provide a national standard without regional discrimination, the 1-percent annual chance flood has been adopted by FEMA as the base flood for floodplain management purposes. The 0.2-percent annual chance flood is employed to indicate additional areas of flood risk in the county. For the streams studied in detail, the 1- and 0.2-percent annual chance floodplain boundaries have been delineated using the flood elevations determined at each cross section.

Floodplain delineation was based on topographic data provided by Monmouth County. This data were derived from April 2003 aerial photogrammetry.

Similarly, using datum-converted effective flood profiles for non-revised, detailed streams, all flood boundaries were made current with the topography supplied by Monmouth County.

In the Township of Aberdeen, aerial photographs at a scale of 1:9,600 and 1:12,000 were used to delineate floodplain boundaries between cross sections, as well as topographic maps at a scale of 1"=400' with a 4-foot contour interval. Information was supplemented by 7.5-Minute Series Topographic Maps at a scale of 1:24,000 and contour interval of 20 feet.

In the Borough of Allenhurst, aerial contour maps at a scale of 1"=500' and a 2-foot contour interval were used to delineate floodplain boundaries between cross sections, as well as 7.5-Minute Series Topographic Maps at a scale of 1:24,000 and contour interval of 20 feet.

In the Borough of Allentown, aerial photographs and topographic maps at a scale of 1:2,400 and 5-foot contour interval were used to delineate floodplain boundaries between cross sections, as well as 7.5-Minute Series Topographic Maps at a scale of 1:24,000 and 10-foot contour interval.

In the City of Asbury Park, floodplain boundaries between cross sections were interpolated using aerial contour maps at a scale of 1:6,000 with a 2-foot contour interval, as well as 7.5-Minutes Series Topographic Maps at a scale of 1:24,000 and a 20-foot contour interval.

In the Borough of Avon-by-the-Sea, aerial photographs and topographic maps at a scale of 1:6,000 and contour interval of 2 feet were used to delineate floodplain boundaries between cross sections, as well as 7.5-Minute Series topographic maps at a scale of 1:24,000 and contour interval of 20 feet.

In the Borough of Belmar, floodplain boundaries between cross sections were interpolated using aerial photographs at a scale of 1:12,000, topographic maps at a scale of 1:2,400 with a 4-foot contour interval, as well as 7.5-Minute Series topographic maps at a scale of 1:24,000 and a 20-foot contour interval.

In the Borough of Bradley Beach, floodplain boundaries between cross sections were interpolated using aerial photographs and topographic maps at a scale of 1:6,000 with a 2-foot contour interval, and 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

In the Borough of Brielle, aerial contour maps of a 1:6,000 scale and 2-foot contour interval were used to delineate floodplain boundaries between cross sections, as well as aerial contour maps of a 1:2,400 scale with a 5-foot contour interval. Additionally, 7.5-Minute Series topographic maps were used, with a scale of 1:24,000 and 20-foot contour interval.

In the Township of Colts Neck, floodplain boundaries between cross sections were interpolated using topographic maps of 1:2,400 scale with a 5-foot contour interval.

In the Borough of Deal, aerial photographs and aerial plotting plates at a scale of 1:12,000 were used to delineate floodplain boundaries between cross sections, as well as topographic maps at scales of 1:24,000 and 1:2,400 with contour intervals of 4 feet. Additionally, 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval were used.

In the Borough of Eatontown, topographic maps of Northeast Monmouth County, at a scale of 1:1,200 and a one-foot contour interval, were used to delineate floodplain boundaries between cross sections, as well as 7.5-Minute Series topographic maps at a scale of 1:24,000 and a 20-foot contour interval.

In the Borough of Englishtown, topographic maps sheets at a scale of 1"=200' and a contour interval of 5 feet were used to delineate floodplain boundaries between cross sections, as well as 7.5-Minute Series topographic maps at a scale of 1:24,000 and a 20-foot contour interval.

In the Borough of Fair Haven, floodplain boundaries between cross sections were interpolated using topographic maps of Northeast Monmouth County, at a scale of 1:1,200 and a one-foot contour interval.

In the Township of Freehold, topographic maps at a scale of 1:2,400 with a 5-foot contour interval were used to delineate floodplain boundaries between cross sections, as well as 7.5-Minute Series topographic maps and flood-prone area maps at a scale of 1:24,000 and a 10-foot contour interval.

In the Township of Hazlet, topographic maps at a scale of 1:1,200 and a 2-foot contour interval were used to delineate floodplain boundaries between cross sections.

In the Borough of Highlands, topographic maps at a scale of 1"=200' with a contour interval of 2 feet were used to delineate floodplain boundaries between cross sections.

In the Township of Holmdel, topographic maps at a scale of 1:24,000 with a 5foot contour interval were used to delineate floodplain boundaries between cross sections.

In the Township of Howell, topographic maps at a scale of 1:2,400 with a 5-foot contour interval were used to delineate floodplain boundaries between cross sections.

In the Borough of Keansburg, topographic maps of scales 1" = 500' with a contour interval of 2 feet and 1:600 with a one-foot contour interval were used to delineate floodplain boundaries between cross sections, as well as 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

In the Borough of Keyport, aerial contour maps at a scale of 1:6,000 with a 2-foot contour interval were used to delineate floodplain boundaries between cross sections, as well as 7.5-Minute topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

In the Borough of Little Silver, aerial photographs with a scale of 1:12,000 were used to delineate floodplain boundaries between cross sections, as well as topographic maps at a scale of 1:1,200 with a one-foot contour interval and 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

In the Village of Loch Arbour, aerial contour maps at a scale of 1:6,000 with a 2foot contour interval were used to delineate floodplain boundaries between cross sections, as well as 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

In the City of Long Branch, aerial photographs and aerial plotting plates at a scale of 1:12,000 were used to delineate floodplain boundaries between cross sections, as well as topographic maps at a scale of 1:2,400 with a 4-foot contour interval and 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

In the Township of Manalapan, 7.5-Minute Series topographic maps at a scale of 1:24,000 with contour intervals of 10 feet and 5 feet were used to delineate floodplain boundaries between cross sections.

In the Borough of Manasquan, aerial photographs and aerial plotting plates at a scale of 1:12,000 were used to delineate floodplain boundaries between cross sections, as well as topographic maps at a scale of 1:2,400 with a 4-foot contour interval and 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

In the Township of Marlboro, topographic maps at a scale of 1:2,400 with a 5-foot contour interval were used to delineate floodplain boundaries between cross sections, as well as 7.5-Minute Series topographic maps at a scale of 1:24,000 with contour intervals of 10 feet and 20 feet.

In the Borough of Matawan, floodplain boundaries were interpolated between cross sections using topographic maps at a scale of 1:2,400 with a 5-foot contour interval.

In the Township of Middletown, floodplain boundaries were interpolated between cross sections using topographic maps at a scale of 1"=100' with a one-foot contour interval, as well as 7.5-Minute Series topographic maps at a scale of 1:24,000 with contour intervals of 10 feet and 20 feet.

In the Township of Millstone, aerial photographs and photogrammetric mapping at a scale of 1:2,400 with a 5-foot contour interval were used to delineate floodplain boundaries between cross sections, as well as 7.5-Minute Series topographic maps at a scale of 1:24,000 with contour intervals of 5 feet and 10 feet.

In the Borough of Monmouth Beach, aerial photographs at a scale of 1:12,000 were used to delineate floodplain boundaries between cross sections, as well as topographic maps at a scale of 1:1,200 with a one-foot contour interval and 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

In the Township of Neptune, aerial photographs and aerial plotting plates at a scale of 1:12,000 were used to delineate floodplain boundaries between cross sections, as well as topographic maps at a scale of 1:2,400 with a 4-foot contour interval and 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

In the Township of Ocean, topographic maps at scales of 1:1,200 with a 2-foot contour interval and 1:24,000 with a 4-foot contour interval were used to delineate floodplain boundaries between cross sections.

In the Borough of Oceanport, floodplain boundaries between cross sections were interpolated using topographic maps at a scale of 1"=100' with a one-foot contour interval.

In the Borough of Red Bank, floodplain boundaries between cross sections were interpolated using topographic maps at a scale of 1:1,200 with a 2-foot contour interval.

In the Borough of Rumson, aerial photographs at a scale of 1:12,000 were used to delineate floodplain boundaries between cross sections, as well as topographic maps at a scale of 1:1,200 with a one-foot contour interval and 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

In the Borough of Sea Bright, aerial photographs and aerial plotting plates at a scale of 1:12,000 were used to delineate floodplain boundaries between cross sections, as well as topographic maps at a scale of 1:2,400 with a 4-foot contour interval and 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

In the Borough of Sea Girt, aerial photographs and aerial plotting plates at a scale of 1:12,000 were used to delineate floodplain boundaries between cross sections, as well as topographic maps at a scale of 1:2,400 with a 4-foot contour interval and 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

In the Borough of Shrewsbury, floodplain boundaries between cross sections were interpolated using topographic maps at a scale of 1:1,200 with a one-foot contour interval.

In the Borough of Spring Lake, aerial photographs and aerial plotting plates at a scale of 1:12,000 were used to delineate floodplain boundaries between cross sections, as well as topographic maps at a scale of 1:2,400 with a 4-foot contour interval and 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

In the Borough of Spring Lake Heights, floodplain boundaries between cross sections were interpolated using topographic maps at a scale of 1:2,400 with a contour interval of 5 feet, as well as 7.5-Minute Series flood-prone area maps at a scale of 1:24,000 with a 20-foot contour interval.

In the Borough of Tinton Falls, floodplain boundaries between cross sections were interpolated using topographic maps at a scale of 1:1,200 with a 3-foot contour interval.

In the Borough of Union Beach, floodplain boundaries between cross sections were interpolated using topographic maps at a scale of 1"=50' with a one-foot contour interval and 1:6,000 with a 2-foot contour interval.

In the Township of Wall, floodplain boundaries between cross sections were interpolated using topographic maps at a scale of 1:2,400 with a 2-foot contour interval, as well as the Tax Map for the Township of Wall.

In the Borough of West Long Branch, floodplain boundaries between cross sections were interpolated using topographic maps at a scale of 1:1,200 with a one-foot contour interval as well as 7.5-Minute Series topographic maps at a scale of 1:24,000 with a 20-foot contour interval.

4.2 Floodways

Encroachment on floodplains, such as structures and fill, reduces flood-carrying capacity, increases flood heights and velocities, and increases flood hazards in areas beyond the encroachment itself. One aspect of floodplain management involves balancing the economic gain from floodplain development against the resulting increase in flood hazard. For purposes of the NFIP, a floodway is used as a tool to assist local communities in this aspect of floodplain management. Under this concept, the area of the 1-percent annual chance floodplain is divided into a floodway and a floodway fringe. The floodway is the channel of a stream, plus any adjacent floodplain areas, that must be kept free of encroachment so that the 1-percent annual chance flood can be carried without substantial increases in flood heights. Minimum federal standards limit such increases to 1.0 foot, provided that hazardous velocities are not produced. The floodways in this FIS are presented to local agencies as minimum standards that can be adopted directly or that can be used as a basis for additional floodway studies.

The floodways presented in this FIS were computed for certain stream segments on the basis of equal conveyance reduction from each side of the floodplain.

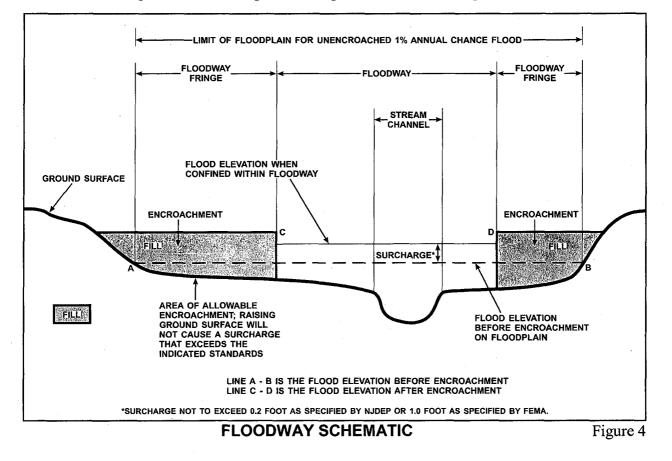
Floodway widths were computed at cross sections. Between cross sections, the floodway boundaries were interpolated. The results of the floodway computations are tabulated for selected cross sections (Table 12). The computed floodways are shown on the FIRM (Exhibit 2). In cases where the floodway and 1-percent annual chance floodplain boundaries are either close together or collinear, only the floodway boundary is shown.

Near the mouths of streams studied in detail, floodway computations are made without regard to flood elevations on the receiving water body. Therefore, "Without Floodway" elevations presented in Table 12 for certain downstream cross sections of Ardena Brook, Bannen Meadow Brook, Burkes Creek, Claypit Creek, Comptons Creek, Gravelly Run, Groundhog Brook, Haystack Brook, Jumping Brook 1, McClees Creek, Mohingson Brook, Parkers Creek, Parkers Creek North Branch, Pine Brook 1, Poplar Creek Tributary 1, Shark River, Waackaack Creek, Weamaconk Creek Tributary, and Yellow Brook Tributary 2 are lower than the regulatory flood elevations in that area, which must take into account the 1-percent annual chance flooding due to backwater from other sources.

No floodways were calculated for Cranberry Brook or Wells Brook.

Portions of the floodways for Metedeconk River North Branch and Rocky Brook (Downstream Reach) extend beyond the county boundary Encroachment into areas subject to inundation by floodwaters having hazardous velocities aggravates the risk of flood damage, and heightens potential flood hazards by further increasing velocities. A listing of stream velocities at selected cross sections is provided in Table 12, "Floodway Data." In order to reduce the risk of property damage in areas where the stream velocities are high, the community may wish to restrict development in areas outside the floodway.

The area between the floodway and 1-percent annual chance floodplain boundaries is termed the floodway fringe. The floodway fringe encompasses the portion of the floodplain that could be completely obstructed without increasing the water-surface elevation of the 1-percent annual chance flood by more than 1.0 foot at any point. Typical relationships between the floodway and the floodway fringe and their significance to floodplain development are shown in Figure 4.



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FLO	OODING SOUF	RCE		FLOODWA	Y	, v	BASE F ATER-SURFAC (FEET N	E ELEVATION	
CROSS S	SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Applegates Cree	ek				0200.02				
A		90 ¹	37	92	5.90	91.7	91.7	91.9	0.2
В		1,375 ¹	69	214	2.50	95.5	95.5	95.7	0.2
C		2,835 ¹	74	198	2.70	102.7	102.7	102.9	0.2
D		2,865 ¹	189	669	0.80	103.1	103.1	103.3	0.2
Ē		4,030 ¹	31	131	4.10	103.7	103.7	103.8	0.1
F		4,080 ¹	15	70	7.70	103.7	103.7	103.8	0.1
Ġ		4,205 ¹	15	85	6.30	104.5	104.5	103.0	0.2
н		4,255 ¹	59	184	2.90	104.5	104.5	105.3	0.2
11		5,0551	34	86	6.20	106.2	106.2	105.3	0.1
1		6,015 ¹							
J		0,015	67	174	3.10	110.0	110.0	110.0	0.0
к		6,665 ¹	97	282	1.90	114.7	114.7	114.7	0.0
L		6,695 ¹	109	247	2.20	114.7	114.7	114.7	0.0
Ardena Brook							2		
A		280 ²	19	60	6.10	63.4	61.5 ³	61.7	0.2
В		1,165 ²	19	59	6.30	66.1	66.1	66.3	0.2
С	,	1,165 ² 1,680 ²	28	62	6.00	70.6	70.6	70.6	0.0
D)	2,274 ²	14	56	6.60	78.8	78.8	78.8	0.0
Ē		2,304 ²	35	138	2.70	79.6	79.6	79.6	0.0
Bannen Meadov	w Brook								
A		310 ²	30	100	6.90	69.5	67.5 ³	67.7	0.2
В		1 028 ²	19	130	5.30	74.3	74.3	74.3	0.0
C		1,028 ² 1,058 ²	69	374	1.80	74.8	74.8	74.8	0.0
D		1,960 ²	152	798	0.90	74.8	75.0	75.0	0.0
E		2,945 ²	86	216	3.20	75.1	75.1	75.1	0.0
E		2,945	00	210	3.20	75.1	75.1	75.1	0.0
¹ East above cor	nfluence with Deb	pois Crook				<u> </u>			
	fluence with Mar								
		sideration of back	uator offecto f	rom Monooquo	n Divor				
Elevation comp		Sideration of back							
FEDE	RAL EMERGEN	CY MANAGEMEN	IT AGENCY			·····		_	
							DWAY DA	ТΔ	
ו ס									
	ONMOUT	H COUNT	Y, NJ						
		RISDICTIO	•	APPLEGATES CREEK – ARDENA BROOK –					K -
									/K –
ა				BANNEN MEADOW BROOK					

 	FLOODING SOUI									
	FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
	CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
	n Meadow Brook						· · · · · · · · · · · · · · · · · · ·			
(contir	F	3,000 ¹	14	59	11.70	75.3	75.3	75.3	0.0	
	G	3,160 ¹	14	83	8.30	78.0	78.0	78.0	0.0	
	н	3,210 ¹	53	256	2.70	79.1	79.1	79.1	0.0	
	1	4,010 ¹	58	155	4.40	80.0	80.0	80.1	0.0	
	l I	4,617 ¹	100	308	2.20	81.4	81.4	81.6	0.1	
	J	5,347 ¹	79	282						
	K	5,347 5,377 ¹			2.40	84.3	84.3	84.3	0.0	
	L Mi	6,034 ¹	52 49	109 182	6.30 3 <i>.</i> 80	84.3 87.5	84.3 87.5	84.3	0.0	
	IVI	0,034	49	182	3.80	67.5	87.5	87.6	0.1	
Barcla	y Brook									
	А	211 ²	30	85	4.60	73.9	73.9	73.9	0.0	
	В	581 ²	70	165	2.30	75.5	75.5	75.5	0.0	
	С	686 ²	70	330	1.20	78.4	78.4	78.4	0.0	
	D	1,214 ²	30	95	4.10	78.6	78.6	78.8	0.2	
	E	1,320 ²	45	225	1.70	82.5	82.5	82.5	0.0	
	F	1,795 ²	25	45	8.20	83.5	83.5	83.5	0.0	
	G	3,010 ²	35	115	3.30	94.0	94.0	94.0	0.0	
	н	3,590 ²	35	50	7.50	99.5	99.5	99.5	0.0	
	1	4,066 ²	80	160	2.40	105.9	105.9	106.1	0.2	
	J	4,435 ²	15	40	9.20	110.1	110.1	110.1	0.0	
Barron	n Neck Creek									
Darren		2,000 ³	102	367	1.40	44.4	44.4	44.4	0.0	
	A B	2,000 2,700 ³	33	75	6.70	44.4 46.6	44.4 46.6	44.4 46.6		
		3,050 ³			1				0.0	
	C		29	64	7.90	50.0	50.0	50.0	0.0	
	D	3,300 ³	30	285	1.80	59.5	59.5	59.5	0.0	
⁻¹ Feet a	above confluence with Mar	nasquan River		I		l		L	I	
² Feet a	above county boundary	•								
°Feet a	above mouth									
			TACENOV							
╏	FEDERAL EMERGEN		I AGENUY							
i i						FLOO[DWAY DA	TA		
₽	MONMOUT	H COUNT	Y. NJ							
Π		RISDICTIO	•		DANNEN					
<u>-</u>			13)		DANNEN	MEADOW BI			OOR -	
১						BARREN	I NECK CR	EEK		

FLOODING SOUR	CE		FLOODWA	 Y	v	BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Barren Neck Creek (continued) E	3,950 ¹	36	235	2.10	59.5	59.5	59.7	0.2	
F G	4,200 ¹ 4,600 ¹	55 119	412 595	1.20 0.80	61.8 61.9	61.8 61.9	61.8 62.0	0.0 0.1	
Betty Brook					!				
A B	0 ¹ 600 ¹	289 50	498 74	1.00 7.00	54.8 61.1	54.8 61.1	55.0 61.1	0.2	
С	1,100 ¹	40	71	7.20	66.5	66.5	66.6	0.1	
D E	1,900 ¹ 2,340 ¹	30 26	76 70	6.80 7.30	72.1 74.5	72.1 74.5	72.3 74.6	0.2 0.1	
Big Brook (Downstream Reach)					'				
A	740 ²	171	532	3.00	38.5	38.5	38.7	0.2	
В	1,700 ²	187	473	3.30	39.9	39.9	40.1	0.2	
С	2,770 ²	252	786	2.00	41.5	41.5	41.7	0.2	
D	3,000 ²	250	629	2.50	42.3	42.3	42.3	0.0	
E	4,500 ²	296	484	3.20	45.1	45.1	45.3	0.2	
F	5,330 ²	475	877	1.80	47.5	47.5	47.6	0.1	
G	6,830 ²	267	484	3.20	50.1	50.1	50.3	0.2	
H	7,880 ²	220	890	1.80	53.9	53.9	54.0	0.1	
i ,	8,400 ²	203	773	2.00	54.1	54.1	54.3	0.2	
J	9,000 ²	71	273	5.70	54.6	54.6	54.8	0.2	
к	10,220 ²	161	633	2.50	57.4	57.4	57.6	0.2	
Big Brook (Upstream Reach)									
A	03	90	375	3.10	78.5	78.5	78.7	0.2	
В	158 ³	90	400	2.90	78.8	78.8	79.0	0.2	
C	2,218 ³	80	245	3.30	81.7	81.7	81.9	0.2	
¹ Feet above mouth	4,118 ¹	80	195	4.20	86.7	86.7	86.7	0.0	

¹Feet above mouth ²Feet above confluence with Swimming River Reservoir ³Feet above limit of detailed study (limit of detailed study is approximately 50 feet below Boundary Road)

TABLE	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
E 12	(ALL JURISDICTIONS)	BARREN NECK CREEK – BETTY BROOK – BIG BROOK (UPSTREAM REACH) – BIG BROOK (DOWNSTREAM REACH)

FLOODING SOUF	RCE	FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)					
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE		
ig Brook (Upstream Reach) continued)										
É	5,861 ¹	40	165	4.20	93.3	93.3	93.3	0.0		
F	5,966 ¹	35	175	4.00	93.9	93.9	93.9	0.0		
G	7,0751	30	105	6.70	97.2	97.2	97.2	0.0		
H	7,181 ¹	35	145	4.80	98.4	98.4	98.5	0.0		
1	9,504 ¹	50	130	3.90	104.3	104.3	104.3	0.0		
' J	11,405 ¹	20	55	8.80	116.2	116.2	116.3	0.0		
				0.00	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		, 10.0	V.1		
lig Brook Tributary H	2	1								
А	211 ²	40	180	3.00	79.2	79.2	79.4	0.2		
В	2,482 ²	70	135	2.90	90.1	90.1	90.2	0.1		
С	2,587 ²	70	430	0.90	94.5	94.5	94.5	0.0		
D	5,702 ²	25	30	3.10	119.8	119.8	120.0	0.2		
Burkes Creek										
A	370 ³	53	161	2.40	90.2	89.3 ⁴	89.5	0.2		
В	590 ³	12	50	7.80	90.3	89.3 ⁴	89.5	0.2		
C	620 ³	55	134	2.90	90.3	90.2 ⁴	90.3	0.2		
D	690 ³									
-		125	158	2.50	94.2	94.2	94.3	0.1		
E	720 ³	185	1,444	0.30	94.3	94.3	94.4	0.1		
F	1,300 ³	143	911	0.40	94.3	94.3	94.4	0.1		
G	1,800 ³	24	158	2.50	99.1	99.1	99.2	0.1		
Н	1,830 ³	195	1,517	0.30	99.3	99.3	99.4	0.1		
ł	2,180 ³	156	1,108	0.40	99.3	99.3	99.4	0.1		
j	2,695 ³	172	573	0.70	99.5	99.5	99.6	0.1		
к	2,725 ³	110	456	0.90	99.5	99.5	99.6	0.1		
L	3,110 ³	25	86	4.50	99.6	99.6	99.7	0.1		
M	3,610 ³	121	250	1.50	100.4	100.4	100.6	0.2		
Feet above limit of detailed stud Feet above confluence with Big Feet above confluence with Det Elevation computed without con	y (limit of detailed Brook pois Creek		ŗ		y Road)					
FEDERAL EMERGEN	CY MANAGEMEN	IT AGENCY								
		Y. N.J	FLOODWAY DATA							
(ALL JUF	RISDICTIO	•		BIG	BROOK (DO	WNSTREA	M REACH)			
-		-			OOK TRIBUI					

FLOODING SOU	JRCE		FLOODWA	Y	BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Claypit Creek A	01	*	2,419	0.30	8.1	4.0 ⁴	4.2	0.2	
B C D E	1,200 ¹ 1,900 ¹ 2,020 ¹ 2,450 ¹	220 250 214	691 390 631 781	0.90 1.60 1.00 0.50	8.1 8.1 10.6 10.8	4.1 ⁴ 4.8 ⁴ 10.6 10.8	4.3 4.9 10.6 10.8	0.2 0.1 0.0 0.0	
Comptons Creek									
A B C D E F	$ \begin{array}{r} 115^{2} \\ 622^{2} \\ 1,430^{2} \\ 3,850^{2} \\ 5,235^{2} \\ 5,615^{2} \\ \end{array} $	359 554 34 253 185 124	1,209 1,713 262 681 570 446	1.10 0.80 5.10 1.90 1.60 2.00	10.5 10.5 10.5 10.5 12.3 13.1	5.9 ⁵ 6.1 ⁵ 7.0 ⁵ 7.9 ⁵ 12.3 13.1	6.0 6.3 7.2 8.1 12.4 13.1	0.1 0.2 0.2 0.2 0.1 0.0	
Cranberry Brook	2								
A B C D	640 ³ 2,280 ³ 2,490 ³ 3,610 ³	*	* * *	*	14.0 16.5 19.6 19.6	*	* * *	* *	
E F G	3,960 ³ 4,340 ³ 5,295 ³	*	*	*	19.6 19.6 19.6 19.6	* *	* * *	* *	
Deal Tributary 1									
A B C	230 ³ 350 ³ 630 ³	12 16 39	88 84 206	0.50 0.50 0.20	10.5 10.6 10.7	10.5 10.6 10.7	10.5 10.6 10.8	0.0 0.0 0.1	
¹ Feet above Locust Avenue Br ² Feet above Campbell Avenue ³ Feet above mouth ⁴ Elevation computed without co			*Data n	not available	hout consideration of	f backwater effects	s from Raritan Bay	<u> </u>	
FEDERAL EMERGE		T AGENCY			FLOOI	DWAY DA	ТА		
T (ALL JU	RISDICTIO	•			PIT CREEK BERRY BRO				

FLOODING SOL	FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Deal Tributary 1 (continued)								
D	1,060 ¹	20	12	3.30	10.7	10.7	10.7	0.0
E	1,560 ¹	35	97	0.40	19.8	19.8	19.8	0.0
F	1,940 ¹	11	17	2.40	19.8	19.8	19.8	0.0
G	1,975 ¹	13	13	3.10	19.9	19.9	19.9	0.0
Deal Tributary 2								
Â	0 ¹	54	450	0.40	10.5	10.5	10.5	0.0
В	137 ¹	177	1,082	0.10	14.8	14.8	14.8	0.0
С	1,200 ¹	157	768	0.20	18.3	18.3	18.5	0.2
D	2,450 ¹	6	15	5.80	25.1	25.1	25.2	0.1
E	2,520 ¹	4	10	8.70	25.5	25.5	25.7	0.2
F	2,990 ¹	4	14	6.20	31.0	31.0	31.0	0.0
Deal Tributary 3								
Â	310 ¹	142	968	0.40	13.7	13.7	13.7	0.0
В	2,650 ¹	98	208	0.60	18.2	18.2	18.2	0.0
С	3,820 ¹	49	65	1.30	28.1	28.1	28.1	0.0
D	4,525 ¹	75	120	0.70	33.7	33.7	33.8	0.1
E	5,295 ¹	4	4	6.50	43.1	43.1	43.1	0.0
F	5,345 ¹	13	6	4.30	44.2	44.2	44.2	0.0
Deal Tributary 3A								
A	470 ²	20	31	0.80	18.2	18.2	18.4	0.2
В	595 ²	41	119	0.40	26.2	26.2	26.2	0.0
С	940 ²	23	49	0.90	26.2	26.2	26.2	0.0
D	985 ²	28	89	0.50	26.2	26.2	26.2	0.0

¹Feet above mouth ²Feet above confluence with Deal Tributary 3

TABL	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
E 12	(ALL JURISDICTIONS)	DEAL TRIBUTARY 1 – DEAL TRIBUTARY 2 – DEAL TRIBUTARY 3 – DEAL TRIBUTARY 3A

FLOODING SO	FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Deal Tributary 4								
Â	15 ¹	144	693	0.00	13.7	13.7	13.9	0.2
В	500 ¹	78	335	0.10	13.7	13.7	13.9	0.2
С	1,000 ¹	84	295	0.10	13.7	13.7	13.9	0.2
D	1,680 ¹	24	49	5.30	15.2	15.2	15.4	0.2
E	1,900 ¹	62	97	2.70	16.6	16.6	16.6	0.0
F	4,000 ¹	8	38	6.40	26.8	26.8	26.9	0.1
Deal Tributary 4A								
A	645 ²	9	9	5.60	16.7	16.7	16.7	0.0
В	730 ²	53	57	0.90	17.5	17.5	17.7	0.2
C	845 ²	53	57	0.90	17.5	17.5	17.7	0.2
Debois Creek								
Α	80 ³	330	973	1.10	76.5	76.5	76.7	0.2
B	1,045 ³	30	182	6.10	77.0	77.0	77.2	0.2
C	1,075 ³	107	354	3.10	77.6	77.6	77.6	0.0
D	1,890 ³	163	425	2.60	78.9	78.9	79.1	0.2
Ē	2,670 ³	210	480	2.30	80.3	80.3	80.5	0.2
F	3,620 ³	96	229	4.80	83.4	83.4	83.5	0.1
G	4,685 ³	55	278	4.00	86.2	86.2	86.3	0.1
н	4,735 ³	31	190	5.80	86.2	86.2	86.3	0.1
i	4,925 ³	31	244	4.60	87.8	87.8	87.9	0.1
' 1	4,975 ³	78	412	2.70	88.1	88.1	88.2	0.1
ĸ	6,035 ³	28	193	5.80	88.3	88.3	88.4	
	6,065 ³	20	193	8.50	88.3	88.3	88.4 88.4	0.1
L M	6,975 ³	24	480	2.30	90.0	90.0	90.2	0.1
N	7,890 ³	68	234	4.70	90.0	90.0 91.0	90.2	0.2 0.1
¹ Feet above confluence with D ² Feet above confluence with D ³ Feet above confluence with N	eal Tributary 3 eal Tributary 4		234	4.70	91.0	91.0	51.1	0.1
FEDERAL EMERG	ENCY MANAGEMEN	T AGENCY					<u> </u>	
MONMOU	TH COUNT	Y. NJ			FLOOD	DWAY DA	ТА	
(ALL JU	RISDICTIO		DEAL TRIBUTARY 4 – DEAL TRIBUTARY 4A – DEBOIS CREEK					

	FLOODING SOU	RCE		FLOODWA	Y	l v	BASE F ATER-SURFAC (FEET N	E ELEVATION	
	CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
	oois Creek								
(cor	ntinued)			1					
	0	8,870	29	122	7.60	93.1	93.1	93.3	0.2
	Р	9,600	29	105	8.80	96.0	96.0	96.0	0.0
	Q	10,525	72	269	3.40	101.3	101.3	101.5	0.2
	R	10,555	64	241	3.80	101.4	101.4	101.6	0.2
	S	11,640	46	145	6.30	104.9	104.9	105.0	0.1
	т	12,520	50	273	3.40	107.2	107.2	107.4	0.2
	U	13,270	28	118	7.80	108.2	108.2	108.4	0.2
	V	14,210	26	155	5.90	114.2	114.2	114.2	0.0
	W	14,240	111	580	1.60	115.0	115.0	115.0	0.0
	X	14,980	20	122	25.80	115.2	115.2	115.3	0.1
	Y	15,865	17	92	3.80	115.6	115.6	115.8	0.2
	Z	16,905	41	167	2.10	119.7	119.7	119.7	0.0
	AA	17,008	8	56	6.20	119.9	119.9	120.0	0.1
	AB	17,038	32	114	3.10	120.5	120.5	120.5	0.0
	AC	17,545	12	81	4.30	120.7	120.7	120.8	0.1
	AD	17,595	45	160	2.20	121.0	121.0	121.1	0.1
	AE	18,235	14	48	7.20	121.0	121.0	121.2	0.2
	AF	18,595	24	84	4.10	123.6	123.6	123.8	0.2
	ÂĞ	19,635	33	89	3.90	125.3	125.3	125.3	0.0
	AH	20,195	10	41	8.50	128.1	128.1	128.1	0.0
	Al	20,530	36	88	4.00	130.2	130.2	130.3	0.0
	AJ	21,240	246	325	1.10	136.8	136.8	136.8	
	AK	21,240	304	1,189	0.30	136.9	136.9	136.8	0.0 0.0
¹ Fe	et above confluence with Ma	nasquan River							
, T	FEDERAL EMERGEN			<u> </u>					<u></u>
	MONMOUT	H COUNT	Y, NJ			FLOOD	DWAY DA	ТА	
;	(ALL JUF	RISDICTIO	NS)			DEBC	DIS CREE	K	

FLOODING SO	JRCE		FLOODWA	Y	BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)					
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	(FEET N WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE		
Debois Creek Tributary A B C D	500 ¹ 950 ¹ 1,750 ¹ 2,660 ¹	39 61 27 12	103 179 86 52	4.40 2.50 5.30 8.70	95.0 96.3 101.6 106.6	95.0 96.3 101.6 106.6	95.2 96.5 101.6 106.6	0.2 0.2 0.0 0.0		
Deep Run A B C D E F G H I J	53 ² 2,851 ² 2,957 ² 6,125 ² 9,029 ² 9,134 ² 10,296 ² 10,982 ² 11,088 ² 13,781 ²	309 412 506 315 280 350 355 220 265 160	2,628 2,629 20.8 2,038 735 1,050 1,340 300 520 245	$\begin{array}{c} 1.00\\ 1.00\\ 1.00\\ 2.80\\ 1.90\\ 1.10\\ 4.70\\ 1.10\\ 2.30\end{array}$	53.7 60.1 61.2 63.8 76.8 77.4 78.3 82.5 85.5 101.3	53.7 60.1 61.2 63.8 76.8 77.4 78.3 82.5 85.5 101.3	53.9 60.1 61.2 63.8 77.0 77.5 78.5 82.5 85.5 101.3	0.2 0.0 0.0 0.2 0.1 0.2 0.0 0.0 0.0		
Deep Run Tributary A A B C	1,373 ³ 3,062 ³ 4,752 ³	140 190 50	180 300 55	3.40 1.60 6.40	89.8 93.4 104.1	89.8 93.4 104.1	89.8 93.4 104.1	0.0 0.0 0.0		
Deep Run Tributary B A B C ¹ Feet above confluence with Do ² Feet above county boundary ³ Feet above confluence with Do		390 180 225	685 270 490	1.40 3.20 1.70	71.4 81.0 82.6	71.4 81.0 82.6	71.5 81.1 82.7	0.1 0.1 0.1		
FEDERAL EMERGE	-			FLOODWAY DATA DEBOIS CREEK TRIBUTARY – DEEP RUN – DEEP RUN TRIBUTARY A – DEEP RUN TRIBUTARY B						
(ALL JU	RISDICTIO	NS)								

FLOODING SOL	IRCE		FLOODWA	Y	BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Deep Run Tributary B									
(continued)	4.0401	050	700	1.00					
D	4,013 ¹	250	700	1.20	83.0	83.0	83.2	0.2	
E F	4,594 ¹	260	305	1.20	83.2	83.2	83.4	0.2	
	4,699 ¹	500	795	0.40	85.3	85.3	85.3	0.0	
G	6,758 ¹	20	64	5.50	97.6	97.6	97.6	0.0	
Н	8,606 ¹	10	30	8.30	107.8	107.8	107.8	0.0	
1	8,712 ¹	310	1345	0.20	113.8	113.8	113.8	0.0	
J	9,240 ¹	10	20	6.30	113.8	113.8	113.8	0.0	
Deep Run Tributary C									
A	898 ²	185	240	2.30	86.0	86.0	86.0	0.0	
В	$1,109^2$	130	155	3.50	86.5	86.5	86.5	0.0	
С	2,429 ²	270	275	1.60	92.5	92.5	92.6	0.1	
D	$2,699^2$	290	200	2.10	110.4	110.4	110.6	0.2	
E	$2,805^{2}$	310	380	0.60	111.9	111.9	111.9	0.0	
F	5,386 ²	20	70	2.60	117.5	117.5	117.6	0.0	
Doctors Creek									
	85 ³	65	539	4.00	60.0	00.0	00.0		
A B	985 ³	166		4.00	60.2	60.2	60.3	0.1	
C	1,320 ³		841	2.60	61.1	61.1	61.2	0.1	
	2,170 ³	149	652	3.30	61.4	61.4	61.6	0.2	
D	2,170	266	1,315	1.60	62.2	62.2	62.3	0.1	
E F	2,900 ³	2,320	1,303	1.70	62.5	62.5	62.6	0.1	
	3,830 ³	166	869	2.50	63.0	63.0	63.2	0.2	
G	4,085 ³	295	2,942	0.70	71.9	71.9	71.9	0.0	
Н	4,610 ³	665	5,682	0.40	71.9	71.9	71.9	0.0	
l	5,695 ³	403	2,482	0.90	71.9	71.9	71.9	0.0	
Feet above confluence with De Feet above confluence with De Feet above Fowlers Bridge Roa	ep Run Tributary B	<u>I</u>	<u> </u>	<u> </u>	I			<u> </u>	
FEDERAL EMERGE		IT AGENCY							
FEDERAL EMERGE		YNI			FLOOD	WAY DA	ΤΑ		
	RISDICTIO	•							
		143)							
					UN TRIBUTA	ΚΥ C - DC	ICTORS CR	(FFK	

FLOODING SO	URCE		FLOODWA	Y	V	BASE F VATER-SURFAC (FEET №	CE ELEVATION			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE		
East Creek A B C D E	4,800 ¹ 5,920 ¹ 6,945 ¹ 7,730 ¹ 8,070 ¹	33 173 320 127 103	232 633 890 332 269	4.10 1.50 0.80 2.30 2.80	15.0 16.8 21.9 24.3 25.7	15.0 16.8 21.9 24.3 25.7	15.1 16.9 21.9 24.5 25.8	0.1 0.1 0.0 0.2 0.1		
Flat Creek B C D E F G H I	1,850 ¹ 2,550 ¹ 3,500 ¹ 4,450 ¹ 4,950 ¹ 5,775 ¹ 6,775 ¹ 7,900 ¹ 8,550 ¹	112 225 98 120 50 46 50 96 21	579 909 351 170 218 185 134 441 47	2.30 1.50 3.80 4.10 3.20 3.70 5.20 1.30 8.50	10.8 11.6 14.3 22.4 25.1 26.2 29.8 39.8 39.9	10.8 11.6 14.3 22.4 25.1 26.2 29.8 39.8 39.9	10.9 11.7 14.4 22.4 25.2 26.4 30.0 39.8 39.9	0.1 0.1 0.0 0.1 0.2 0.2 0.0 0.0		
Gander Brook A B	1,637 ² 5,597 ²	100 35	220 40	0.80 4.30	106.0 123.8	106.0 123.8	106.1 123.8	0.1 0.0		
Gravelly Brook A B C D		290 232 201 290	1,545 425 988 577	0.90 2.90 1.30 2.20	21.1 21.2 26.7 23.6	21.1 21.2 26.7 23.6	21.3 21.4 26.9 23.8	0.2 0.2 0.2 0.2		
	latawan Creek ENCY MANAGEMEN TH COUNT				FLOO	DWAY DA	ТА			
(ALL JU	RISDICTIO	•		EAST CREEK – FLAT CREEK – GANDER BROOK – GRAVELLY BROOK						

FLOODING SOU	RCE		FLOODWA	Y	v	BASE F	E ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	(FEET N WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Gravelly Brook (continued) E F G H I	5,000 ¹ 6,092 ¹ 11,976 ¹ 13,549 ¹ 14,077 ¹	217 190 100 24 20	1,316 767 280 123 55	1.00 1.60 3.50 8.10 8.30	26.7 27.1 53.4 66.7 72.6	26.7 27.1 53.4 66.7 72.6	26.9 27.3 53.4 66.7 72.6	0.2 0.2 0.0 0.0 0.0
J K L	14,182 ¹ 16,241 ¹ 19,093 ¹	30 15 10	105 35 15	3.00 2.60 6.00	75.1 91.9 162.2	75.1 91.9 162.2	75.2 91.9 162.2	0.1 0.0 0.0
Gravelly Run A B C D	200 ² 1,040 ² 1,860 ² 2,650 ²	68 16 16 33	151 51 42 69	1.70 4.90 6.00 3.60	19.0 20.0 25.2 32.0	16.2⁴ 20.0 25.2 32.0	16.4 20.2 25.3 32.2	0.2 0.2 0.1 0.2
Groundhog Brook A B C D E F	100 ³ 1,134 ³ 1,142 ³ 1,165 ³ 2,430 ³ 2,970 ³	23 16 50 274 153 23	59 60 252 1,836 572 68	6.40 6.40 1.50 0.20 0.50 4.40	49.0 52.7 57.5 57.6 57.6 57.6	47.4 ⁵ 52.7 57.5 57.6 57.6 57.6	47.6 52.7 57.5 57.6 57.6 57.6	0.2 0.0 0.0 0.0 0.0 0.0
¹ Feet above confluence with Ma ² Feet above confluence with Me ³ Feet above confluence with Ha ⁴ Elevation computed without con	tedeconk River Noi ystack Brook		rom Metedecoi		nputed without consi- anch	deration of backwa	ater effects from H	aystack Brook
FEDERAL EMERGEN					FLOOI	DWAY DA	ТА	
ALL JUI	RISDICTIO	•	G	RAVELLY B	ROOK – GRAV	/ELLY RUN -	GROUNDHO	G BROOK

FLOODING SC	DURCE		FLOODWA	Y	Ŵ	BASE F ATER-SURFAC	CE ELEVATION				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE			
Hannabrand Brook											
A	1,050 ¹	290	1,231	3.10	11.9	11.9	12.1	0.2			
В	2,9461	28	169	7.40	18.0	18.0	18.2	0.2			
C	5,075 ¹	80	209	7.50	24.5	24.5	24.7	0.2			
D	8,150 ¹	277	621	2.10	33.4	33.4	33.6	0.2			
E	11,200 ¹	64	171	4.50	45.3	45.3	45.5	0.2			
F F	12,950 ¹	77	192	4.00	55.7	45.3 55.7	55.9	0.2			
G	15,000 ¹	46	103	7.30	65.6	65.6	65.8	0.2			
Н	16,850 ¹	98	194	4.40	81.2	81.2	81.4	0.2			
Haystack Brook											
Â	55 ²	257	919	1.10	22.0	20.1 ³	20.3	0.2			
В	830 ²	28	169	6.00	22.0	20.5 ³	20.6	0.1			
С	865 ²	30	152	6.70	22.0	20.5 ³	20.6	0.1			
D	2,250 ²	127	348	2.90	23.7	23.7	23.8	0.1			
Ē	3,030 ²	226	413	2.50	24.8	24.8	24.8	0.0			
F	4,235 ²	84	266	3.80	26.9	26.9	27.0	0.1			
Ġ	5,165 ²	67	226	3.70	28.8	28.8	29.0	0.2			
Н	6,025 ²	89	249	3.30	30.3	30.3	30.4	0.1			
1	6,934 ²	28	249	3.30	30.3	30.3	30.4	0.1			
1	6,964 ²	66	183	4.60	31.7	31.7	31.7	0.0			
5 12	8,325 ²	204	520	1.60	34.2	34.2	34.3	0.0			
K	0,325										
L	9,126 ²	28	148	5.60	35.2	35.2	35.3	0.1			
M	9,156 ²	76	176	4.70	35.6	35.6	35.6	0.0			
N	10,865 ²	112	354	2.00	39.0	39.0	39.2	0.2			
0	11,785 ²	198	458	1.60	39.8	39.8	40.0	0.2			
P	12,685 ²	57	156	4.60	42.1	42.1	42.3	0.2			
Q	13,356 ²	27	166	4.40	44.4	44.4	44.6	0.2			
R ¹ Feet above confluence with ³ Elevation computed without FEDERAL EMERC	Metedeconk River Nor	vater effects f	166 rom Metedecor	4.40 hk River North Br			44.6	0.2			
	MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)					FLOODWAY DATA HANNABRAND BROOK – HAYSTACK BROOK					

FLOODING SOL	IRCE		FLOODWA	Y	v	BASE F VATER-SURFAC (FEET N	CE ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Haystack Brook (continued)								
S T U V W X Y Z AA AB AC AD AE AF AG	$\begin{array}{c} 14,176^1 \\ 14,830^1 \\ 14,860^1 \\ 15,640^1 \\ 16,365^1 \\ 17,055^1 \\ 17,845^1 \\ 19,280^1 \\ 20,266^1 \\ 20,296^1 \\ 21,565^1 \\ 22,670^1 \\ 23,493^1 \\ 23,523^1 \\ 24,505^1 \end{array}$	87 27 126 111 164 50 176 111 16 97 99 66 88 38 38 64	234 121 377 278 310 120 371 278 104 161 240 163 281 131 220	$\begin{array}{c} 3.10\\ 6.00\\ 1.80\\ 2.40\\ 2.20\\ 5.60\\ 1.80\\ 2.40\\ 6.50\\ 4.20\\ 2.80\\ 4.10\\ 2.40\\ 5.10\\ 2.10\\ 2.10\end{array}$	46.2 47.3 48.0 49.0 50.1 52.1 54.5 56.3 57.8 58.4 62.0 64.3 68.5 68.5 69.8	46.2 47.3 48.0 49.0 50.1 52.1 54.5 56.3 57.8 58.4 62.0 64.3 68.5 68.5 69.8	46.4 47.5 48.1 49.2 50.3 52.3 54.7 56.4 58.0 58.6 62.0 64.5 68.6 68.6 69.9	0.2 0.2 0.1 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.0 0.2 0.1 0.1 0.1
AH AI AJ	25,905 ¹ 26,875 ¹ 27,785 ¹	56 111 34	141 284 78	3.20 1.60 5.90	70.7 72.1 73.6	70.7 72.1 73.6	70.9 72.3 73.8	0.2 0.2 0.2
Heroys Pond Creek A B C D	230 ² 2,950 ² 4,500 ² 5,100 ²	140 129 112 65	670 836 249 100	0.90 0.80 2.50 6.20	8.9 32.1 34.4 36.7	4.9 ³ 32.1 34.4 36.7	5.1 32.3 34.6 36.9	0.2 0.2 0.2 0.2
¹ Feet above confluence with Me ² Feet above confluence with Sh ³ Minimum elevation for establish	ark River Estuary		is tidal elevatio	on (8.9 feet NAVI))	I	I	<u> </u>
FEDERAL EMERGE					FLOO	OWAY DA	ТА	
(ALL JUI	RISDICTIO	•		HAYSTAC	CK BROOK	– HEROY	S POND	CREEK

	FLOODING SOU	RCE		FLOODWA	Y	v v	BASE F ATER-SURFAC (FEET N	CE ELEVATION	
	CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Hock	nockson Brook								
	A	01	227	1,470	0.80	46.3	46.3	46.5	0.2
	В	280 ¹	99	522	2.30	46.3	46.3	46.5	0.2
	С	1,500 ¹	161	666	1.80	47.0	47.0	47.2	0.2
	D	2,520 ¹	190	571	2.10	47.6	47.6	47.8	0.2
	E	3,000 ¹	74	252	4.70	48.0	48.0	48.2	0.2
	F	3,500 ¹	139	291	4.10	49.7	49.7	49.9	0.2
	G	4,250 ¹	196	577	2.10	51.4	51.4	51.6	0.2
Hoa S	Swamp Brook								
	A	300 ²	80	1,215	0.80	20.5	20.5	20.7	0.2
	В	1,450 ²	65	780	1.20	20.5	20.5	20.7	0.2
	c	2,570 ²	130	1,247	0.80	20.5	20.5	20.7	0.2
	D	3,550 ²	140	573	1.70	21.5	21.5	21.7	0.2
	E	5,280 ²	69	405	2.40	32.5	32.5	32.6	0.1
	F	6,070 ²	115	283	3.40	33.2	33.2	33.4	0.2
	G	6,980 ²	85	254	3.80	35.4	35.4	35.4	0.0
	H	8,560 ²	190	406	2.40	41.9	41.9	42.1	0.0
	11	9,140 ²	170	400	1.40	43.3	43.3	43.4	0.2
	1	9,840 ²	100	172	4.00	43.4	43.4	43.6	0.1
	K	11,220 ²	280	539	1.30	43.4	48.6	43.0	0.2
		12,320 ²	55	101	6.70	49.9	48.0	49.9	0.1
	<u>نــــــــــــــــــــــــــــــــــــ</u>	12,320 12,710 ²	55	141		49.9 51.0		49.9 51.2	0.0
	M	13,910 ²			4.80		51.0		
	N		70	100	4.40	54.6	54.6	54.8	0.2
	O P	14,930 ²	150	982	0.50	63.5	63.5	63.5	0.0
	F	16,410 ²	91	232	1.90	63.5	63.5	63.6	0.1
	above confluence with Pin above confluence with Dea		L	<u></u>					
<u>-</u> T	FEDERAL EMERGEN		T AGENCY			<u>, 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 1997 - 199</u>			
TABL						FLOO	WAY DA	TA	
	MONMOUT		•					<u> </u>	
12	(ALL JUF	RISDICTIO	но	оскнос	KSON BRO	OK – HO	G SWAMP	BROOK	

		<u></u>							
	FLOODING SOUR	CE		FLOODWA	Y	W	BASE FI ATER-SURFAC (FEET N	E ELEVATION	
	CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Holl	ow Brook						· · · · · · · · · · · · · · · · · · ·		
	A	0 ¹	177	827	0.80	13.2	13.2	13.4	0.2
	B	1,140 ¹	118	1,461	0.30	16.3	16.3	16.5	0.2
	Ċ	2,348 ¹	272	836	0.80	20.2	20.2	20.4	0.2
	D	5,538 ¹	90	205	2.30	34.3	34.3	34.5	0.2
	Ē	6,748 ¹	24	56	8.60	45.2	45.2	45.2	0.0
	F	6,948 ¹	35	82	5.80	47.1	47.1	47.2	0.1
	G	7,148 ¹	39	86	5.60	48.1	48.1	48.1	0.0
	н	7,488 ¹	29	58	8.10	50.9	50.9	50.9	0.0
	1	7,588 ¹	645	2,186	0.20	51.9	51.9	52.1	0.2
	1	7,948 ¹	163	112	4.70	52.9	52.9	53.1	0.2
	J	8,308 ¹							
	К	8,308	164	144	3.40	55.7	55.7	55.9	0.2
Indi	an Run								
		370 ²	145	521	2.00	67.9	67.9	68.1	0.2
	A	1,220 ²	38	294	3.50	69.5			
	В	1,220					69.5	69.7	0.2
	С	$1,320^2$	275	2,821	0.40	80.4	80.4	80.5	0.1
	D	2,440 ²	174	1,026	1.00	81.1	81.1	81.3	0.2
	E	3,480 ²	147	613	1.70	82.8	82.8	83.0	0.2
1									
Jua	as Creek (Upstream Reach)	200 ³	400	1.070	0.00	00.4	00.4	00.0	
	A	500 ³	138	1,078	0.20	29.1	29.1	29.3	0.2
	B	500	102	550	0.40	29.1	29.1	29.3	0.2
	C	830 ³	50	44	5.40	29.3	29.3	29.3	0.0
	D	1,050 ³	102	108	2.20	31.1	31.1	31.1	0.0
	E	1,600 ³	60	47	5.10	35.2	35.2	35.2	0.0
	F	2,200 ³	52	72	3.30	39.9	39.9	40.0	0.1
² Fe	et above mouth et above county boundary et above limit of detailed study	/ (limit of detailed	study is appro	oximately 900 fe	et below State H	lighway 35)			
-1	FEDERAL EMERGEN		T AGENCY						
TABLE	MONMOUT	H COUNT	Y. NJ			FLOOI	OWAY DA	TA	
E 12	(ALL JURISDICTIONS)				HOLLOW	BROOK – IN (UPSTR	IDIAN RUN REAM REAG		CREEK

FLOODING SO	JRCE		FLOODWA		BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
umping Brook 1	-							
A	0 ¹	318	614	2.70	16.0	16.0 ³	16.1	0.1
В	565 ¹	312	1,787	0.50	16.6	16.6	16.7	0.1
С	885 ¹	215	1,096	0.70	17.6	17.6	17.7	0.1
D	1,615 ¹	100	705	1.10	22.2	22.2	22.2	0.0
E	2,365 ¹	105	409	1.90	22.3	22.3	22.4	0.1
F	2,955 ¹	122	.463	1.60	24.1	24.1	24.3	0.2
G	3,985 ¹	96	445	1.70	27.6	27.6	27.8	0.2
Н	4,465 ¹	70	194	3.80	29.4	29.4	29.6	0.2
ł	5,335 ¹	65	313	2.20	32.7	32.7	32.9	0.2
J	5,556 ¹	130	1,019	0.70	39.3	39.3	39.3	0.0
ĸ	7,466 ¹	109	239	2.40	40.7	40.7	40.8	0.0
L	8,006 ¹	121	574	1.00	44.1	44.1	44.3	0.2
Μ	8,966 ¹	101	462	1.20	44.6	44.6	44.8	0.2
umping Brook 2								
А	0 ²	130	1,335	2.00	8.9	8.9	9.1	0.2
В	500 ²	580	2,225	1.20	8.9	8.9	9.1	0.2
С	2,850 ²	325	1,035	2.60	8.9	8.9	9.1	0.2
Ð	3,360 ²	120	490	5.40	10.3	10.3	10.5	0.2
E	3,900 ²	135	700	3.80	11.7	11.7	11.9	0.2
F	$4,400^2$	255	1,035	2.60	12.4	12.4	12.6	0.2
G	4,750 ²	280	765	3.55	12.9	12.9	13.1	0.2
Н	5,150 ²	50	245	10.90	15.1	15.1	15.2	0.1
I	5,400 ²	50	250	10.70	17.9	17.9	18.0	0.1
J	5,700 ²	130	430	6.20	21.3	21.3	21.5	0.2
ĸ	5,904 ²	328	1,057	2.90	25.4	25.4	25.6	0.2
L	6,300 ²	150	897	3.00	25.5	25.5	25.7	0.2
Feet above Private Drive (Ups Feet above mouth	tream Face) (Private	drive is appr	oximately 775 f	eet below West F	Front Street)			
Elevation computed without co	onsideration of backw	ater effects f	rom Nut Swamp	o Brook				
FEDERAL EMERGE	NCY MANAGEMEN	T AGENCY						
MONMOU	TH COUNT	Y. NJ			FLOOD	WAY DA	ТА	
	RISDICTIO	•		JUMPIN	IG BROOK	1 – JUMP		OK 2

FLOODING SOU	JRCE		FLOODWA	Y	v	BASE FI ATER-SURFAC (FEET N	E ELEVATION	
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
umping Brook 2 continued)								
M	6,700	348	1,732	1.60	25.7	25.7	25.9	0.2
N	6,900	116	569	4.70	25.7	25.7	25.9	0.2
0	8,052	386	581	4.60	27.2	27.2	27.4	0.2
Р	8,972	205	1,834	1.40	38.3	38.3	38.3	0.0
Q	9,100	317	3,924	0.80	38.3	38.3	38.3	0.0
R	10,050	390	3,086	0.80	38.3	38.3	38.3	0.0
S	11,400	132	835	2.80	38.3	38.3	38.3	0.0
Т	12,600	123	454	5.20	38.7	38.7	38.7	0.0
U	13,100	87	249	9.60	39.6	39.6	39.8	0.2
V	13,500	99	554	4.10	42.3	42.3	42.5	0.2
W	13,620	283	3,287	0.80	52.2	52.2	52.4	0.2
X	15,000	225	1,778	1.40	52.2	52.2	52.4	0.2
Y	15,700	199	1,255	1.80	52.2	52.2	52.4	0.2
Z	17,100	176	822	2.70	52.4	52.4	52.6	0.2
AA	18,500	170	316	7.40	54.2	54.2	54.4	0.2
AB	18,900	67	207	8.70	57.0	57.0	57.2	0.2
AC	19,720	95	313	5.70	60.8	60.8	61.0	0.2
AD	19,940	187	2,340	0.80	71.4	71.4	71.5	0.1
AE	19,990	240	3,858	0.00	71.4	71.4	71.5	0.1
AF	21,220	140	3,200	0.00	71.4	71.4	71.5	0.1
AG	21,470	250	2,013	0.90	71.4	71.4	71.6	0.2
AH	22,125	350	2,704	0.70	71.4	71.4	71.6	0.2
Ał	22,240	350	2,070	0.90	71.4	71.4	71.6	0.2
AJ	23,230	319	1,115	1.10	72.1	72.1	72.2	0.1
AK	23,620	289	808	1.60	72.2	72.2	72.3	0.1
AL	24,495	84	322	3.90	73.9	73.9	74.0	0.1

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MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)

FEDERAL EMERGENCY MANAGEMENT AGENCY

FLOODWAY DATA

JUMPING BROOK 2

EL OOF			FLOODWA	~		BASE F			
FLOOL	ING SOURCE		FLOODWA	.T	WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SEC		WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Jumping Brook 2 (continued) AM AN AO AP	25,070 25,770 26,605 27,220	172 109 83 150	688 370 244 2,076	1.80 4.10 5.20 0.60	78.1 78.7 82.2 90.1	78.1 78.7 82.2 90.1	78.1 78.9 82.3 90.1	0.0 0.2 0.1 0.0	
Little Silver Creek A B C ² D E F G H	9,480 9,980 10,479 10,980 11,580 12,080 12,680 13,180	35 150 46 100 268 140 99 110	204 449 283 487 683 363 224 280	0.4 1.7 2.7 1.6 1.1 2.1 3.4 2.4	8.0 8.2 11.3 11.5 11.6 11.9 12.9 13.8	8.0 8.2 11.3 11.5 11.6 11.9 12.9 13.8	8.0 8.4 11.3 11.6 11.8 12.1 13.1 14.0	0.0 0.2 0.0 0.1 0.2 0.2 0.2 0.2 0.2	
Little Silver Tributary A	A 140	113	240	0.04	12.9	12.9	13.1	0.2	
¹ Feet above mouth ² Data at bridge culve	erts reflect conditions on upstre	am side of bri	dge						
	EMERGENCY MANAGEMEN				FLOOI	DWAY DA	TA		
(ALL JURISDICTIONS)				JUMPIN	G BROOK 2 LITTLE SIL			EEK –	

	FLOODING SOU	JRCE		FLOODWA	Y	Ŵ	BASE F ATER-SURFAC (FEET N	CE ELEVATION	
	CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Little	e Silver Tributary 1				SLOOND)				
	A^2	1,830	30	142	0.9	10.4	10.4	10.4	0.0
	В	1,910	10	55	2.2	10.4	10.4	10.5	0.1
	C^2	2,020	152	334	0.4	11.1	11.1	11.3	0.2
	D	2,450	28	60	2.0	11.1	11.1	11.3	0.2
	E F ²	2,850	91	91	1.3	11.9	11.9	12.1	0.2
	F^2	3,150	33	142	0.9	14.9	14.9	15.1	0.2
ittl2	e Silver Tributary 2								
	A	3,270	138	232	1.6	8.4	8.4	8.5	0.1
		3,570	55	126	2.8	8.9	8.9	9.0	0.1
	B C ²	3,870	115	250	1.4	12.9	12.9	13.1	0.2
	D	4,270	140	501	0.7	13.0	13.0	13.2	0.2
	E	4,670	80	235	1.5	13.1	13.1	13.3	0.2
ĺ	F	5,070	102	199	1.8	13.5	13.5	13.7	0.2
	G	5,450	56	91	2.4	13.7	13.7	13.8	0.1
	\tilde{H}^2	5,950	40	81	2.7	18.7	18.7	18.8	0.1
	1	6,350	53	59	3.7	19.1	19.1	19.3	0.1
	J	6,850	42	64	3.4	22.4	22.4	22.5	0.2
1 1441									
	e Silver Tributary 2A	500	20	50	0.5				
	A	500	30	56	2.5	14.1	14.1	14.4	0.3
1	B C	1,220 1,820	38 38	37 37	3.1 3.1	15.9 19.5	15.9 19.5	16.0 19.7	0.1 0.2
	Ū	1,020			5.1	19.0	19.5	19.7	0.2
	et above mouth								
Dai	ta at bridge culverts reflect	conditions on upstrea	ann Siùe ot Dri	lage					
					··· <u>···</u> ·····				
<u> </u>	FEDERAL EMERGE	NCY MANAGEMEN	T AGENCY						
⊼							DWAY DA	ТΔ	
BL									
	MONMOU	TH COUNT	Y, NJ						
						LVER TRIBUTAR			

			1						
	FLOODING SOUF	RCE		FLOODWA	Y	v	BASE F ATER-SURFAΩ FEET №	CE ELEVATION	
	CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Little	e Silver Tributary 2B A ¹ B	400 ² 450 ²	101 101	656 656	0.04 0.04	25.1 25.1	25.1 25.1	25.2 25.2	0.1 0.1
Long	g Brook A B C D E F G H I J K L M N O P Q R S T	570^3 $1,075^3$ $1,938^3$ $1,968^3$ $2,665^3$ $3,913^3$ $3,948^3$ $5,017^3$ $5,047^3$ $6,065^3$ $6,781^3$ $6,811^3$ $7,535^3$ $8,255^3$ $8,285^3$ $8,450^3$ $9,440^3$ $10,589^3$ $10,619^3$ $11,615^3$	20 47 95 28 36 16 104 41 153 100 25 63 116 114 26 35 43 98 61 20	72 112 447 129 118 92 547 171 570 344 90 208 324 216 72 118 111 204 206 73	$\begin{array}{c} 8.30\\ 5.30\\ 1.30\\ 4.60\\ 5.00\\ 6.50\\ 1.10\\ 3.50\\ 1.00\\ 1.70\\ 6.60\\ 2.90\\ 1.60\\ 2.40\\ 7.20\\ 4.40\\ 4.70\\ 2.50\\ 2.50\\ 5.00\end{array}$	74.0 78.8 85.5 85.5 86.7 93.4 94.3 96.5 96.7 97.0 98.0 99.0 99.7 101.1 101.1 101.1 101.1 101.8 104.0 108.0 108.2 110.0	74.0 78.8 85.5 85.5 86.7 93.4 94.3 96.5 96.7 97.0 98.0 99.0 99.7 101.1 101.1 101.1 101.8 104.0 108.0 108.2 110.0	74.2 78.8 85.7 85.7 86.9 93.5 94.4 96.5 96.9 97.2 98.2 99.0 99.9 101.3 101.3 102.0 104.2 108.2 108.3 110.1	$\begin{array}{c} 0.2 \\ 0.0 \\ 0.2 \\ 0.2 \\ 0.2 \\ 0.1 \\ 0.1 \\ 0.0 \\ 0.2 \\ 0.2 \\ 0.2 \\ 0.2 \\ 0.2 \\ 0.2 \\ 0.2 \\ 0.2 \\ 0.2 \\ 0.2 \\ 0.2 \\ 0.2 \\ 0.1 \\ 0.1 \end{array}$
² Fee	ta at bridge culverts reflect co et above mouth et above confluence with Mar	-	am side of bri	dge					
	FEDERAL EMERGENCY MANAGEMENT AGENCY					FLOO	OWAY DA	ТА	
= 12	(ALL JURISDICTIONS)			L	ITTLE SI		UTARY 2	B - LONG	BROOK

FLOODING SOU	RCE	FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
ong Brook									
continued)									
U	12,510 ¹	52	141	2.60	111.4	111.4	111.6	0.2	
V	13,429 ¹	42	77	4.70	113.6	113.6	113.7	0.1	
W	13,459 ¹	25	66	5.50	113.6	113.6	113.8	0.2	
X	14,615 ¹	21	33	10.80	116.3	116.3	116.3	0.0	
Y	14,645 ¹	70	224	1.60	118.2	118.2	118.2	0.0	
Z	15,664 ¹	38	135	2.70	118.7	118.7	118.8	0.1	
AA	16,669 ¹	37	80	4.50	120.1	120.1	120.3	0.2	
AB	16,989 ¹	74	165	2.20	122.4	122.4	122.4	0.0	
AC	17,019 ¹	59	165	2.20	122.4	122.4	122.4	0.0	
AD	17,354 ¹	34	117	3.10	124.1	124.1	124.2	0.1	
AE	17,384 ¹	57	211	1.70	124.2	124.2	124.3	0.1	
Mahoras Brook									
Α	500 ²	94	197	7.20	16.9	16.9	17.0	0.1	
В	1,900 ² 2,900 ²	380	1,052	1.40	21.7	21.7	21.9	0.2	
С	2.900^{2}	175	245	5.80	24.0	24.0	24.1	0.1	
D	3,520 ²	285	704	2.00	27.3	27.3	27.5	0.2	
Ē	3,900 ²	150	272	5.20	29.5	29.5	29.6	0.1	
F	4,300 ²	260	613	2.30	32.4	32.4	32.6	0.2	
G	4,588 ²	56	350	3.30	35.4	35.4	35.6	0.2	
н	4,690 ²	292	540	2.10	35.5	35.5	35.6	0.1	
1	6,330 ²	423	353	3.20	37.1	37.1	37.2	0.1	
,	7,460 ²	183	217	5.30	42.7	42.7	42.9	0.2	
ĸ	8,180 ²	298	3,063	0.30	55.6	55.6	55.6	0.2	
	10,510 ²	185	470	1.40	58.6	58.6	58.8	0.0	
L.	11,580 ²								
M Feet above confluence with Ma Feet above confluence with Wa	nasquan River	106	222	3.10	61.2	61.2	61.2	0.0	
	FEDERAL EMERGENCY MANAGEMENT AGENCY MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)						ТА		

MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)

12

LONG BROOK – MAHORAS BROOK

FLOODING SC	FLOODING SOURCE			FLÓODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE			
Mahoras Brook											
continued)	40.0701	400	450		07.7	· · · ·					
N	12,070 ¹	100	156	4.30	67.7	67.7	67.7	0.0			
0	13,935 ¹	226	310	2.00	79.8	79.8	79.8	0.0			
P	15,140 ¹	68	168	3.80	86.0	86.0	86.2	0.2			
Q	16,780 ¹	118	248	2.60	100.5	100.5	100.7	0.2			
Manalapan Brook											
А	945 ²	436	1,369	1.30	76.3	76.3	76.4	0.1			
В	1,968 ²	484	1,241	1.40	76.6	76.6	76.7	0.1			
c	2,211 ²	463	1,372	1.30	77.6	77.6	77.7	0.1			
D	3,654 ²	476	1,104	1.60	78.0	78.0	78.1	0.1			
Ē	4,967 ²	247	469	3.80	79.2	79.2	79.2	0.0			
E	5,138 ²	142	533	3.10	81.1	81.1	81.2	0.0			
G	6,079 ²	384	1,003	1.60	82.3	82.3	82.3	0.1			
H	7,165 ²	450	1,003	1.40	83.0						
17	8,363 ²					83.0	83.0	0.0			
1	0,303	510	1,177	1.30	84.2	84.2	84.2	0.0			
J	9,705 ²	445	1,023	1.50	85.9	85.9	85.9	0.0			
K	10,978 ²	509	1,649	1.00	86.5	86.5	86.5	0.0			
L	12,507 ²	516	686	2.30	87.6	87.6	87.7	0.1			
M	13,526 ²	707	1,040	1.50	89.5	89.5	89.5	0.0			
Ν	14,424 ²	265	465	3.40	91.0	91.0	91.0	0.0			
0	14,597 ²	82	326	3.90	92.3	92.3	92.3	0.0			
Р	15,657 ²	491	1,365	0.70	93.6	93.6	93.6	0.0			
Q	17,126 ²	318	736	1.40	94.5	94.5	94.5	0.0			
R	17,277 ²	280	765	1.30	94.6	94.6	94.7	0.1			
S	18,423 ²	327	706	1.40	95.8	95.8	95.9	0.1			
Т	19,457 ²	70	273	3.70	96.7	96.7	96.8	0.1			
Feet above confluence with V Feet above county boundary	l Vaackaack Creek	l		<u> </u>	· · · · · · · · · · · · · · · · · · ·						
					FLOOD	DWAY DA	TA				
MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)				MAHORAS BROOK – MANALAPAN BROOK							

								·····
FLOODING S	OURCE		FLOODWA	Y	v	BASE F ATER-SURFAC/ FEET N	CE ELEVATION	
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Manalapan Brook								
(continued) U	20.222	160	200	2.50	99.1	99.1	99.1	0.0
V	20,223	160	399	2.50 2.00	99.1	99.1 99.7	99.1	0.0
Ŵ	20,835	217 202	506 401	2.50	101.2	101.2	101.3	0.1
X	22,023 22,677	71	373	2.50	101.2	101.2	101.3	0.1
Ŷ	22,998	281	1,915	1.20	113.2	113.2	113.2	0.0
Z	22,998	201	1,585	0.70	113.2	113.2	113.2	0.0
AA	24,032	244 243	1,585	0.70	113.3	113.2	113.2	0.0
AA AB	25,239	243	481	2.10	113.4	113.3	113.4	0.0
AC	26,815	206	587	1.40	113.4	113.4	113.4	0.0
AD	27,870	200	532	1.60	114.8	114.8	114.8	0.0
AE	28,991	92	308	2.70	118.5	118.5	118.5	0.0
AF	29,165	148	525	1.60	119.5	119.5	119.5	0.0
AG	29,105	348	1,031	0.80	119.8	119.8	119.8	0.0
AH	31,299	234	375	1.70	120.9	120.9	120.9	0.0
Al	32,499	234	672	0.90	120.9	120.9	120.9	0.0
AJ	33,605	139	173	3.60	125.0	125.0	125.0	0.0
AJ	33,605	139	412	1.50	125.0	123.0	123.0	0.0
AL	36,479	28	94	4.00	129.9	129.9	120.0	0.0
AM	36,677	306	1,623	1.20	136.7	136.7	136.7	0.0
AN	37,734	316	869	0.50	136.9	136.9	136.9	0.0
AO	38,552	112	170	2.20	137.3	130.9	137.3	0.0
AD	39,661	102	199	1.90	140.6	140.6	140.6	0.0
AC	40,305	118	236	1.60	140.0	140.0	140.0	0.0
AQ	40,305	44	181	2.10	142.1	143.2	143.2	0.0
AS	40,485	95	255	1.50	143.2	143.6	143.6	0.0
AT	40,890	153	233	1.50	145.3	145.3	145.3	0.0
Feet above county boundar	y							
FEDERAL EMER	GENCY MANAGEMEN	T AGENCY						
MONMO	MONMOUTH COUNTY, NJ				FLOOI	DWAY DA	TA	
(ALL JURISDICTIONS)					MANAL	APAN BR	ΟΟΚ	

FLOC	DING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION						
					(FEET NAVD)							
CROSS SE	CTION DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE				
Manalapan Brook (continued)												
AU AV AW AX AZ BA BB BC BD BE BF BG	42,611 ¹ 43,974 ¹ 45,097 ¹ 46,091 ¹ 46,216 ¹ 47,225 ¹ 48,259 ¹ 48,945 ¹ 49,288 ¹ 50,034 ¹ 51,071 ¹ 51,704 ¹ 52,735 ¹	110 131 157 35 149 110 227 290 130 150 70 79 38	210 256 290 112 829 439 688 599 127 247 105 133 29	$ \begin{array}{r} 1.80\\ 1.50\\ 1.30\\ 2.70\\ 0.40\\ 0.70\\ 0.40\\ 0.50\\ 2.40\\ 1.20\\ 2.90\\ 2.30\\ 1.60\\ \end{array} $	113.4 114.8 116.5 118.5 119.5 119.8 120.9 123.0 125.0 164.4 167.2 170.6 176.2	113.4 114.8 116.5 118.5 119.5 119.8 120.9 123.0 125.0 164.4 167.2 170.6 176.2	113.4 114.8 116.6 118.5 119.6 119.9 121.0 123.0 125.0 164.5 167.3 170.8 176.3	0.0 0.0 0.1 0.0 0.1 0.1 0.1 0.0 0.0 0.1 0.1				
BH Bl Manalapan Brook ⊺ ∧	52,902 ¹ 53,335 ¹ Fributary A	51 10 120	97 12 110	0.50 3.80 2.60	178.9 181.1 91.7	178.9 181.1 91.7	178.9 181.2 91.8	0.0 0.1				
A B C D E F	7,181 ² 8,237 ² 9,134 ² 10,296 ² 14,942 ²	100 100 100 60 50	200 200 150 150 50	1.50 1.40 1.90 1.50 4.60	91.7 108.3 109.8 112.3 117.3 131.7	91.7 108.3 109.8 112.3 117.3 131.7	91.8 108.3 109.9 112.3 117.5 131.7	0.1 0.0 0.1 0.0 0.2 0.0				
Feet above county Feet above conflue	/ boundary ence with Manalapan Brook											
	IL EMERGENCY MANAGEM				FLOOI	OWAY DA	ТА					
(ALL JURISDICTIONS)				MA		MANALAPAN BROOK – MANALAPAN BROOK TRIBUTARY A						

FLOO	FLOODING SOURCE			FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SE	CTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE		
Manalapan Brook	Tributary B	·····			02001.07						
A	,	264 ¹	140	70	2.20	123.6	123.6	123.7	0.1		
В		528 ¹	140	140	1.10	124.8	124.8	125.0	0.2		
С		2,746 ¹	20	40	3.90	132.7	132.7	132.8	0.1		
D		5,861 ¹	20	50	3.30	147.1	147.1	147.2	0.1		
Manasquan River											
A		12 ²	81	532	4.20	43.3	43.3	43.5	0.2		
B		44 ²	59	373	4.20 6.00	43.3 43.3	43.3 43.3				
Б С		7852	59 164	636	3.50	43.3 44.1		43.5	0.2		
D		1,685 ²	50				44.1	44.3	0.2		
E		3,245 ²		327	6.80	44.9	44.9	45.1	0.2		
F		3,245 4,035 ²	68 70	397	5.60	47.3	47.3	47.4	0.1		
		4,035	70	402	5.60	48.2	48.2	48.3	0.1		
G		4,835 ²	50	388	5.80	49.1	49.1	49.2	0.1		
Н		5,346 ²	94	608	3.70	49.8	49.8	49.9	0.1		
I.		5,396 ²	69	457	4.90	49.8	49.8	49.9	0.1		
J		6,635 ²	52	292	7.70	51.2	51.2	51.2	0.0		
K		7,490 ²	44	301	7.40	53.5	53.5	53.5	0.0		
L		8,340 ²	95	520	4.30	55.2	55.2	55.2	0.0		
M		9,280 ²	273	917	2.40	55.8	55.8	55.8	0.0		
N		10,200 ²	123	550	3.60	56.8	56.8	56.9	0.1		
0		12,330 ²	319	1,257	1.60	58.0	58.0	58.2	0.2		
Р		13,232 ²	105	753	2.69	58.5	58.5	58.7	0.2		
Q		13,282 ²	47	394	5.00	58.5	58.5	58.7	0.2		
R		13,950 ²	50	367	5.40	58.9	58.9	59.1	0.2		
S		15,410 ²	50	395	5.00	60.2	60.2	60.4	0.2		
Т		16,585 ²	55	465	4.30	61.1	61.1	61.3	0.2		
U		17,605 ²	51	420	4.70	61.7	61.7	61.8	0.1		
¹ Feet above conflu Feet from Southar		 alapan Brook					<u> </u>				
FEDER/	L EMERGEN	CY MANAGEMEN	T AGENCY								
	MONMOUTH COUNTY, NJ					FLOOD	OWAY DA	ТА			
(ALL JURISDICTIONS)					MANALAPAN BROOK TRIBUTARY B – MANASQUAN RIVER						

FLOODING SOL	JRCE		FLOODWA	Y	BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Ianasquan River				<i>(</i>					
continued)									
V	18,425	51	371	5.30	62.2	62.2	62.4	0.2	
W	19,230	45	333	5.90	63.0	63.0	63.2	0.2	
X	20,190	70	463	3.90	64.1	64.1	64.3	0.2	
Y	20,990	61	511	3.50	64.4	64.4	64.6	0.2	
Z	21,585	57	389	4.60	64.6	64.6	64.7	0.1	
AA	22,515	77	560	3.20	65.8	65.8	66.0	0.2	
AB	22,565	55	378	4.70	65.8	65.8	66.0	0.2	
AC	23,225	48	332	5.40	66.2	66.2	66.4	0.2	
AD	23,890	77	.370	4.80	67.0	67.0	67.2	0.2	
AE	24,990	53	388	4.60	68.1	68.1	68.3	0.2	
AF	26,755	75	450	4.00	69.3	69.3	69.5	0.2	
AG	28,096	83	456	2.80	70.4	70.4	70.6	0.2	
AH	28,146	48	264	6.00	70.4	70.4	70.6	0.2	
AI	29,265	66	335	4.70	71.9	71.9	72.0	0.1	
AJ	30,240	40	261	6.10	73.0	73.0	73.2	0.2	
AK	31,340	70	530	3.00	74.0	74.0	74.2	0.2	
AL	33,180	45	279	5.20	74.7	74.7	74.8	0.1	
AM	34,820	247	764	1.90	76.5	76.5	76.7	0.2	
AN	35,650	396	1,166	0.80	76.8	76.8	77.0	0.2	
AO	36,450	43	147	6.00	76.8	76.8	77.0	0.2	
AP	37,330	42	145	6.10	78.9	78.9	78.9	0.0	
AQ	38,400	37	169	4.40	79.8	79.8	79.8	0.0	
AR	39,000	32	128	5.80	80.7	80.7	80.7	0.0	
AS	39,760	31	144	5.20	82.4	82.4	82.4	0.0	
AT	40,630	31	105	6.00	83.4	83.4	83.4	0.0	
AU	41,155	31	130	4.90	84.8	84.8	84.8	0.0	

¹Feet from Southard Road

TABLE

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FEDERAL EMERGENCY MANAGEMENT AGENCY

FLOODWAY DATA

MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)

MANASQUAN RIVER

FLOODING SOU	RCE	FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
anasquan River ontinued)			· · · · · ·	C					
ÁV	41,205 ¹	16	68	9.30	84.8	84.8	84.8	0.0	
AW	41,650 ¹	43	182	3.50	86.3	86.3	86.4	0.1	
AX	41,950 ¹	39	164	3.90	86.5	86.5	86.6	0.1	
AY	42,000 ¹	46	181	3.50	86.6	86.6	86.7	0.1	
AZ	42,465 ¹	42	103	6.20	87.3	87.3	87.4	0.1	
BA	42,495 ¹	45	147	4.30	87.3	87.3	87.4	0.1	
BB	43,970 ¹	26	88	7.20	91.0	91.0	91.0	0.0	
BC	44,480 ¹	44	116	3.30	93.3	93.3	93.3	0.0	
anasquan River Tributary A									
A	130 ²	34	126	5.40	92.0	92.0	92.2	0.2	
В	1,325 ²	33	154	4.40	98.1	98.1	98.1	0.0	
С	2,310 ²	32	96	7.10	101.7	101.7	101.7	0.0	
D	3,220 ²	28	72	9.50	105.7	105.7	105.7	0.0	
E	3,250 ²	28	120	5.70	107.1	107.1	107.1	0.0	
F	3,620 ²	42	119	5.80	109.1	109.1	109.1	0.0	
G	4,350 ²	57	204	3.40	113.8	113.8	113.9	0.1	
Н	5,175 ²	45	132	5.20	117.2	117.2	117.4	0.2	
anasquan River Tributary B									
A	430 ²	16	55	8.30	82.8	82.8	83.0	0.2	
В	1,160 ²	20	70	6.60	88.9	88.9	88.9	0.0	
С	1,620 ²	20	79	5.80	91.9	91.9	92.0	0.1	
D	2,015 ²	31	110	4.20	93.8	93.8	94.0	0.2	
E	2,820 ²	32	73	6.20	96.4	96.4	96.5	0.1	
F	3,470 ²	111	190	2.40	99.9	99.9	100.1	0.2	
eet from Southard Road eet above confluence with Mar		1	L		I	L		<u></u>	

FEDERAL EMERGENCY MANAGEMENT AGENCY MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)

TABLE

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FLOODWAY DATA

MANASQUAN RIVER – MANASQUAN RIVER TRIBUTARY A – MANASQUAN RIVER TRIBUTARY B

FLOODING SOU	RCE		FLOODWA	Y	V	BASE F /ATER-SURFAC (FEET N	CE ELEVATION		
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Manasquan River Tributary B (continued)									
G	4,490	42	148	3.10	105.2	105.2	105.4	0.2	
н	4,520	163	354	1.30	105.5	105.5	105.7	0.2	
1	5,160	20	66	7.00	106.5	106.5	106.7	0.2	
, I	5,905	513	1,010	0.50	110.9	110.9	111.1	0.2	
K	5,935	206	477	1.00	111.2	111.2	111.3	0.2	
	6,965	15	71	6.50	111.2	114.7	114.8	0.1	
L M	6,995	24	94	4.90	114.7	115.2	115.3	0.1	
N		13	58	7.90	115.2	117.0	117.2	0.1	
	7,460				117.0	118.1	118.2		
0	7,490	25 17	85	5.40				0.1	
P	8,185		62	7.40	121.7	121.7	121.7	0.0	
Q R	9,000	30	334	1.40	129.2	129.2	129.4 129.4	0.2	
ĸ	9,030	28	155	2.90	129.2	129.2	129.4	0.2	
lanasquan River Tributary C									
А	80	135	120	5.50	79.8	79.8	80.0	0.2	
В	620	19	83	7.90	82.2	82.2	82.4	0.2	
С	1,126	68	189	3.50	85.8	85.8	85.8	0.0	
D	1,156	22	138	4.70	85.9	85.9	85.9	0.0	
E	1,811	117	121	5.40	88.4	88.4	88.4	0.0	
F	2,510	115	311	2.10	90.4	90.4	90.5	0.1	
G	3,520	94	273	2.40	96.7	96.7	96.9	0.2	
Н	4,435	68	237	2.80	101.1	101.1	101.3	0.2	
1	4,465	56	182	3.60	101.1	101.1	101.3	0.2	
J	5,010	73	236	2.80	102.5	102.5	102.7	0.2	
К						102.6	102.7	0.1	
Ĺ									
Feet above confluence with Ma FEDERAL EMERGEN	5,040 5,670 nasquan River	56 33	230 191 118	2.80 3.40 5.60	102.5 102.6 104.2	102.5 102.6 104.2			
FEDERAL EMERGENCY MANAGEMENT AGENCY MONMOUTH COUNTY, NJ				FLOODWAY DATA					
(ALL JURISDICTIONS)				MA	NASQUAN F		BUTARY B -		

MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)

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FLOODWAY DATA

MANASQUAN RIVER TRIBUTARY B -**MANASQUAN RIVER TRIBUTARY C**

FLOODING SOU	JRCE		FLOODWA	Y	l v	BASE F ATER-SURFAC/ FEET N	E ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Manasquan River Tributary C (continued)			· · /					
M N O P Q	6,290 ¹ 7,030 ¹ 7,060 ¹ 7,903 ¹ 9,165 ¹	32 16 33 247 57	121 84 134 634 150	5.40 7.80 4.90 1.00 4.40	106.1 108.3 109.3 110.8 115.2	106.1 108.3 109.3 110.8 115.2	106.1 108.5 109.4 110.9 115.4	0.0 0.2 0.1 0.1 0.2
Marl Brook								
A B C	1,660 ² 2,590 ² 4,930 ²	70 72 70	149 110 166	3.90 4.30 3.10	71.2 74.6 82.4	71.2 74.6 82.4	71.2 74.6 82.6	0.0 0.0 0.2
Matawan Creek								
A B C D E F G H I J K L M	$19,674^{3}$ $19,907^{3}$ $21,098^{3}$ $22,229^{3}$ $23,026^{3}$ $23,917^{3}$ $24,885^{3}$ $26,360^{3}$ $26,466^{3}$ $27,522^{3}$ $27,628^{3}$ $28,684^{3}$ $29,951^{3}$	333 402 378 286 179 165 107 120 140 90 220 300 215	2,488 3,273 2,351 302 411 717 178 530 645 95 730 345 175	$ \begin{array}{r} 1.00\\ 0.60\\ 0.70\\ 4.40\\ 3.20\\ 1.40\\ 5.50\\ 0.90\\ 0.70\\ 4.30\\ 0.60\\ 1.00\\ 1.50\\ \end{array} $	18.9 18.9 19.1 24.4 25.3 30.2 39.6 40.3 48.3 50.8 51.0 66.9	18.9 18.9 19.1 24.4 25.3 30.2 39.6 40.3 48.3 50.8 51.0 66.9	19.0 19.0 19.3 24.6 25.5 30.4 39.6 40.3 48.3 50.9 51.1 67.1	0.1 0.1 0.2 0.2 0.2 0.2 0.2 0.0 0.0 0.0 0.0 0.1 0.1 0.2
¹ Feet above confluence with M ² Feet above confluence with M ³ Feet above mouth						<u> </u>		
	FEDERAL EMERGENCY MANAGEMENT AGENCY				FLOOI	DWAY DA	ТА	
(ALL JURISDICTIONS)				ANASQU	AN RIVER TR MATA	NIBUTARY (WAN CREE		BROOK –

FLOODING SO	JRCE		FLOODWA	Y	v	BASE F ATER-SURFAC (FEET N	CE ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Matchaponix Brook		······	/					· · · · · · · · · · · · · · · · · · ·
A B C D	581 ¹ 1,531 ¹ 2,112 ¹ 6,072 ¹	350 730 510 500	2,205 3,070 2,935 920	1.20 0.70 0.70 2.40	56.2 56.2 56.9 58.2	56.2 56.2 56.9 58.2	56.4 56.4 57.0 58.2	0.2 0.2 0.1 0.0
B	0,072	500	920	2.40	50.2	56.2	50.2	0.0
McClees Creek		-						
Α	970 ²	178	497	2.50	8.1	7.2 ⁴	7.4	0.2
В	2,510 ²	111	259	4.80	9.5	9.5	9.6	0.1
С	4,250 ²	129	744	1.70	17.3	17.3	17.4	0.1
D	5,340 ²	244	623	2.00	24.7	24.7	24.7	0.0
E	6,235 ²	163	843	1.00	27.8	27.8	27.8	0.0
F	7,595 ²	170	371	2.20	32.4	32.4	32.5	0.1
G	8,695 ²	110	374	1.70	34.1	34.1	34.3	0.2
Н	9,555 ²	284	1,156	0.50	39.1	39.1	39.2	0.1
McGellairds Brook								
A	2,114 ³	189	360	4.18	61.1	61.1	61.1	0.0
B	2,114 2,399 ³	180	373	4.16	62.0	62.0	62.0	0.0
C	2,469 ³	188	759	2.01	62.6	62.6	62.6	0.0
D	3,314 ³	350	968	1.60	63.1	63.1	63.1	0.0
E	4,277 ³	300	1,480	1.00	63.7	63.7	63.7	0.0
F	5,650 ³	400	1,310	1.10	65.4	65.4	65.6	0.0
G	7,286 ³	240	415	3.60	66.9	66.9	66.9	0.0
Н	8,290 ³	515	1,030	1.10	68.3	68.3	68.3	0.0
	9,557 ³	175	720	1.50	71.1	71.1	71.1	0.0
J	12,144 ³	510	775	1.40	72.7	72.7	72.7	0.0
ĸ	12,778 ³	145	80	6.70	73.4	73.4	73.4	0.0
¹ Feet above county boundary ² Feet above Cooper Road (up ³ Feet above confluence with M ⁴ Elevation computed without c	stream face) atchaponix Brook	water effects	from Navesink	l			<u> </u>	
							— -	
					FLOOI	DWAY DA	ATA	
	TH COUNT	Y N.I			_			
		•						
(ALL JU	RISDICTIO	NS)		МАТСН	APONIX BRO MCGELL	DOK – MCC AIRDS BR		EK –

FLOODING SOL	JRCE		FLOODWA	Y	Ŵ	BASE FI ATER-SURFAC (FEET N	CE ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
cGellairds Brook								
, L	14,678 ¹	160	140	3.80	77.1	77.1	77.1	0.0
м	15,946 ¹	150	335	1.60	81.4	81.4	81.4	0.0
N	16,843 ¹	210	715	0.70	83.7	83.7	83.7	0.0
0	18,374 ¹	320	385	1.40	87.2	87.2	87.3	0.1
P	19,114 ¹	290	334	1.60	89.4	89.4	89.4	0.0
Q	19,853 ¹	250	565	1.00	90.7	90.7	90.7	0.0
R	20,750 ¹	170	180	3.00	92.4	92.4	92.4	0.0
S	22,070 ¹	205	475	1.10	97.6	97.6	97.7	0.1
T	23,971 ¹	155	315	1.70	100.8	100.8	100.8	0.0
Ŭ	24,735 ¹	28	54	6.20	101.5	101.5	101.7	0.2
v	25,945 ¹	44	104	3.20	107.3	107.3	107.5	0.2
Ŵ	25,975 ¹	13	49	6.90	107.3	107.3	107.5	0.2
X	26,150 ¹	13	62	5.40	108.3	108.3	108.5	0.2
Y	26,180 ¹	52	174	1.90	108.8	108.8	109.0	0.2
Z	26,530 ¹	155	574	0.60	108.9	108.9	109.1	0.2
AA	27,865 ¹	101	305	1.00	109.2	109.2	109.4	0.2
AB	28,215 ¹	43	97	3.20	109.3	109.3	109.5	0.2
AC	28,705 ¹	32	103	3.00	110.7	110.7	110.7	0.0
AD	29,215 ¹	23	50	6.20	112.3	112.3	112.3	0.0
AE	29,265 ¹	34	122	2.50	112.9	112.9	112.9	0.0
AF	29,295 ¹	34	129	2.40	113.0	113.0	113.2	0.2
Metedeconk River North Branch								
А	10,195 ²	157/80 ³	563	2.10	16.6	16.6	16.8	0.2
В	11,025 ²	118/50 ³	557	2.00	18.6	18.6	18.8	0.2

² Feet above confluence with South Branch Metedeconk River ³ Width / width within county boundary

۲Ъ	FEDERAL EMERGENCY MANAGEMENT AGENCY
TABLE 12	MONMOUTH COUNTY, NJ
	(ALL JURISDICTIONS)
2	

FLOODWAY DATA

MCGELLAIRDS BROOK -**METEDECONK RIVER NORTH BRANCH**

	FLOODING SOU	RCE		FLOODWA	Y	V	BASE F ATER-SURFAC (FEET N	CE ELEVATION	
	CROSS SECTION	DISTANCE ¹	WIDTH ² (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Nor	edeconk River th Branch ntinued)								
	C D E F	13,120 14,345 15,450 15,480	433/400 371/275 28/23 97/70	1,985 1,613 252 418	0.60 0.60 3.70 2.20	21.0 21.8 23.0 23.0	21.0 21.8 23.0 23.0	21.2 22.0 23.1 23.2	0.2 0.2 0.1 0.2
	G H J	17,545 18,790 19,810 20,585	182/105 184/160 141/95 173/60	757 797 657 689	1.20 1.20 1.40 1.30	25.6 26.7 27.8 28.8	25.6 26.7 27.8 28.8	25.7 26.9 28.0 28.9	0.1 0.2 0.2 0.1
	K L M N O	21,440 22,020 22,325 22,355 24,125	219/200 136/65 74/25 81/55 292/215	828 493 519 348 348	1.10 1.90 1.80 2.60 0.60	30.1 31.0 31.9 31.9 35.2	30.1 31.0 31.9 31.9 35.2	30.3 31.2 32.1 32.1 35.4	0.2 0.2 0.2 0.2 0.2 0.2
	P Q R S	25,125 25,285 26,285 27,110 28,130	166/140 74/53 363/310 231/195	830 365 889 645	1.10 2.50 1.00 1.40	35.2 35.8 36.8 37.7 38.9	35.2 35.8 36.8 37.7 38.9	36.0 36.9 37.8 39.0	0.2 0.1 0.1 0.1
	T U V W	29,300 29,330 30,060 30,090	45/20 38/20 48/40 207/105	270 220 435 959	3.40 4.20 2.10 0.90	40.4 40.5 42.2 42.2	40.4 40.5 42.2 42.2	40.4 40.5 42.4 42.4	0.0 0.0 0.2 0.2
	X Y Z AA	31,660 32,590 33,480 34,520	113/90 240/205 188/145 234/70	580 887 716 961	1.60 1.00 1.30 0.90	43.7 44.8 45.9 47.0	43.7 44.8 45.9 47.0	43.9 45.0 46.1 47.2	0.2 0.2 0.2 0.2
	et above confluence with So idth / width within county bou		conk River						
TABL	FEDERAL EMERGEN					FLOOI	DWAY DA	ТА	
.E 12		RISDICTIO	•		METE	DECONK R			NCH

FLOODING SO	JRCE		FLOODWA	Y	V	BASE F /ATER-SURFAC (FEET N	CE ELEVATION	
CROSS SECTION	DISTANCE ¹	WIDTH ² (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Metedeconk River North Branch								
(continued)								
AB	43,800	140/65	531	1.70	48.2	48.2	48.4	0.2
AC	43,850	45/20	348	2.60	51.4	51.4	51.6	0.2
AD	45,170	40/25	241	3.70	51.4	51.4	51.6	0.2
AE	46,250	125/120	494	1.80	52.3	52.3	52.5	0.2
AF	47,300	213/130	913	1.00	53.8	53.8	54.0	0.2
AG	48,355	417/360	1,723	0.50	54.5	54.5	54.7	0.2
AH	49,300	221/190	850	1.00	55.4	55.4	55.6	0.2
Al	50,250	295/185	1,187	0.70	56.4	56.4	56.5	0.1
AJ	51,115	286/230	1,179	0.80	57.2	57.2	57.4	0.2
AK	51,950	134/110	735	1.20	58.0	58.0	58.2	0.2
AL	54,425	181/145	1,145	0.80	58.1	58.1	58.3	0.2
AM	55,530	157/95	686	1.30	58.4	58.4	58.6	0.2
AN	56,670	95/85	381	1.10	59.5	59.5	59.6	0.1
AO	57,770	300/280	1,001	0.80	61.1	61.1	61.3	0.2
AP	58,570	154/130	661	1.30	62.2	62.2	62.4	0.2
AQ	59,400	231/191	557	1.50	63.6	63.6	63.8	0.2
AR	43,800	330/280	1,053	0.80	65.4	65.4	65.5	0.1
AS	43,850	321/261	1,147	0.70	66.6	66.6	66.8	0.2
AT	45,170	452/250	1,798	0.50	67.6	67.6	67.8	0.2
AU	46,250	125/55	878	1.00	68.8	68.8	69.0	0.2
AU	47,300	152/60	525	1.50	69.8	69.8	70.0	0.2
AV	48,355	166/43	693	1.10	71.1	71.1	71.3	0.2
AVV AX		190/160	609	1.30	71.1	72.4	71.3	0.2
AX AY	49,300 50,250	190/160	650	1.20	72.4	72.4	72.5	0.1
AT	51,115	131/58	569	1.40	73.5	73.5	74.6	0.0
¹ Feet above confluence with S ² Width / width within county bo	outh Branch Metede							
FEDERAL EMERGE	TH COUNT				FLOOI	DWAY DA	TA	
ALL JU	RISDICTIO	NS)		METE	DECONK R	IVER NO		NCH

FLOODING S	DURCE		FLOODWA	Y	W	BASE F ATER-SURFAC (FEET N	CE ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Metedeconk River North Branch								
(continued)								
BA	59,965	183/120 ³	617	1.30	75.4	75.4	75.5	0.1
BB	60,660 ¹	196/100 ³	819	1.00	76.6	76.6	76.7	0.1
BC	61,820 ¹	180/85 ³	859	0.90	77.7	77.7	77.8	0.1
BD	62,800 ¹	110/53 ³	342	2.30	78.5	78.5	78.6	0.1
BE	64,565 ¹	97/57 ³	398	1.70	80.0	80.0	80.2	0.1
BF	65,385 ¹	424/349 ³	1,017	0.70	80.6	80.6	80.8	0.2
BG	66,695 ¹	402/240 ³	1,349	0.50	81.7	81.7	81.8	0.2
BH	67,700 ¹	178/68 ³	1,012	0.50	84.6	84.6	1	
BI	68,785 ¹	99/39 ³	283	2.10	84.6		84.7	0.1
BJ	69,335 ¹	99/39 125/40 ³	283 665			85.5	85.7	0.2
BK	70,785 ¹	212/142 ³		0.90	87.6	87.6	87.8	0.2
BL			890	0.70	88.4	88.4	88.6	0.2
	71,815 ¹	194/100 ³	870	0.70	89.3	89.3	89.5	0.2
BM	72,815 ¹	90/20 ³	476	1.30	90.1	90.1	90.2	0.1
BN	74,495 ¹	40/20 ³	247	2.30	91.2	91.2	91.4	0.2
BO	75,700 ¹	78/53 ³	424	1.30	92.1	92.1	92.3	0.2
Milford Brook								
А	1,320 ²	285	470	1.20	69.8	69.8	69.8	0.0
В	2,957 ²	325	500	1.20	74.4	74.4	74.5	0.1
С	3,590 ²	230	140	4.30	75.1	75.1	75.1	0.0
D	4,277 ²	250	615	1.00	76.2	76.2	76.3	0.1
E	6,125 ²	180	370	1.60	77.3	77.3	77.3	0.0
F	7,022 ²	180	180	3.30	80.3	80.3	80.3	0.0
G	8,078 ²	325	385	1.50	85.4	85.4	85.4	0.0
H	9,082 ²	280	610	1.00	86.7	86.7	86.7	0.0
Feet above confluence with Feet above confluence with Width / width within county I	South Branch Metede McGellairds Brook							
FEDERAL EMERC	ENCY MANAGEMEN	IT AGENCY						······································
	ERGENCY MANAGEMENT AGENCY			FLOO	WAY DA	TA		
	JRISDICTIO	•		METE	DECONK RI	VER NORT	H BRANCH	
		• •	1			ORD BROO		-

FLOODING SOU	IRCE		FLOODWA	Y	N	BASE FI ATER-SURFAC (FEET N	E ELEVATION	
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
lilford Brook continued)								
	10,560	210	145	4.00	90.0	90.0	90.0	0.0
J	11,352	225	395	1.50	92.0	92.0	92.0	0.0
ĸ	13,306	205	265	2.20	97.8	97.8	97.9	0.0
	14,256	210	190	2.10	100.1	100.1	100.1	0.0
M	15,629	240	405	1.00	103.7	103.7	103.8	0.0
	16,632	240	170	2.30	109.9	109.9	103.8	0.1
N								
0	18,480	210	320	1.20	113.8	113.8	113.8	0.0
P	19,378	60	55	7.30	115.6	115.6	115.6	0.0
Q	19,484	160	210	1.90	116.8	116.8	116.8	0.0
R	20,223	90	200	2.00	119.3	119.3	119.3	0.0
S	21,015	370	405	1.00	121.3	121.3	121.3	0.0
Т	21,912	150	100	4.10	123.2	123.2	123.2	0.0
U	22,916	240	255	1.60	125.4	125.4	125.4	0.0
V	23,391	360	180	2.20	129.1	129.1	129.2	0.1
W	23,496	450	495	0.80	129.6	129.6	129.7	0.1
х	24,130	860	1,155	0.30	129.8	129.8	129.8	0.0
Y	25,133	200	170	2.40	131.1	131.1	131.1	0.0
Z	25,239	210	365	1.10	133.2	133.2	133.2	0.0
ĀA	25,820	450	60	6.60	139.4	139.4	139.4	0.0
AB	26,136	470	825	0.50	142.9	142.9	143.0	0.1
AC	26,823	400	2,095	0.10	142.9	142.9	143.0	0.1
AD	28,090	370	85	1.60	142.3	143.3	143.4	0.1
AE	28,512	150	80	1.80	145.3	145.3	146.3	0.1
AE		130	50	2.70	149.4	140.3	140.3	0.0
AF	28,882					149.4		
AG	29,252	180	160	2.50	150.4	150.4	150.5	0.1
Feet above confluence with M	 cGellairds Brook	<u> </u>	<u> </u>	I	I		L	L
	FEDERAL EMERGENCY MANAGEMENT AGENCY				FLOO	OWAY DA	TA	
	TH COUNT RISDICTIO	•						
-		-				ORD BROO	JN	

FLOODING SOL	JRCE		FLOODWA	Y	V	BASE F /ATER-SURFAC (FEET N	CE ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Millstone River				OLOOND/				
A	308,020 ¹	260	794	2.00	118.6	118.6	118.7	0.1
В	310,200 ¹	190	570	2.00	122.6	122.6	122.8	0.2
C	312,530 ¹	354	902	1.30	126.1	126.1	126.3	0.2
D	314,750 ¹	570	1,306	0.80	127.5	127.5	127.7	0.2
E	317,100 ¹	280	801	1.10	133.1	133.1	133.3	0.2
F	318,430 ¹	370	828	1.00	134.4	134.4	134.6	0.2
G	310,430 322,225 ¹	152	199	3.60	142.9	142.9	143.0	0.2
H	322,225 324,766 ¹	23	71	10.00	142.9	142.9	143.0	0.0
	326,785 ¹	140	165	3.40	152.7	147.1	152.8	0.0
1	328,885 ¹	140	346	1.60	157.1	157.1	157.3	0.1
J K	420,490 ¹	402	1,071	0.70	140.7	140.7	140.8	0.2
ĸ	420,490	402	1,071	0.70	140.7	140.7	140.0	0.1
Mine Brook								
A	1,000 ²	102	417	3.70	41.7	41.7	41.9	0.2
В	2,650 ²	80	307	5.00	45.3	45.3	45.5	0.2
С	4,100 ²	114	589	2.60	49.3	49.3	49.4	0.1
D	4,500 ²	67	341	4.50	49.6	49.6	49.7	0.1
E F	5,570 ²	83	283	5.40	54.9	54.9	55.1	0.2
F	6,051 ²	79	44	3.50	58.7	58.7	58.7	0.0
G	6,455 ²	132	318	4.80	59.1	59.1	59.1	0.0
Н	7,200 ²	63	400	3.80	64.1	64.1	64.1	0.0
J	7,750 ²	105	667	2.30	64.4	64.4	64.6	0.2
J	8,750 ²	82	284	3.70	65.6	65.6	65.8	0.2
ĸ	9,300 ²	71	219	4.80	67.4	67.4	67.6	0.2
Ĺ	9,547 ²	190	1,021	0.90	74.8	74.8	74.8	0.0
M	11,870 ²	133	308	3.00	76.1	76.1	76.2	0.1
N	12,500 ²	80	440	2.10	79.4	79.4	79.4	0.0
¹ Feet above confluence with R ² Feet above confluence with Y				1	<u> </u>			<u> </u>
FEDERAL EMERGE	NCY MANAGEMEN				FLOOI	DWAY DA	ТА	
3 (ALL JU	RISDICTIO	•		MIL		IVER – MI	NE BROO	K

FLOODING SOU	JRCE		FLOODWA	Y	BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Mine Brook				······································					
(continued) O P Q R	13,720 ¹ 15,540 ¹ 16,500 ¹ 17,300 ¹	50 66 88 118	125 183 292 310	7.30 5.00 3.10 2.90	80.4 84.4 87.3 88.7	80.4 84.4 87.3 88.7	80.4 84.6 87.5 88.9	0.0 0.2 0.2 0.2	
Miry Bog Brook									
A B C D E F G H I	600 ¹ 1,300 ¹ 1,500 ¹ 1,770 ¹ 2,080 ¹ 2,340 ¹ 2,550 ¹ 3,040 ¹ 3,530 ¹	26 26 40 61 184 184 191 160 49	53 68 141 124 990 872 1,024 742 166	7.20 5.60 2.70 3.10 0.40 0.40 0.40 0.50 2.30	66.9 72.9 75.2 75.5 79.1 79.1 83.3 83.3 83.3	66.9 72.9 75.2 75.5 79.1 79.1 83.3 83.3 83.3	67.0 73.0 75.2 75.5 79.1 79.1 83.3 83.3 83.3 83.3	0.1 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	
Mohingson Brook									
A B C D E F G H I ¹ Feet above confluence with Y ² Feet above confluence with M	113 ² 625 ² 1,165 ² 1,631 ² 2,719 ² 3,077 ² 3,407 ² 4,798 ² 5,103 ² ellow Brook	447 170 130 116 266 90 498 117 431	695 1,015 842 976 3,836 740 6,078 1,150 4,329	1.90 1.30 1.60 1.20 0.30 0.40 0.00 0.20 0.10	10.5 10.5 16.1 21.1 25.5 25.5 25.5 25.5 25.5	3.4^3 8.9^3 16.1 21.1 25.5 25.5 25.5 25.5 25.5	3.6 8.9 16.3 21.1 21.3 25.5 25.5 25.5 25.5 25.5	0.2 0.0 0.2 0.0 0.2 0.0 0.0 0.0 0.0	

	FEDERAL EMERGENCY MANAGEMENT AGENCY	
	MONMOUTH COUNTY, NJ	FLOODWAY DATA
12	(ALL JURISDICTIONS)	MINE BROOK – MIRY BOG BROOK – MOHINGSON BROOK

	FLOODING SOU				~	14	BASE FI			
	FLOODING SOU	KUE		FLOODWA	T	WATER-SURFACE ELEVATION (FEET NAVD)				
	CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
	ningson Brook ntinued)									
(001	J K L M N O P Q R S T U V W	$5,337^{1}$ $6,932^{1}$ $7,732^{1}$ $8,630^{1}$ $9,002^{1}$ $9,580^{1}$ $9,932^{1}$ $10,862^{1}$ $11,221^{1}$ $11,561^{1}$ $11,887^{1}$ $12,220^{1}$ $12,585^{1}$ $13,232^{1}$	188 156 29 40 129 72 124 33 36 33 37 58 33 24	1,377 402 106 194 115 338 273 91 156 152 346 157 272 187	0.20 1.70 11.0 6.0 8.60 2.90 3.60 8.90 5.20 5.30 2.30 5.20 3.00 3.40	25.5 25.5 26.5 34.0 39.6 41.8 45.5 48.0 53.4 53.9 59.9 65.3 75.7 77.3	25.5 25.5 26.5 34.0 39.6 41.8 45.5 48.0 53.4 53.9 59.9 65.3 75.7 77.3	25.5 25.5 26.5 34.0 39.6 41.8 45.7 48.0 53.4 53.9 59.9 65.3 75.7 77.3	0.0 0.0 0.0 0.0 0.0 0.0 0.2 0.0 0.0 0.0	
	x	13,623 ¹	60	234	2.70	77.5	77.5	77.6	0.1	
Mor	nascunk Creek A B C D E F G	450 ² 980 ² 1,400 ² 1,950 ² 2,810 ² 4,070 ² 5,115 ²	61 40 18 100 100 135 73	169 164 132 144 243 520 245	3.40 3.50 4.40 4.00 2.40 1.00 2.10	19.6 23.9 26.8 29.7 33.7 42.6 43.4	19.6 23.9 26.8 29.7 33.7 42.6 43.4	19.7 24.0 26.9 29.8 33.9 42.6 43.5	0.1 0.1 0.1 0.2 0.0 0.1	
	eet above confluence with Ma eet above confluence with Fla		I			l	<u> </u>	J	I	
	FEDERAL EMERGEN					FLOOI	OWAY DA	TA		
П 12	(ALL JUF	RISDICTIO	NS)		MOHIN	GSON BROC	OK – MONA	SCUNK CR	REEK	

	, <u> </u>			·	r	B 400 - 5		
FLOODING SC	ROSS SECTION DISTANCE (FE ash Brook 200^1 (FE B 800^1 200^1 (FE B 800^1 $2,400^1$ $4,450^1$ $6,400^1$ $7,425^1$ 29 J $7,425^1$ 29 $7,425^1$ 29 $7,425^1$ 29 J $7,425^1$ 20 $7,425^1$ 20 $7,425^2$ 895^2 24 C $1,835^2$ 8 90 $2,185^2$ 111 $10,435^2$ $10,435^2$ $10,435^2$ $10,435^2$ $10,435^2$ $10,435^2$ $10,435^2$ $10,435^2$ $10,435^2$ $10,435^2$ $10,435^2$ $10,435^2$ $10,435^2$ $10,435^2$ $10,435^2$			Y	w N	BASE F ATER-SURFAC/ FEET N	E ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Musquash Brook								
		*	*	*	9.0	*	*	*
		*	*	*	9.0	*	*	*
		*	*	*	9.0	*	*	*
		*	*	*	14.0	*	*	*
		*	*	*	18.3	*	*	*
•		*	*	*	38.5	*	*	*
		*	*	*	53.2	*	*	*
Н		291	2303	0.1	53.2	53.2	53.4	0.2
I	7,425	23	61	10.4	57.7	57.7	57.9	0.2
Nut Swamp Brook								
	0 ²	318	614	2.70	16.0	16.0	16.1	0.1
	895 ²	240	894	0.90	16.7	16.7	16.8	0.1
	1 835 ²	80	259	3.20	18.0	18.0	18.1	0.1
	2.185^2	114	359	2.30	19.1	19.1	19.3	0.2
_	2,100 2.645^{2}	201	1,315	0.60	24.7	24.7	24.8	0.1
	1 305 ²	69	237	3.50	25.6	25.6	25.8	0.2
	4,395 5 195 ²		606	1.30	29.5	29.5	29.5	0.0
	5,165 6,405 ²	73	163	3.70	31.3	31.3	31.3	0.0
	7,925 ²	120	216	2.60	40.2	40.2	40.4	0.2
l I	8,495 ²	120	416	1.30	40.2	40.2	40.4	0.2
J	9,175 ²	190	398	1.30	49.3	49.3	49.4	0.1
L M	9,175 9,595 ²	192	398	1.60	49.3 50.0	49.3 50.0	49.4 50.2	0.1
IVI	9,595	101	323	1.60	50.0	50.0	50.2	0.2
Parkers Creek						2		
A	6,420 ¹	250	2,893	0.50	8.0	5.1 ³	5.3	0.2
В	7,970 ¹	465	3,031	0.40	8.0	5.1 ³	5.3	0.2
¹ Feet above mouth			* Data not a	u available		L	I	
² Feet above limit of detailed s	study (limit of detailed	study is appr	oximately 2300	feet below Norm	andy Road)			
³ Elevation computed without	consideration of back	water effects	from Shrewsbu	ry River	- ,			
FEDERAL EMERG		I AGENUT					-	
F I					FLOOI	DWAY DA	AIA	
	JTH COUNT	Y. N.I						
		•						01/
	JRISDICTIO	(Gri		MUSQL	JASH BROOK	K – NUT SV	VAMP BRO	0K –
3					PARK	ERS CREE	K	
1					I / \I \I \			

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Parkers Creek									
(continued) C	11,250 ¹	294	1,116	1.20	8.0	6.2 ⁴	6.4	0.2	
D	12,900 ¹	186	672	0.80	8.0	6.2 ⁴	6.5	0.2	
	13,995 ¹					6.6^4			
E	13,995 14,520 ¹	99	177	3.00	8.0		6.8	0.2	
F		183	351	1.50	8.0	8.0	8.2	0.2	
G	14,950 ¹	137	265	2.00	8.7	8.7	8.9	0.2	
Н	15,320 ¹	39	127	4.30	9.9	9.9	10.1	0.2	
	15,610 ¹	41	164	3.30	10.8	10.8	11.0	0.2	
J	16,020 ¹	216	574	0.90	12.5	12.5	12.6	0.1	
K	16,350 ¹	124	216	2.50	12.7	12.7	12.8	0.1	
L	16,652 ¹	118	224	2.40	13.6	13.6	13.8	0.2	
M	17,220 ¹	156	349	1.50	14.6	14.6	14.8	0.2	
arkers Creek North Branch	-					_			
Α	110 ²	197	1,008	0.30	8.2	8.2 ⁵ 8.2 ⁵	8.4	0.2	
В	2,170 ²	187	456	0.80	8.2	8.2 ⁵	8.4	0.2	
С	3,900 ²	48	124	2.80	12.3	12.3	12.5	0.2	
D	4,020 ²	45	137	2.50	12.6	12.6	12.8	0.2	
E	4,420 ²	23	61	5.60	14.8	14.8	14.8	0.0	
rine Brook 1									
A	300 ³	110	473	3.90	10.6	2 1 ⁶	2.3	0.2	
B	525 ³	240	612	3.00	10.6	2.1 ⁶	2.6	0.2	
C	1,705 ³	240	882	2.10	10.6	2.1 ⁶ 2.4 ⁶ 3.2 ⁶ 3.5 ⁶	3.4	0.2	
D	2,225 ³	345	778	2.40	10.6	3.56	3.7	0.2	
E	3,100 ³	343	1,622	1.10	10.6	6.5 ⁶	6.7	0.2	
F	3,700 ³	265	1,022	1.10	10.6	6.5 ⁶	6.7	0.2	
Feet above mouth Feet above confluence with Pa Feet above confluence with Sw		<u> </u>	⁵ Elevation of	computed without	consideration of bac consideration of bac t consideration of bac	kwater effects fro	m Parkers Creek	er	
FEDERAL EMERGEN	ICY MANAGEMEN	IT AGENCY			FLOOI	OWAY DA	TA		
MONMOUT	H COUNT	Y, NJ			_	·			
(ALL JURISDICTIONS)				PARKERS CREEK – PARKERS CREEK, NORTH BRANCH					
			PINE BROOK 1						

FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Pine Brook 1									
continued)	4,500 ¹	044	4 000	1 20	100	0.03		0.2	
G		314	1,280	1.20	10.6	6.6^{3} 6.8^{3}	6.8 7.0	0.2	
H	5,500 ¹ 6,000 ¹	311 215	1,177 877	1.60 2.10	10.6	6.8 6.9 ³	7.0	0.2	
1	6,000 6,500 ¹	124		2.60	10.6	7.1 ³	7.1	0.2	
J	6,500 6,730 ¹	124	709 715	2.60	10.6 10.6	7.1 7.3 ³	7.5	0.2	
K			880			7.3 7.4 ³	7.5 7.6	0.2	
L	7,000 ¹	140		2.10	10.6	7.4 7.5 ³	7.6	0.2	
M	7,500 ¹	257	1,570	1.20	10.6	8.1 ³		0.2	
N	8,500 ¹	147	323	5.70	10.6		8.2 12.0	0.1	
0	9,750 ¹	161	518	3.50	11.8	11.8		0.2	
P	10,500 ¹	245	669	2.70	13.1	13.1	13.3	0.2	
Q	11,300 ¹	166	432	4.20	14.7	14.7	14.9	0.2	
R	12,303 ¹	154	1,090	1.70	33.1	33.1	33.1		
S	12,520 ¹	48	304	6.00	33.4	33.4	33.4	0.0	
Т	13,300 ¹	48	297	6.20	35.5	35.5	35.7	0.2	
U	13,700 ¹	48	316	5.80	36.6	36.6	36.8	0.2	
V	14,920 ¹	48	300	6.10	40.3	40.3	40.5	0.2	
W	15,500 ¹	48	288	6.30	41.9	41.9	42.1	0.2	
Х	16,620 ¹	102	643	2.80	44.1	44.1	44.3	0.2	
Pine Brook 2									
A	1,056 ²	160	570	1.90	56.7	56.7	56.9	0.2	
В	2,587 ²	350	845	1.30	60.5	60.5	60.5	0.0	
С	3,221 ²	155	550	2.00	62.4	62.4	62.4	0.0	
D	4,277 ²	170	200	4.40	64.6	64.6	64.6	0.0	
E	5,280 ²	300	730	1.20	66.8	66.8	66.8	0.0	
F	6,811 ²	210	290	3.10	69.5	69.5	69.5	0.0	

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¹Feet above confluence with Swimming River ²Feet above confluence with Matchaponix Brook ³Elevation computed without consideration of backwater effects from Swimming River

TA	FEDERAL EMERGENCY MANAGEMENT AGENCY
TABLE	MONMOUTH COUNTY, NJ
П	

MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)

FLOODWAY DATA

PINE BROOK 1 – PINE BROOK 2

		·····							
FLOODING SOURCE		FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Pine Brook 2 (continued)									
G H J K	9,240 ¹ 11,246 ¹ 12,778 ¹ 14,203 ¹ 15,787 ¹ 19,166 ¹	200 230 335 195 250 410	640 640 615 360 610 980	1.40 1.00 1.00 1.80 1.00 0.90	75.6 82.1 85.4 87.9 93.5 102.2	75.6 82.1 85.4 87.9 93.5 102.2	75.7 82.1 85.4 87.9 93.5 102.3	0.1 0.0 0.0 0.0 0.0 0.1	
M N	20,223 ¹ 21,120 ¹	200 520	150 385	3.40 1.30	102.2 107.7 113.4	107.7 113.4	102.3	0.0	
Pine Brook 2 Tributary C A B C D	211 ² 1,584 ² 1,742 ² 2,482 ²	20 15 100 100	120 40 315 155	1.30 3.80 0.50 1.00	77.6 77.8 83.7 84.0	77.6 77.8 83.7 84.0	77.6 77.9 83.7 84.1	0.0 0.1 0.0 0.1	
Polypod Brook A B	170 ³ 815 ³	123 41	555 151	0.50 1.70	57.6 57.6	57.6 57.6	57.8 57.8	0.2 0.2	
Poly Pond Brook A B C D	100^4 1,000 ⁴ 2,000 ⁴ 3,500 ⁴	431 446 669 206	1,581 2,453 3,244 477	0.20 0.10 0.10 0.80	17.0 17.0 17.0 17.0 17.0	17.0 17.0 17.0 17.0	17.2 17.2 17.2 17.2	0.2 0.2 0.2 0.2	
¹ Feet above confluence with I ² Feet above confluence with I ³ Feet above confluence with 0 ⁴ Feet above limit of detailed s	Pine Brook 2 Groundhog Brook	study is appr	oximately 950 f	eet below Route	71)	I	L	I	
FEDERAL EMERGENCY MANAGEMENT AGENCY MONMOUTH COUNTY, NJ				FLOODWAY DATA					
(ALL JURISDICTIONS)				PINE BROOK 2 – PINE BROOK 2 TRIBUTARY C – POLY POND BROOK – POLYPOD BROOK					

FLOODING SOU	JRCE		FLOODWA	Y	BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
oplar Brook	· · · · · · · · · · · · · · · · · · ·	+	+,	,	<u>}</u>	· ['	t'	1	
A	490 ¹	25	127	10.20	8.4	8.4	8.6	0.2	
В	1,665 ¹	236	1,523	0.80	13.6	13.6	13.8	0.2	
С	2,370 ¹	200	986	1.30	13.7	13.7	13.9	0.2	
D	3,305 ¹	230	689	1.90	14.1	14.1	14.3	0.2	
E F	4,080 ¹	155	715	1.80	18.1	18.1	18.2	0.1	
	6,630 ¹	280	1,164	1.10	27.6	27.6	27.7	0.1	
G	7,110 ¹	260	1,892	0.50	27.6	27.6	27.8	0.2	
Н	8,190 ¹	140	800	1.20	27.6	27.6	27.8	0.2	
I	9,600 ¹	80	284	3.40	27.7	27.7	27.9	0.2	
J	10,360 ¹	440	823	1.20	30.1	30.1	30.2	0.1	
К	11,600 ¹	180	338	2.80	30.7	30.7	30.9	0.2	
L	12,565 ¹	260	371	2.10	33.8	33.8	33.9	0.1	
Μ	13,055 ¹	200	445	1.70	34.1	34.1	34.3	0.2	
Ν	14,870 ¹	45	101	7.60	36.7	36.7	36.7	0.0	
0	15,140 ¹	37	94	8.10	38.7	38.7	38.7	0.0	
Р	16,550 ¹	190	559	1.10	45.2	45.2	45.4	0.2	
Q	17,600 ¹	23	101	5.90	47.1	47.1	47.3	0.2	
R	18,430 ¹	50	174	3.40	50.1	50.1	50.2	0.2	
Poplar Brook Tributary 1						'	'		
Α	750 ²	11	23	4.80	27.6	25.4 ³	25.4	0.0	
В	1,650 ²	24	65	1.70	28.6	28.6	28.8	0.2	
Poplar Brook Tributary 2				1	1		1	1	
A	1,000 ²	7	16	3.60	35.8	35.8	35.8	0.0	
В	2,150 ²	9	16	3.60	42.2	42.2	42.2	0.0	

¹ Feet above Atlantic Ocean
 ² Feet above confluence with Poplar Brook
 ³ Elevation computed without consideration of backwater effects from Poplar Brook

TABL	FEDERAL EMERGENCY MANAGEMENT AGENCY	FLOODWAY DATA
E 12	(ALL JURISDICTIONS)	POPLAR BROOK – POPLAR BROOK TRIBUTARY 1 – POPLAR BROOK TRIBUTARY 2

	FLOODING SOU	RCE		FLOODWA	Y	v	BASE F ATER-SURFAC (FEET N	E ELEVATION	
	CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Pop	lar Brook Tributary 3			/					
	A B	200 ¹ 1,200 ¹	100 15	4,522 33	0.00 5.90	34.8 37.1	34.8 37.1	35.0 37.1	0.2 0.0
Por	icy Brook								
	A B C D E F G H I J K L M N	$\begin{array}{c} 1,800^2\\ 2,820^2\\ 3,900^2\\ 5,200^2\\ 6,760^2\\ 8,420^2\\ 9,490^2\\ 10,470^2\\ 11,200^2\\ 11,610^2\\ 12,200^2\\ 13,000^2\\ 13,825^2\\ 14,260^2\end{array}$	371 * 320 263 196 92 119 142 139 164 120 45 40	5,470 4,719 4,108 4,563 2,281 613 224 219 206 255 589 244 117 139	0.20 0.20 0.20 0.30 1.10 3.10 2.90 3.10 2.50 1.00 2.40 5.10 4.10	24.2 24.2 24.2 24.2 24.2 24.5 26.3 32.9 39.3 43.8 47.2 50.0 54.1 57.3	24.2 24.2 24.2 24.2 24.2 24.5 26.3 32.9 39.3 43.8 47.2 50.0 54.1 57.3	24.2 24.2 24.2 24.2 24.5 26.5 33.0 39.5 44.0 47.4 50.2 54.3 57.3	0.0 0.0 0.0 0.0 0.0 0.0 0.2 0.2 0.2 0.2
	0	15,000 ²	17	77	7.40	62.7	62.7	62.8	0.1
Bar	nanessin Brook								
- Nai	A	1,760 ³	549	1,785	1.90	41.6	44.0	44.0	
	B C D E F G	2,000 ³ 3,050 ³ 4,820 ³ 6,600 ³ 8,520 ³ 8,860 ³	400 224 224 179 127 79	1,473 865 544 544 438 437	2.30 2.30 3.70 3.70 4.60 4.30	41.0 42.7 43.3 46.4 52.0 58.4 60.6	41.6 42.7 43.3 46.4 52.0 58.4 60.6	41.8 42.7 43.5 46.4 52.2 58.5 60.7	0.2 0.0 0.2 0.0 0.2 0.1 0.1
² Fe	et above confluence with Pop et above confluence with Na et above confluence with Wil	plar Brook vesink River					ta not available	00.7	0.1
TABLE	FEDERAL EMERGEN					FLOOD	WAY DA	ТА	<u></u>
E 12		RISDICTIO	•	F	POPLAR B	ROOK TRIBI RAMANI	JTARY 3 – ESSIN BRC		ROOK –

FLOODING SOL	JRCE		FLOODWA		BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
Ramanessin Brook									
continued)									
Н	10,820	226	498	3.80	65.5	65.5	65.6	0.1	
1	12,285 ¹	112	358	5.30	71.3	71.3	71.5	0.2	
J	12,500 ¹	166	1,046	1.80	76.4	76.4	76.5	0.1	
К	13,700 ¹	202	676	2.80	77.1	77.1	77.1	0.0	
L	15,560 ¹	120	497	3.00	83.1	83.1	83.2	0.1	
Μ	17,600 ¹	182	664	2.30	87.3	87.3	87.5	0.2	
Ν	18,500 ¹	169	395	3.80	93.1	93.1	93.3	0.2	
0	20,085 ¹	177	413	2.40	100.7	100.7	100.8	0.1	
Р	20,900 ¹	110	375	2.70	102.5	102.5	102.6	0.1	
Q	22,500 ¹	189	499	2.00	110.7	110.7	110.8	0.1	
R	23,590 ¹	105	341	2.90	116.3	116.3	116.4	0.1	
S	24,200 ¹	127	303	2.00	118.1	118.1	118.3	0.2	
Т	25,185 ¹	130	253	2.50	125.6	125.6	125.8	0.2	
Roberts Swamp Brook									
Upstream Reach)									
Α	300 ²	20	42	8.30	16.4	16.4	16.6	0.2	
В	730 ²	22	58	6.10	20.3	20.3	20.5	0.2	
С	1,080 ²	36	129	3.40	24.4	24.4	24.6	0.2	
D	1,400 ²	72	138	3.70	24.9	24.9	25.1	0.2	
E	2,000 ²	20	42	8.30	28.6	28.6	28.7	0.1	
Rocky Brook (Downstream Reach)									
А	26,687 ³	162/118 ⁴	626	1.60	106.5	106.5	106.6	0.1	
В	27,887 ³	450/360 ⁴	995	1.00	107.0	107.0	107.2	0.2	
С	28,987 ¹	419/379 ⁴	1,064	0.90	107.4	107.4	107.6	0.2	

² Feet above confluence with Willow Brook ² Feet above limit of detailed study (limit of detailed study is approximately 785 feet below Algonkin Trail)

³ Feet above county boundary
 ⁴ Width / Width within Monmouth County corporate limits

TA	FEDERAL EMERGENCY MANAGEMENT AGENCY	
TABLE	MONMOUTH COUNTY, NJ	
≡ 12	(ALL JURISDICTIONS)	RAMENESSIN REACH

FLOODWAY DATA

N BROOK – ROBERTS SWAMP BROOK (UPSTREAM H) – ROCKY BROOK (DOWNSTREAM REACH)

					· · · · · · · · · · · · · · · · · · ·					
FLOODING SOUR	CE		FLOODWA	Y	, v	BASE F ATER-SURFAC/ FEET N	CE ELEVATION			
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE		
Rocky Brook (Downstream										
Reach) (continued)	1									
D E	29,937 ¹ 30,637 ¹	516/481 ⁴ 525/435 ⁴	755 1,145	1.30 0.90	109.4 110.0	109.4 110.0	109.4 110.1	0.0 0.1		
Rocky Brook (Upstream Reach)										
Α	-2,240 ²	243	796	1.00	141.1	141.1	141.3	0.2		
В	-545 ²	100	399	2.00	146.8	146.8	146.8	0.0		
С	519 ²	192	505	1.50	148.5	148.5	148.7	0.2		
D	1,118 ²	43	194	4.00	155.3	155.3	155.5	0.2		
Shark River										
А	1,900 ³	130	554	1.80	8.9	4.9 ⁵	5.1	0.2		
В	$2,530^{3}$	435	690	1.40	8.9	5.2 ⁵	5.4	0.2		
С	$3,230^{3}$	445	265	3.80	8.9	6.2 ⁵	6.3	0.1		
D	4.600^{3}	630	2,193	0.50	8.9	6.4 ⁵	6.5	0.1		
E	5.100 ³	399	1,507	0.80	8.9	6.4 ⁵	6.5	0.1		
F	6,530 ³	327	826	1.10	8.9	6.5^{5}	6.6	0.1		
G	7,400 ³	148	265	3.80	8.9	6.8 ⁵	7.0	0.2		
н	8,120 ³	50	160	6.30	10.2	10.2	10.4	0.2		
1	8,710 ³	50	146	6.10	11.5	11.5	11.7	0.2		
J	9,330 ³	90	339	2.70	12.8	12.8	12.9	0.1		
К	9,780 ³	110	432	2.10	13.0	13.0	13.1	0.1		
L	10,600 ³	90	409	2.10	13.3	13.3	13.4	0.1		
Μ	11,160 ³	43	174	5.10	13.6	13.6	13.7	0.1		
Ν	12,350 ³	49	167	5.30	16.2	16.2	16.4	0.2		
0	13,355 ³	50	161	5.90	19.2	19.2	19.4	0.2		
Feet above county boundary		5	Elevation comp	uted without con	sideration of backwa	ter effects from SI	hark River Estuary			
² Feet from Perrineville Road										
³ Feet above mouth ⁴ Width / width within Monmouth 0	County boundary									
		NT AGENCY								
>							ΤA			
			FLOODWAY DATA							
		•								
(ALL JUR	12010110	N2)			OCKY BROO BROOK (UP			IVER		

	JRCE	FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)				
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE	
hark River		<u> </u>			 		+		
continued)				1			,	1	
P	14,100 ¹	92	326	2.80	22.1	22.1	22.3	0.2	
Q	15,600 ¹	95	265	3.30	23.8	23.8	24.0	0.2	
R	16,100 ¹	146	261	3.40	24.7	24.7	24.7	0.0	
S	16,720 ¹	131	322	2.80	25.8	25.8	26.0	0.2	
Т	17,600 ¹	59	180	5.20	27.2	27.2	27.4	0.2	
U	18,200 ¹	40	161	5.50	28.8	28.8	29.0	0.2	
V	18,660 ¹	30	137	6.50	29.5	29.5	29.7	0.2	
Ŵ	19,520 ¹	123	387	2.30	31.5	31.5	31.7	0.2	
X	20,050 ¹	30	104	8.50	32.0	32.0	32.0	0.0	
Ŷ	20,460 ¹	59	238	3.70	33.5	33.5	33.7	0.2	
Z	20,850 ¹	53	201	4.60	35.1	35.1	35.3	0.2	
ĀA	21,600 ¹	36	156	3.90	36.4	36.4	36.6	0.2	
AB	22,900 ¹	30	115	5.20	38.6	38.6	38.8	0.2	
AC	24,400 ¹	30	131	4.50	42.1	42.1	42.2	0.1	
AD	25,980 ¹	30	109	5.50	45.1	45.1	45.3	0.2	
AE	27,480 ¹	24	75	8.00	49.0	49.0	49.2	0.2	
AF	30,185 ¹	19	48	6.40	55.8	55.8	56.0	0.2	
AG	30,665 ¹	24	69	4.50	57.9	57.9	57.9	0.0	
AH	31,255 ¹	21	44	7.00	59.6	59.6	59.8	0.2	
Al	33,493 ¹	30	90	3.40	67.5	67.5	67.6	0.1	
AJ	35,285 ¹	17	40	7.60	72.8	72.8	72.9	0.1	
hark River Tributary D			1				1		
	400 ²	24	47	6.80	35.1	35.1	35.3	1 02	
A B	400 600 ²	34 169	47 340	0.90	35.1 36.7	35.1	35.3 36.9	0.2	
B C	1,200 ²	90	340 67	4.70	36.7 40.3	40.3	40.3	0.2	
				7.70	40.0	40.0	40.0	0.0	
Feet above mouth									
Feet above confluence with Sh	ark River								
FEDERAL EMERGE		IT AGENCY							
1									
1			í		FLUUI	DWAY DA	AIA		
MONMOUT	CH COUNT	YNJ							
		• • • • •							

MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)

12

SHARK RIVER – SHARK RIVER TRIBUTARY D

			T						
	FLOODING SOU	RCE		FLOODWA	Y	v	BASE F VATER-SURFAC (FEET N	E ELEVATION	
	CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
	rk River Tributary D htinued)								
	D E F	1,800 ¹ 2,150 ¹ 2,410 ¹	56 50 24	58 54 49	5.50 5.90 7.70	51.6 57.8 64.1	51.6 57.8 64.1	51.7 57.8 64.3	0.1 0.0 0.2
Sha	rk River Tributary E								
	A B C D	500 ¹ 1,900 ¹ 2,800 ¹ 3,450 ¹	56 464 125 256	129 672 340 1,370	9.00 2.50 4.30 0.10	51.7 59.4 63.7 70.4	51.7 59.4 63.7 70.4	51.9 59.6 63.9 70.6	0.2 0.2 0.2 0.2
Still	House Brook A B C D E	1,056 ² 2,270 ² 3,432 ² 4,435 ² 6,600 ²	120 120 150 60 60	390 110 250 50 90	0.90 3.10 0.70 3.30 1.80	94.1 97.5 101.1 103.3 121.6	94.1 97.5 101.1 103.3 121.6	94.3 97.5 101.3 103.3 121.8	0.2 0.0 0.2 0.0 0.2
Swi	mming River								
	A B C D E F G	$\begin{matrix} 0^{3} \\ 3,480^{3} \\ 7,180^{3} \\ 7,480^{3} \\ 11,780^{3} \\ 14,110^{3} \\ 14,550^{3} \end{matrix}$	170 639 1,085 250 434 483 566	1,930 5,926 6,703 3,088 9,247 7,706 7,896	5.80 1.90 1.40 3.10 1.00 1.20 1.20	8.1 9.7 10.9 11.0 21.8 21.9 21.9	8.1 9.7 10.9 11.0 21.8 21.9 21.9	8.3 9.9 11.1 11.2 22.0 22.1 22.1	0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2
² Fe	eet above confluence with Sh eet above confluence with Ma eet above Newman Springs F	inalapan Brook	face)	L			I	1	<u> </u>
TARI	FEDERAL EMERGEN					FLOOI	DWAY DA	TA	
T 13		RISDICTIO	•	s		R TRIBUTARY			

	FLOODING SOU	RCE		FLOODWA	Y	V	BASE FI ATER-SURFAC/ FEET N	CE ELEVATION				
	CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE			
Tepe	ehemus Brook											
iopt	A	1,267 ¹	220	200	3.40	75.9	75.9	75.9	0.0			
	В	2,006 ¹	365	720	0.90	77.3	77.3	77.3	0.0			
	C	3,379 ¹	300	400	1.70	79.8	79.8	79.8	0.0			
	D	4,488 ¹	175	255	2.60	81.6	81.6	81.7	0.1			
	E	5,227 ¹	260	285	2.40	84.4	84.4	84.4	0.0			
	F	6,336 ¹	275	590	1.10	87.2	87.2	87.4	0.2			
	Ġ	8,184 ¹	310	535	1.20	88.9	88.9	89.0	0.1			
	н	9,187 ¹	230	115	6.00	90.7	90.7	90.7	0.0			
	1	9,821 ¹	290	315	2.10	92.4	92.4	92.4	0.0			
	, ,	10,243 ¹	120	235	2.90	93.1	93.1	93.1	0.0			
	ĸ	12,302 ¹	100	140	2.70	98.7	98.7	98.7	0.0			
	R I	13,464 ¹	40	80	4.60	102.6	102.6	102.6	0.0			
	L M	13,728 ¹	100	115	4.30	102.0	102.0	102.0	0.0			
		14,625 ¹	180	215	4.30	104.1	104.1	104.2	0.2			
	N	15,523 ¹	230	445	1.90	109.8	109.8	109.8	0.2			
	O P						110.8	110.8	0.0			
	P	16,420 ¹	200	145	3.30	110.8	110.6	110.0	0.0			
Ton	ehemus Brook											
	th Branch											
300		1,162 ²	100	140	3.10	100.0	100.0	100.0	0.0			
	A	1,954 ²	130	235	1.80	104.1	100.0	104.1	0.0			
	B C	2,799 ²	200	300	1.40	107.1	107.1	107.1	0.0			
		2,799 3,538 ²	130	135	3.20	107.1	109.9	107.1	0.0			
	D	3,338 2,504 ²		760		112.3	112.3	112.3	0.0			
	E	3,591 ²	220		0.60		112.5	112.5	0.0			
		4,436 ²	140	410	1.00	112.5						
	G	5,069 ²	50	85	5.10	113.9	113.9	114.0	0.1 0.1			
	Н	5,702 ²	160	480	0.90	115.9	115.9	116.0	0.1			
	1	6,336 ²	150	125	3.50	118.3	118.3	118.3	0.0			
	et above confluence with Me											
Fe	et above confluence with Te	penemus Brook										
<u> </u>												
-1	FEDERAL EMERGE		IT AGENCY									
				FLOODWAY DATA								
πΙ												
	MONMOUTH COUNTY, NJ											
пΙ			•									
2	(ALL JUI	RISDICTIO	N)			TEPEHE	MUS BRO	JK –				
5 I	-		-		TEPEHEMUS BROOK NORTH BRANCH							
-				IEPEHEMUS BROOK NORTH BRANCH								

FLOODING SO	JRCE	FLOODWAY			BASE FLOOD WATER-SURFACE ELEVATION (FEET NAVD)					
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE		
Toms River				0100112/						
А	380 ¹	281	800	0.60	149.5	149.5	149.7	0.2		
В	1,967	39	109	4.30	150.4	150.4	150.5	0.1		
С	3,830 ¹	93	277	1.70	155.0	155.0	155.2	0.2		
D	4,622 ¹	190	452	1.00	155.6	155.6	155.8	0.2		
Town Brook										
A	7,675 ²	336	1,458	0.50	17.5	17.5	17.5	0.0		
В	9,175 ²	117	518	1.50	18.7	18.7	18.8	0.0		
c	10,870 ²	249	1,434	0.50	26.6	26.6	26.6	0.0		
D	11,285 ²	245	1,232	0.50	27.5	27.5	27.5	0.0		
Ē	12,135 ²	163	719	0.90	29.7	29.7	29.7	0.0		
F	13,930 ²	252	253	1.80	30.9	30.9	30.9	0.0		
G	15,065 ²	209	162	2.80	36.7	36.7	36.7	0.0		
Н	15,975 ²	473	2,580	0.20	43.7	43.7	43.7	0.0		
I	16,925 ²	228	267	1.70	44.2	44.2	44.2	0.0		
Turtle Mill Brook										
A	820 ³	90	289	3.80	3.9	3.9	4.1	0.2		
В	970 ³	53	146	7.60	3.9	3.9	4.1	0.2		
С	1,200 ³	137	611	2.00	5.8	5.8	5.9	0.1		
D	1,535 ³	36	186	5.90	5.8	5.8	5.9	0.1		
E	1,700 ³	55	205	5.40	6.4	6.4	6.6	0.2		
F	1,980 ³	40	199	5.50	7.3	7.3	7.5	0.2		
G	2,390 ³	173	473	2.90	9.0	9.0	9.2	0.2		
Н	3,950 ³	130	565	2.10	9.3	9.3	9.5	0.2		
]	5,150 ³	162	385	3.50	9.7	9.7	9.9	0.2		
Feet above Monmouth Road Feet above Campbell Avenue Feet above confluence with B								······································		
FEDERAL EMERGE	NCY MANAGEMEN				FLOOD	DWAY DA	ТА			
(ALL JU	RISDICTIO	NS)	Т	TOMS RIVER – TOWN BROOK – TURTLE MILL BROOK						

FLOODING SOL	IRCE		FLOODWA	Y	. W	BASE F /ATER-SURFAC (FEET N	CE ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Waackaack Creek					· · · · ·			
Α	9,130 ¹	511	2,740	0.70	10.5	8.0 ³	8.2	0.2
В	9,980 ¹	683	3,130	0.60	10.5	8.0 ³	8.2	0.2
c	10,430 ¹	552	2,622	0.70	10.5	8.1 ³	8.3	0.2
D	10,680 ¹	712	3,315	0.60	10.5	8.1 ³	8.3	
E	11,490 ¹	458	2,180	0.90	10.5	8.1 ³		0.2
F	12,665 ¹	1,080	3,068	0.60	10.5	8.1 8.4 ³	8.3	0.2
Ġ	13,165 ¹					8.4	8.6	0.2
H	13,740 ¹	1,140	4,263	0.50	10.5	8.4^{3}	8.6	0.2
		937	3,025	0.60	10.5	8.5^{3}	8.7	0.2
I	14,640 ¹	240	1,030	1.90	10.5	8.7 ³	8.9	0.2
J	14,915 ¹	530	2,234	0.90	10.5	9.0 ³	9.2	0.2
К	15,620 ¹	238	560	3.50	10.5	9.4 ³	9.6	0.2
L	15,720 ¹	293	740	2.60	10.5	9.8 ³	10.0	0.2
M	16,570 ¹	207	478	4.10	13.9	13.9	14.1	0.2
N	16,700 ¹	552	1,458	1.30	16.1	16.1	16.3	0.2
0	18,060 ¹	511	1,749	1.10	16.8	16.8	17.0	0.2
Р	18,750 ¹	150	164	3.70	19.4	19.4	19.4	0.0
Q	19,170 ¹	42	132	4.70	20.1	20.1	20.1	0.0
R	19,270 ¹	74	136	4.20	22.0	22.0	22.0	0.0
S	19,850 ¹	120	345	1.70	23.1	23.1	23.2	0.0
Т	20,500 ¹	80	121	4.80	24.9	24.9	24.9	0.0
Ŭ	21,150 ¹	100	254	2.30	26.4	26.4	26.6	0.0
v	21,630 ¹	254	339	1.70	29.6	29.6	20.0	0.2
Ŵ	22,240 ¹	70	97	5.90	33.0	33.0	33.0	0.1
	,			0.00	00.0	00.0	00.0	0.0
Nampum Brook								
А	430 ²	170	1,137	0.70	12.4	12.4	12.6	0.2
В	2,000 ²	39	207	3.70	12.4	12.4	12.6	0.2
¹ Feet above confluence with Ra ² Feet above Limit of Flood Affee ³ Elevation computed without co	cting Community (Li nsideration of back	water effects of	ffecting Comm of Raritan Bay	unity is approxim	ately 110 feet downs	tream of western	boundary of Fort M	lonmouth)
FEDERAL EMERGEN					FLOOD	WAY DA	ТА	
	RISDICTIO	NS)		WAAG	KAACK CR	EEK - WAN		ОК

FLOODING	SOURCE		FLOODWA		v	BASE FI	CE ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	(FEET N WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Wampum Brook		<u> </u>	,,,	, , , , , , , , , , , , , , , , , , , ,	ţ!	'	}	
(continued)	a aca1		,		1	1	,	1
C	2,390 ¹	20	141	5.40	13.7	13.7	13.9	0.2
D	3,000 ¹	225	791	1.00	16.5	16.5	16.5	0.0
E	3,500 ¹	240	703	1.10	16.5	16.5	16.5	0.0
F	3,800 ¹	105	240	3.20	16.6	16.6	16.6	0.0
G	4,000 ¹	143	271	2.80	16.9	16.9	17.0	0.1
Н	4,210 ¹	41	131	5.80	19.0	19.0	19.0	0.0
l	4,480 ¹	45	154	4.60	20.0	20.0	20.0	0.0
J	4,680 ¹	65	470	1.50	23.5	23.5	23.5	0.0
К	4,8801	400	718	1.00	24.5	24.5	24.5	0.0
L	5,050 ¹	75	508	1.40	24.5	24.5	24.5	0.0
М	5,360 ¹	60	359	2.00	24.6	24.6	24.6	0.0
N	5,480 ¹	60	352	2.00	24.6	24.6	24.6	0.0
Watson Creek							ļ	
А	90 ²	25	118	6.70	20.2	20.2	20.4	0.2
В	390 ²	22	498	1.60	20.5	20.5	20.7	0.2
С	1,490 ²	50	106	8.20	21.1	21.1	21.1	0.0
D	2,110 ²	50	115	5.20	25.9	25.9	26.1	0.2
Ē	2,860 ²	42	124	5.70	29.5	29.5	29.7	0.2
Weamaconk Creek					1			
А	53 ³	420	800	1.00	59.5	59.5	59.5	0.0
В	1,547 ³	347	800	0.99	60.5	60.5	60.5	0.0
C	2,032 ³	430	784	1.01	61.1	61.1	61.1	0.0
D	2,647 ³	230	382	2.07	62.4	62.4	62.4	0.0
E	3,012 ³	61	151	5.23	63.9	63.9	63.9	0.0
Feet above western bound	Idary of Fort Monmouth			L	<u> </u>	<u> </u>	!	<u> </u>
² Feet above limit of detaile	ed study (limit of detailed	studv is appr	oximately 1150	feet below Blans	sina Avenue)			
³ Feet above confluence with			-		, , , , , , , , , , , , , , , , , , ,			
FEDERAL EME	RGENCY MANAGEMEN							
π I					FLUUI	DWAY DA	AL	
[∠] Ι ΜΟΝΜC	DUTH COUNT	YNJ						
		•						
	JURISDICTIO	NJ)		WAR	MPUM BROO			
v					WEAMA	ACONK CRE	EEK	

FLOODING	SOURCE		FLOODWA		v	BASE F ATER-SURFAC (FEET N	E ELEVATION	
CROSS SECTION	DISTANCE ¹	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Veamaconk Creek continued)								
F	3,107	240	811	0.97	64.9	64.9	64.9	0.0
G	4,107	290	430	1.84	65.2	65.2	65.2	0.0
Ĥ	4,547	133	594	1.33	65.5	65.5	65.5	0.0
1	4,667	370	1,994	0.40	71.1	71.1	71.1	0.0
J	4,957	430	1,990	0.40	71.1	71.1	71.1	0.0
ĸ	5,707	239	508	1.60	71.1	71.1	71.1	0.0
L	6,758	290	380	2.10	72.3	72.3	72.3	0.0
M	9,029	220	475	1.70	76.3	76.3	76.4	0.0
N	10,085	100	155	5.20	78.9	78.9	78.9	0.0
0	10,718	210	625	1.30	81.2	81.2	81.2	0.0
P	11,880	220	475	1.70	82.5	82.5	82.5	0.0
Q	13,622	640	730	1.10	85.5	85.5	85.6	0.0
R	15,312	310	540	1.50	88.9	88.9	88.9	0.1
S	15,998	310	580	1.40	91.3	91.3	91.3	0.0
J T	17,582	200	420	0.90	95.6	95.6	95.7	0.0
Ŭ	18,374	190	310	1.10	96.6	96.6	95.7 96.7	0.1
V	19,219	140	235	1.50	98.3	96.6 98.3		
Ŵ		260	380				98.3 00.2	0.0
	20,011			0.90	99.3	99.3	99.3	0.0
X Y	20,750	355 255	415	0.90	99.8	99.8 102.2	99.9 102.2	0.1
ř Z	21,173		220	1.60	102.2	102.2	102.2	0.0
	22,493	280	285	1.20	104.8	104.8	104.8	0.0
AA	23,285	190	280	1.30	106.1	106.1	106.1	0.0
AB	24,022	154	375	0.80	106.6	106.6	106.8	0.2
AC	24,082	201	592	0.50	107.3	107.3	107.5	0.2
AD	24,957	280	118	2.50	109.7	109.7	109.9	0.2
AE	25,817	90	104	2.90	113.3	113.3	113.5	0.2
Feet above confluence wi	h Matchaponix Brook	A	···	<u> </u>	<u> </u>		.	<u> </u>
FEDERAL EMER		TAGENCY			<u> </u>			
			FLOO	OWAY DA	TA			
(ALL J	URISDICTIO	•		FLOODWAY DATA WEAMACONK CREEK				

									-
	FLOODING SOU	RCE		FLOODWA	Y	W	BASE F ATER-SURFAC (FEET N	CE ELEVATION	
	CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Wea	maconk Creek			//					
(con	tinued)								
	AF	26,497 ¹	27	79	3.80	115.6	115.6	115.8	0.2
	AG	27,057 ¹	97	188	1.60	118.6	118.6	118.8	0.2
	AH	27,507 ¹	66	63	4.70	119.7	119.7	119.9	0.2
	Al	27,727 ¹	92	376	0.60	123.5	123.5	123.5	0.0
	AJ	28,657 ¹	45	86	2.50	123.6	123.6	123.7	0.1
	AK	29,117 ¹	27	73	3.00	124.7	124.7	124.9	0.2
	AL	29,652 ¹	28	47	4.70	127.5	127.5	127.5	0.0
	AM	30,247 ¹	42	88	2.50	130.0	130.0	130.0	0.0
	AN	30,332 ¹	89	190	1.10	133.6	133.6	133.8	0.2
	AO	30,927 ¹	34	84	2.60	133.9	133.9	134.1	0.2
	AP	31,562 ¹	24	66	3.30	135.6	135.6	135.8	0.2
	AQ	32,087 ¹	28	67	3.30	137.8	137.8	137.9	0.2
		32,223 ¹	20		3.20	138.6	138.6	138.6	0.0
	AR	32,223	21	68	3.20	138.0	130.0	130.0	0.0
Wee	maconk Creek Tributary								
**66	A	400 ²	50	36	6.00	64.9	62.6 ⁴	62.6	0.0
	B	730 ²	47	99	2.20	64.9	64.0 ⁴	64.1	0.0
	C	1,080 ²	55	78	2.80	64.9	64.5 ⁴	64.6	0.1
		1,504 ²	40	53			65.8	65.8	0.0
	D	1,504			4.20	65.8			
	E	1,604 ²	35	367	0.60	79.6	79.6	79.6	0.0
	F	1,719 ²	100	1,087	0.20	79.6	79.6	79.6	0.0
Well	ls Brook								
1101	A	22 ³	262	*	*	27.5	*	*	*
	B	151 ³	115	*	*	27.5	*	*	*
	C	1,173 ³	64	*	*	38.1	*	*	*
Fe	et above confluence with Ma	tchaponix Brook			* Data no	t available			
	et above confluence with We	amaconk Creek							
³ Fe	et above mouth			· · · ·					
Ele	evation computed without con	isideration of back	water effects	trom Weamaco	onk Creek				
-1	FEDERAL EMERGEN	CY MANAGEMEN	IT AGENCY						
⊳∣								ТЛ	
						FLUUL	DWAY DA	A	
~ 	MONMOUT	H COUNT	Y. NJ						
₽ ⊓		RISDICTIO	•						
3	(ALL JUP		140)						
				WEAMACONK CREEK – WEAMACONK CREEK TRIBUTARY – WELLS BF					

	IRCE		FLOODWA	Y	v N	BASE F ATER-SURFAC (FEET N	E ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Vells Brook								
continued)	1							
D	1,862	60	*	*	49.5	*	*	*
E	2,704	65	*	*	58.2	*	*	*
F	3,172 ¹	47	*	*	60.9	*	*	*
G	3,714 ¹	*	*	*	66.5	*	*	*
Н	3,804 ¹	298	*	*	67.6	*	*	*
l	4,953 ¹	*	*	*	68.4	*	*	*
J	5,303 ¹	106	*	*	73.1	*	*	*
к	4,999 ¹	160	*	*	73.3	*	*	*
L	5,503 ¹	58	*	*	73.3	*	*	*
М	6,001 ¹	48	*	*	78.2	*	*	*
N	6,747 ¹	97	*	*	83.6	*	*	*
0	6,848 ¹	66	*	*	86.9	*	*	*
Р	7,002 ¹	52	*	*	88.0	*	*	*
Nemrock Brook								
А	1,478 ²	565	785	0.60	94.0	94.0	94.0	0.0
В	2,323 ²	250	280	1.80	95.4	95.4	95.4	0.0
С	3,274 ²	240	495	1.00	97.3	97.3	97.3	0.0
D	4,382 ²	125	245	2.10	98.8	98.8	98.8	0.0
E	5,016 ²	110	235	2.10	99.6	99.6	99.6	0.0
F	5,755 ²	120	140	3.50	101.5	101.5	101.5	0.0
G	7,250 ²	167	275	1.50	103.7	103.7	103.9	0.2
Ĥ	7,325 ²	410	1,801	0.20	107.6	107.6	107.6	0.0
1	7,625 ²							
							-	
ĸ		1						
I J K Feet above mouth Feet above confluence with W Data not available	7,695 ² 8,620 ² 9,345 ²	234 134 264	768 227 409	0.60 1.90 1.00	107.6 108.9 111.4	107.6 108.9 111.4	107.6 109.1 111.6	0.0 0.2 0.2
FEDERAL EMERGENCY MANAGEMENT AGENCY						DWAY DA	Τ Λ	

MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)

12

FLOODWAY DATA

WELLS BROOK – WEMROCK BROOK

FLOODING SOL	IRCE		FLOODWA	Y	v	BASE F ATER-SURFAC (FEET N	CE ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Wemrock Brook								
continued)	0.0051	100						
L	9,935 ¹	188	381	0.70	112.0	112.0	112.2	0.2
M	10,405 ¹	32	54	4.60	112.8	112.8	113.0	0.2
N	10,960 ¹	77	152	1.60	116.1	116.1	116.3	0.2
0	11,290 ¹	10	55	4.60	116.8	116.8	117.0	0.2
Whale Pond Brook								
А	1,070 ²	125	755	1.70	19.7	19.7	19.9	0.2
В	1,450 ²	205	952	1.30	19.8	19.8	19.9	0.1
С	$1,960^2$	170	812	1.60	19.9	19.9	20.1	0.2
D	2,200 ²	126	986	1.30	22.6	22.6	22.6	0.0
E	2,515 ²	233	1,423	0.90	22.6	22.6	22.6	0.0
F	3,150 ²	255	1,306	1.00	22.6	22.6	22.7	0.1
G	4,175 ²	85	379	3.40	22.8	22.8	23.0	0.2
Ĥ	4,555 ²	240	1,032	1.20	24.4	22.6	23.0	0.2
i.	6,465 ²	126	390	3.30	31.8	24.4 31.8	24.6 32.0	
.1	7,060 ²	305	906	1.40	31.8			0.2
S K	7,660 ²	410	906 848	1.50		32.4	32.6	0.2
	8,362 ²				34.6	34.6	34.8	0.2
M	9,260 ²	385	1,366	0.90	35.0	35.0	35.2	0.2
N	9,260 9,860 ²	564	1,439	0.60	35.3	35.3	35.5	0.2
	9,860	397	812	1.10	35.4	35.4	35.6	0.2
0	11,225 ²	342	613	1.50	36.4	36.4	36.6	0.2
P	12,235 ²	339	387	2.30	38.1	38.1	38.3	0.2
Q	12,945 ²	183	841	1.10	47.9	47.9	48.1	0.2
R	13,560 ²	282	479	1.90	48.0	48.0	48.2	0.2
S	16,300 ²	237	295	1.70	57.2	57.2	57.4	0.2
Т	18,370 ²	200	228	2.20	64.0	64.0	64.1	0.1
Feet above confluence with W Feet above Norwood Avenue	eamaconk Creek	L	I	L	L		11	
FEDERAL EMERGE	NCY MANAGEMEN	T AGENCY				DWAY DA	ΤΔ	
MONMOU	TH COUNT	Y, NJ						
(ALL JUI	RISDICTIO	•		WEMR				

(FEET) (SQUARE (FEETPER FLOODWAY FLOODWAY FLOODWAY							BASE F		
CROSS SECTION DISTANCE WIDTH (FEET) SECTION (SQUARE FEET) MEAN VELOCITY (FEET PER SECOND) REGULATORY FLOODWAY WITHOUT FLOODWAY WITH FLOODWAY INCREAS Whate Pond Brook (continued) U 19,471 ¹ 237 280 1.80 67.2 67.2 67.4 0.2 Whate Pond Brook (continued) U 19,471 ¹ 237 280 1.80 67.2 67.2 67.4 0.2 Whate Pond Brook Tributary 1 A 1,650 ² 35 76 2.40 38.0 38.0 38.2 0.2 B 2,500 ² 100 264 0.70 42.2 42.2 42.2 0.0 C 3,700 ² 104 180 1.00 43.3 43.3 0.2 D 5,000 ² 108 225 0.80 48.8 48.9 0.1 Whate Pond Brook Tributary 2 A 2,300 ² 71 86 2.10 55.9 55.9 56.1 0.2 D 4,400 ² 92	FLOODING SC	DURCE		FLOODWA	Y	∨			
$ \begin{array}{c continued) \\ U & 19,471^1 & 237 & 280 & 1.80 & 67.2 & 67.2 & 67.4 & 0.2 \\ V & 20,451^1 & 96 & 122 & 4.20 & 71.7 & 71.7 & 71.9 & 0.2 \\ \end{array} \\ \hline \begin{tabular}{lllllllllllllllllllllllllllllllllll$	CROSS SECTION	DISTANCE		AREA (SQUARE	VELOCITY (FEET PER	REGULATORY	WITHOUT	WITH	INCREASE
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	Whale Pond Brook			1					
V 20,451 ¹ 96 122 4.20 71.7 71.7 71.9 0.2 Whale Pond Brook Tributary 1 A 1,650 ² 35 76 2.40 38.0 38.0 38.2 0.2 B 2,500 ² 100 264 0.70 42.2 42.2 42.2 0.0 C 3,700 ² 104 180 1.00 43.3 43.3 43.5 0.2 D 5,000 ² 108 225 0.80 48.5 48.5 48.5 0.0 Whale Pond Brook Tributary 2		10 1711	0.07		1.00				
Whale Pond Brook Tributary 1 A 1,650 ² 35 76 2.40 38.0 38.0 38.2 0.2 B 2,500 ² 100 264 0.70 42.2 42.2 42.2 0.0 C 3,700 ² 104 180 1.00 43.3 43.3 43.5 0.2 D 5,000 ² 16 59 3.10 48.5 48.5 48.5 0.0 E 5,200 ² 108 225 0.80 48.8 48.8 48.9 0.1 Whale Pond Brook Tributary 2									
A 1,650 ² 35 76 2.40 38.0 38.0 38.2 0.2 B 2,500 ² 100 264 0.70 42.2 42.2 42.2 42.2 0.0 C 3,700 ² 104 180 1.00 43.3 43.3 43.5 0.2 D 5,000 ² 16 59 3.10 48.5 48.5 48.5 0.0 E 5,200 ² 108 225 0.80 48.8 48.8 48.9 0.1 Whale Pond Brook rributary 2 - </td <td>V</td> <td>20,451</td> <td>96</td> <td>122</td> <td>4.20</td> <td>/1./</td> <td>(1.7</td> <td>71.9</td> <td>0.2</td>	V	20,451	96	122	4.20	/1./	(1.7	71.9	0.2
A 1,650 ² 35 76 2.40 38.0 38.0 38.2 0.2 B 2,500 ² 100 264 0.70 42.2 42.2 42.2 42.2 0.0 C 3,700 ² 104 180 1.00 43.3 43.3 43.5 0.2 D 5,000 ² 16 59 3.10 48.5 48.5 48.5 0.0 E 5,200 ² 108 225 0.80 48.8 48.8 48.9 0.1 Whale Pond Brook									
B 2,500 ² 100 264 0.70 42.2 42.2 42.2 42.2 0.0 C 3,700 ² 104 180 1.00 43.3 43.3 43.5 0.2 D 5,000 ² 108 225 0.80 48.5 48.5 48.5 0.0 Whale Pond Brook -	-	$1,650^2$	35	76	2.40	38.0	38.0	38.2	0.2
C 3,700 ² 104 180 1.00 43.3 43.3 43.5 0.2 D 5,000 ² 16 59 3.10 48.5 48.5 48.5 48.5 0.1 E 5,200 ² 108 225 0.80 48.8 48.8 48.9 0.1 Whale Pond Brook Tributary 2 A 2,300 ² 68 66 2.70 52.6 52.6 52.8 0.2 B 3,200 ² 71 86 2.10 55.9 56.1 0.2 C 3,750 ² 75 93 1.90 57.2 57.2 57.4 0.2 D 4,400 ² 92 80 2.20 59.1 59.3 0.2 E 5,400 ² 90 91 2.00 62.6 62.6 62.8 0.2 Willow Brook									
D 5,000 ² 16 59 3.10 48.5 48.5 48.5 48.5 0.0 E 5,200 ² 108 225 0.80 48.8 48.8 48.9 0.1 Whale Pond Brook Tributary 2 A 2,300 ² 68 66 2.70 52.6 52.6 52.8 0.2 B 3,200 ² 71 86 2.10 55.9 56.1 0.2 C 3,750 ² 75 93 1.90 57.2 57.4 0.2 D 4,400 ² 92 80 2.20 59.1 59.3 0.2 E 5,400 ² 90 91 2.00 62.6 62.6 62.8 0.2 Willow Brook		3.700^2							
E 5,200 ² 108 225 0.80 48.8 48.8 48.9 0.1 Whale Pond Brook Tributary 2 A 2,300 ² 68 66 2.70 52.6 52.6 52.8 0.2 B 3,200 ² 71 86 2.10 55.9 55.9 56.1 0.2 C 3,750 ² 75 93 1.90 57.2 57.4 0.2 D 4,400 ² 92 80 2.20 59.1 59.3 0.2 E 5,400 ² 90 91 2.00 62.6 62.6 62.8 0.2 Willow Brook									
Tributary 2 A 2,300 ² 68 66 2.70 52.6 52.6 52.8 0.2 B 3,200 ² 71 86 2.10 55.9 55.9 56.1 0.2 C 3,750 ² 75 93 1.90 57.2 57.2 57.4 0.2 D 4,400 ² 92 80 2.20 59.1 59.1 59.3 0.2 E 5,400 ² 90 91 2.00 62.6 62.6 62.8 0.2 Willow Brook									
A 2,300 ² 68 66 2.70 52.6 52.6 52.8 0.2 B 3,200 ² 71 86 2.10 55.9 56.1 0.2 C 3,750 ² 75 93 1.90 57.2 57.2 57.4 0.2 D 4,400 ² 92 80 2.20 59.1 59.1 59.3 0.2 E 5,400 ² 90 91 2.00 62.6 62.6 62.8 0.2 Willow Brook									
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C 3,750 ² 75 93 1.90 57.2 57.2 57.4 0.2 D 4,400 ² 92 80 2.20 59.1 59.1 59.3 0.2 E 5,400 ² 90 91 2.00 62.6 62.6 62.8 0.2 Willow Brook A 3,510 ³ 549 1,785 1.9 41.6 41.6 41.8 0.2 B 4,730 ³ 266 1,147 2.4 43.3 43.3 43.5 0.2 C 5,640 ³ 267 775 3.5 45.1 45.1 45.2 0.1 D 7,000 ³ 272 860 3.2 49.9 49.9 50.0 0.1 E 8,100 ³ 181 673 4.1 53.3 53.3 53.4 0.1 F 8,900 ¹ 229 951 2.9 57.0 57.0 57.2 0.2									
D 4,400 ² 92 80 2.20 59.1 59.1 59.3 0.2 E 5,400 ² 90 91 2.00 62.6 62.6 62.8 0.2 Willow Brook A 3,510 ³ 549 1,785 1.9 41.6 41.6 41.8 0.2 B 4,730 ³ 266 1,147 2.4 43.3 43.3 43.5 0.2 C 5,640 ³ 267 775 3.5 45.1 45.1 45.2 0.1 D 7,000 ³ 272 860 3.2 49.9 49.9 50.0 0.1 E 8,100 ³ 181 673 4.1 53.3 53.3 53.4 0.1 F 8,900 ¹ 229 951 2.9 57.0 57.0 57.2 0.2		3,2002							
E 5,400 ² 90 91 2.00 62.6 62.6 62.8 0.2 Willow Brook A 3,510 ³ 549 1,785 1.9 41.6 41.6 41.8 0.2 B 4,730 ³ 266 1,147 2.4 43.3 43.3 43.5 0.2 C 5,640 ³ 267 775 3.5 45.1 45.1 45.2 0.1 D 7,000 ³ 272 860 3.2 49.9 49.9 50.0 0.1 E 8,100 ³ 181 673 4.1 53.3 53.3 53.4 0.1 F 8,900 ¹ 229 951 2.9 57.0 57.0 57.2 0.2		3,750 ²	75	93	1.90	57.2	57.2	57.4	0.2
Willow Brook A 3,510 ³ 549 1,785 1.9 41.6 41.6 41.8 0.2 B 4,730 ³ 266 1,147 2.4 43.3 43.3 43.5 0.2 C 5,640 ³ 267 775 3.5 45.1 45.1 45.2 0.1 D 7,000 ³ 272 860 3.2 49.9 49.9 50.0 0.1 E 8,100 ³ 181 673 4.1 53.3 53.3 53.4 0.1 F 8,900 ¹ 229 951 2.9 57.0 57.0 57.2 0.2		4,400 ²	92	80	2.20	59.1	59.1	59.3	0.2
A 3,510 ³ 549 1,785 1.9 41.6 41.6 41.8 0.2 B 4,730 ³ 266 1,147 2.4 43.3 43.3 43.5 0.2 C 5,640 ³ 267 775 3.5 45.1 45.1 45.2 0.1 D 7,000 ³ 272 860 3.2 49.9 49.9 50.0 0.1 E 8,100 ³ 181 673 4.1 53.3 53.3 53.4 0.1 F 8,900 ¹ 229 951 2.9 57.0 57.0 57.2 0.2	E	5,400 ²	90	91	2.00	62.6	62.6	62.8	0.2
A 3,510 ³ 549 1,785 1.9 41.6 41.6 41.8 0.2 B 4,730 ³ 266 1,147 2.4 43.3 43.3 43.5 0.2 C 5,640 ³ 267 775 3.5 45.1 45.1 45.2 0.1 D 7,000 ³ 272 860 3.2 49.9 49.9 50.0 0.1 E 8,100 ³ 181 673 4.1 53.3 53.3 53.4 0.1 F 8,900 ¹ 229 951 2.9 57.0 57.0 57.2 0.2	Millow Brook								
B 4,730 ³ 266 1,147 2.4 43.3 43.3 43.5 0.2 C 5,640 ³ 267 775 3.5 45.1 45.1 45.2 0.1 D 7,000 ³ 272 860 3.2 49.9 49.9 50.0 0.1 E 8,100 ³ 181 673 4.1 53.3 53.3 53.4 0.1 F 8,900 ¹ 229 951 2.9 57.0 57.0 57.2 0.2		3 5103	549	1 785	10	41.6	416	41.8	0.2
C 5,640 ³ 267 775 3.5 45.1 45.1 45.2 0.1 D 7,000 ³ 272 860 3.2 49.9 49.9 50.0 0.1 E 8,100 ³ 181 673 4.1 53.3 53.3 53.4 0.1 F 8,900 ¹ 229 951 2.9 57.0 57.0 57.2 0.2									
D 7,000 ³ 272 860 3.2 49.9 49.9 50.0 0.1 E 8,100 ³ 181 673 4.1 53.3 53.3 53.4 0.1 F 8,900 ¹ 229 951 2.9 57.0 57.0 57.2 0.2 Feet above Norwood Avenue Feet above confluence with Whale Pond Brook									
E 8,100 ³ 181 673 4.1 53.3 53.3 53.4 0.1 F 8,900 ¹ 229 951 2.9 57.0 57.0 57.2 0.2 ¹ Feet above Norwood Avenue ² Feet above confluence with Whale Pond Brook		5,040 7,000 ³							
F 8,900 ¹ 229 951 2.9 57.0 57.0 57.2 0.2 ¹ Feet above Norwood Avenue ² Feet above confluence with Whale Pond Brook		7,000							
¹ Feet above Norwood Avenue ² Feet above confluence with Whale Pond Brook									
	C D E F ¹ Feet above Norwood Avenu ² Feet above confluence with	5,640 ³ 7,000 ³ 8,100 ³ 8,900 ¹	267 272 181	775 860 673	3.5 3.2 4.1	45.1 49.9 53.3	45.1 49.9 53.3	45.2 50.0 53.4	0.1 0.1 0.1
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MONMOUTH COUNTY, NJ	ALL JU	JRISDICTIO	NS)			BROOK – WH ND BROOK T			

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	FLOODING SOL	IRCE		FLOODWA	Y	ν.	BASE F ATER-SURFAC/ FEET N	CE ELEVATION	
	CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Willo	ow Brook				0200112)				
(con	tinued)								
	G	10,250 ¹	196	675	4.1	58.2	58.2	58.4	0.2
	Н	10,500 ¹	230	1,411	1.9	60.5	60.5	60.5	0.0
	I	13,200 ¹	270	963	2.8	65.6	65.6	65.8	0.2
	J	14,500 ¹	158	788	3.5	69.1	69.1	69.3	0.2
	К	15,200 ¹	155	789	1.8	73.6	73.6	73.7	0.1
	L	15,600 ¹	108	612	2.4	73.7	73.7	73.8	0.1
	M	15,752 ¹	85	416	3.5	73.8	73.8	73.9	0.1
	N	16,544 ¹	122	251	5.8	74.8	74.8	74.8	0.0
	0	17,125	110	245	5.9	77.3	77.3	77.3	0.0
	Р	17,230 ¹	160	525	2.8	79.6	79.6	79.6	0.0
	Q	19,606 ¹	140	445	2.6	85.3	85.3	85.3	0.0
	R	22,194	300	385	2.0	106.1	106.1	106.1	0.0
	S	22,299 ¹	300	1,495	0.5	109.8	109.8	109.8	0.0
	Т	24,358 ¹	200	162	3.2	124.9	124.9	125.0	0.1
	U	26,312 ¹	20	75	3.3	136.1	136.1	136.1	0.0
	V	28,054 ¹	15	30	8.5	207.5	207.5	207.5	0.0
Wille	ow Brook Tributary F		1						
	A	158 ²	279	210	4.80	77.4	77.4	77.4	0.0
	В	2,112 ²	100	270	3.70	92.0	92.0	92.0	0.0
	С	3,485 ²	100	410	2.50	93.4	93.4	93.4	0.0
	D	6,072 ²	1200	225	3.90	98.0	98.0	98.0	0.0
	E	8,501 ²	270	475	1.30	107.6	107.6	107.6	0.0
	F	8,606 ²	270	220	2.80	112.7	112.7	112.7	0.0
	G	11,774 ²	220	270	1.60	131.4	131.4	131.6	0.2
	et above mouth et above confluence with W	illow Brook							
ŢŢ	FEDERAL EMERGE					·····		· · · · · · · · · · · · · · · · · · ·	
┢							DWAY DA	ТЛ	
₽ ⊓	MONMOU [.]		Y N.I						
		RISDICTIO	•						
3	,		-		WILLOW E	BROOK – WI	LLOW BRC	DOK TRIBU	TARY F

	RCE		FLOODWA	Y	v	BASE FI ATER-SURFAC (FEET N	E ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Willow Brook Tributary G			·····					
A B	634 ¹ 2,957 ¹	20 10	55 20	7.70 7.40	114.8 169.4	114.8 169.4	114.9 169.4	0.1 0.0
Willow Brook East Branch								
A B	830 ² 1,300 ²	90 32	195 82	1.30 3.10	80.7 81.2	80.7 81.2	80.9 81.3	0.2 0.1
С	2,330 ²	48	70	3.60	84.4	84.4	84.5	0.1
D	3,890 ²	28	62	4.10	99.0	99.0	99.2	0.2
E	4,710 ²	18	56	4.40	106.7	106.7	106.9	0.2
Wreck Pond Brook								
A	4,300 ³	350	1,090	2.60	9.2	9.2	9.4	0.2
В	6,170 ³	273	730	3.90	15.6	15.6	15.6	0.0
c	7,460 ³	338	1,789	1.80	20.3	20.3	20.3	0.0
D	9,080 ³	255	828	5.60	21.6	21.6	21.8	0.2
Ē	11,000 ³	376	3,093	0.80	28.7	28.7	28.9	0.2
F	12,620 ³	216	861	3.80	30.2	30.2	30.4	0.2
G	14,300 ³	283	861	4.40	33.9	33.9	34.1	0.2
Ĥ	15,000 ³	191	815	4.40	39.0	39.0	39.2	0.2
	17,200 ³	131	334	9.00	41.6	41.6	41.8	0.2
J	18,500 ³	281	1,024	2.50	46.7	46.7	46.8	0.1
ĸ	19,650 ³	264	650	4.10	48.1	48.1	48.3	0.2
L	21,000 ³	138	306	8.30	53.9	53.9	54.1	0.2
M	$22,680^3$	313	1,185	1.30	59.7	59.7	59.9	0.2
N	$24,400^3$	107	214	7.70	63.0	63.0	63.2	0.2
0	25,374 ³	34	129	7.70	68.4	68.4	68.6	0.2
P	26,900 ³	182	483	3.10	71.5	71.5	71.7	0.2
Q	29,625 ³	50	111	9.00	87.8	87.8	87.8	0.0

MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)

12

WILLOW BROOK TRIBUTARY G -WILLOW BROOK EAST BRANCH – WRECK POND BROOK

FLOODING SC	OURCE		FLOODWA	Y	n v	BASE F ATER-SURFAΩ (FEET N	CE ELEVATION	
CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Wreck Pond Brook East Branch								
A B C D E F G H I J	$\begin{array}{c} 1,025^{1}\\ 2,424^{1}\\ 3,100^{1}\\ 4,950^{1}\\ 5,350^{1}\\ 5,773^{1}\\ 6,574^{1}\\ 7,800^{1}\\ 8,395^{1}\\ 9,400^{1} \end{array}$	301 34 40 51 110 90 128 411 191 47	647 89 95 85 262 1,694 196 2,407 332 43	1.10 4.10 3.80 6.10 1.90 0.20 1.90 0.10 0.80 2.80	28.7 30.9 32.2 41.7 43.1 47.1 47.2 56.9 56.9 64.3	28.7 30.9 32.2 41.7 43.1 47.1 47.2 56.9 56.9 64.3	28.9 31.1 32.4 41.7 43.3 47.3 47.4 56.9 56.9 64.5	0.2 0.2 0.0 0.2 0.2 0.2 0.2 0.0 0.0 0.0
Yellow Brook A B C D E F G H I J K L M N	965 ² 2,525 ² 4,000 ² 4,700 ² 5,100 ² 6,220 ² 6,860 ² 8,330 ² 8,860 ² 10,200 ² 11,550 ² 12,600 ² 15,370 ² 17,050 ² Wreck Pond Brook	86 200 216 228 238 140 199 129 101 150 90 95 132 86	851 1,392 964 1,033 1,147 573 948 490 599 538 287 397 564 328	3.70 2.30 3.30 1.90 1.70 3.50 2.10 4.00 3.30 3.70 6.90 5.00 3.00 5.10	39.7 40.5 41.2 41.9 42.1 42.8 46.1 47.2 50.2 51.6 55.8 60.8 69.3 72.0	39.7 40.5 41.2 41.9 42.1 42.8 46.1 47.2 50.2 51.6 55.8 60.8 69.3 72.0	39.7 40.5 41.3 42.1 42.3 43.0 46.2 47.4 50.2 51.8 56.0 60.8 69.5 72.2	0.0 0.0 0.1 0.2 0.2 0.2 0.1 0.2 0.0 0.2 0.2 0.2 0.2 0.2 0.2 0.2
	ENCY MANAGEMEN				FLOOI	DWAY DA	ТА	
ALL JU	JRISDICTIO	•	WR	ECK PONI	D BROOK EA	ST BRAN	CH – YELLO	W BROOK

FLO	ODING SOUF	RCE		FLOODWA	Y	Ŵ	BASE F ATER-SURFAΩ FEET №	CE ELEVATION	
CROSS SE	CTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Yellow Brook (continued)									
O P Q R S T U V W Yellow Brook 2 A B C D E		$\begin{array}{c} 17,300^{1} \\ 18,500^{1} \\ 19,170^{1} \\ 19,440^{1} \\ 20,940^{1} \\ 22,800^{1} \\ 23,450^{1} \\ 24,380^{1} \\ 26,970^{1} \\ \end{array}$ $\begin{array}{c} 30,020^{2} \\ 30,050^{2} \\ 30,570^{2} \\ 30,591^{2} \\ 31,551^{2} \end{array}$	174 169 174 270 162 210 175 308 124 10 30 21 52 33	348 721 697 2,995 472 1,160 793 790 287 75 193 165 231 129	4.80 2.30 2.40 0.60 3.60 0.70 1.00 1.00 2.70 10.30 4.00 3.30 2.40 4.20	75.1 76.7 77.2 83.4 83.4 88.2 88.2 88.3 95.4 98.5 100.6 101.6 101.7 102.4	75.1 76.7 77.2 83.4 83.4 88.2 88.2 88.3 95.4 98.5 100.6 101.6 101.7 102.4	75.1 76.9 77.4 83.4 83.4 88.2 88.3 88.5 95.6 98.7 100.6 101.7 101.8 102.4	0.0 0.2 0.2 0.0 0.0 0.1 0.2 0.2 0.2 0.2 0.2 0.2 0.1 0.1 0.1 0.1 0.1 0.0
F G H I		32,595 ² 32,625 ² 34,160 ² 34,190 ²	14 35 14 21	75 159 75 69	7.30 3.40 7.30 7.90	108.2 109.0 114.2 114.3	108.2 109.0 114.2 114.3	108.2 109.0 114.2 114.3	0.0 0.0 0.0 0.0 0.0
Yellow Brook 2 Tri A B C	ibutary	150 ³ 856 ³ 886 ³	30 53 56	109 270 302	3.50 1.40 1.20	100.9 107.3 107.3	99.6⁴ 107.3 107.3	99.8 107.3 107.3	0.2 0.0 0.0
¹ Feet above mout ² Feet above confl ³ Feet above confl ⁴ Elevation compu	uence with Swi uence with Yell	ow Brook 2	vater effects	from Yellow Bro	bok 2	<u> </u>		<u> </u>	
		CY MANAGEMEN				FLOOI	DWAY DA	TA	
		ISDICTIO	•			LOW BROOK			! –

							BASE F	LOOD	
	FLOODING SOU	RCE		FLOODWA	Y	N N	ATER-SURFAC	CE ELEVATION	
	CROSS SECTION	DISTANCE	WIDTH (FEET)	SECTION AREA (SQUARE FEET)	MEAN VELOCITY (FEET PER SECOND)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Yell	low Brook Tributary K A B C D E F G H I J J Iow Brook Tributary L A B C D E F F	475 ¹ 2,270 ¹ 2,376 ¹ 3,802 ¹ 3,907 ¹ 5,174 ¹ 5,333 ¹ 5,438 ¹ 5,650 ¹ 6,336 ¹ 950 ¹ 1,056 ¹ 2,112 ¹ 2,376 ¹ 3,538 ¹ 6,019 ¹	230 300 250 250 20 20 20 50 40 90 90 200 230 15 30	355 595 635 380 635 30 70 70 70 70 70 70 70 70 70 70 70 70 70	1.90 0.80 0.70 1.00 0.60 4.40 2.00 2.00 2.00 2.00 2.00 1.80 1.00 1.50 0.40 5.70 4.40	105.5 109.9 110.1 115.9 117.1 124.0 126.9 128.3 129.7 134.6 109.3 111.0 111.6 115.6 139.6 171.1	105.5 109.9 110.1 115.9 117.1 124.0 126.9 128.3 129.7 134.6 109.3 111.0 111.6 115.6 139.6 171.1	105.6 110.1 110.2 116.1 124.1 126.9 128.3 129.7 134.8 109.3 111.0 111.8 115.8 139.6 171.1	0.1 0.2 0.1 0.2 0.0 0.1 0.0 0.0 0.0 0.2 0.2 0.2 0.2 0.2 0.2 0.0 0.0
	FEDERAL EMERGE		IT AGENCY						
	MONMOUT		-			FLOOI	DWAY DA	TA	
= 12	(ALL JUI	RISDICTIO	NS)			ELLOW BROYELLOW BR			

5.0 INSURANCE APPLICATIONS

For flood insurance rating purposes, flood insurance zone designations are assigned to a community based on the results of the engineering analyses. The zones are as follows:

Zone A

Zone A is the flood insurance rate zone that corresponds to the 1-percent annual chance floodplains that are determined in the FIS by approximate methods. Because detailed hydraulic analyses are not performed for such areas, no base flood elevations or depths are shown within this zone.

Zone AE

Zone AE is the flood insurance rate zone that corresponds to the 1-percent annual chance floodplains that are determined in the FIS by detailed methods. In most instances, whole-foot base flood elevations derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone AH

Zone AH is the flood insurance rate zone that corresponds to the areas of 1-percent annual chance shallow flooding (usually areas of ponding) where average depths are between 1 and 3 feet. Whole-foot base flood elevations derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone AO

Zone AO is the flood insurance rate zone that corresponds to the areas of 1-percent annual chance shallow flooding (usually sheet flow on sloping terrain) where average depths are between 1 and 3 feet. Average whole-foot depths derived from the detailed hydraulic analyses are shown within this zone.

Zone AR

Area of special flood hazard formerly protected from the 1-percent annual chance flood event by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1-percent annual chance or greater flood event.

Zone A99

Zone A99 is the flood insurance rate zone that corresponds to areas of the 1-percent annual chance floodplain that will be protected by a Federal flood protection system where construction has reached specified statutory milestones. No base flood elevations or depths are shown within this zone.

Zone V

Zone V is the flood insurance rate zone that corresponds to the 1-percent annual chance coastal floodplains that have additional hazards associated with storm

waves. Because approximate hydraulic analyses are performed for such areas, no base flood elevations are shown within this zone.

Zone VE

Zone VE is the flood insurance rate zone that corresponds to the 1-percent annual chance coastal floodplains that have additional hazards associated with storm waves. Whole-foot base flood elevations derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone X

Zone X is the flood insurance rate zone that corresponds to areas outside the 0.2percent annual chance floodplain, areas within the 0.2-percent annual chance floodplain, and to areas of 1-percent annual chance flooding where average depths are less than 1 foot, areas of 1-percent annual chance flooding where the contributing drainage area is less than 1 square mile, and areas protected from the 1-percent annual chance flood by levees. No base flood elevations or depths are shown within this zone.

Zone D

Zone D is the flood insurance rate zone that corresponds to unstudied areas where flood hazards are undetermined, but possible.

6.0 <u>FLOOD INSURANCE RATE MAP</u>

The FIRM is designed for flood insurance and floodplain management applications.

For flood insurance applications, the map designates flood insurance rate zones as described in Section 5.0 and, in the 1-percent annual chance floodplains that were studied by detailed methods, shows selected whole-foot base flood elevations or average depths. Insurance agents use the zones and base flood elevations in conjunction with information on structures and their contents to assign premium rates for flood insurance policies.

For floodplain management applications, the map shows by tints, screens, and symbols, the 1- and 0.2-percent annual chance floodplains. Floodways and the locations of selected cross sections used in the hydraulic analyses and floodway computations are shown where applicable.

The current FIRM presents flooding information for the entire geographic area of Monmouth County. Previously, separate Flood Hazard Boundary Maps and/or FIRMs were prepared for each identified flood-prone incorporated community and the unincorporated areas of the county. This countywide FIRM also includes flood hazard information that was presented separately on Flood Boundary and Floodway Maps (FBFMs), where applicable. Historical data relating to the maps prepared for each community, up to and including this countywide FIS, are presented in Table 13, "Community Map History."

COMMUNITY	INITIAL	FLOOD HAZARD BOUNDARY MAP	FIRM	FIRM
NAME	IDENTIFICATION	REVISIONS DATE	EFFECTIVE DATE	REVISIONS DATE
Aberdeen, Township of	February 28, 1975	None	March 18. 1985	August 3, 1992 September 25, 2009
Allenhurst, Borouah of	August 24, 1973	April 30, 1976	March 15, 1979	September 15, 1983 September 25, 2009
Allentown, Borough of	December 21, 1973	February 6, 1976	September 16, 1981	September 25, 2009
Asburv Park, Citv of	July 13, 1973	April 30, 1976	February 15, 1979	September 15, 1983 September 25, 2009
Atlantic Highlands. Borough of	December 21, 1975	Februarv 20. 1976	August 3. 1981	July 5, 1984 September 25, 2009
Avon-Bv-The-Sea. Borough of	Februarv 1, 1974	None	March 15. 1979	July 5, 1983 September 25, 2009
Belmar, Borough of	May 13, 1972	None	May 13, 1972	July 1, 1974 February 27, 1976 March 1, 1984 September 25, 2009
Bradley Beach, Borough of	December 21, 1973	February 6, 1976	August 1, 1979	June 15, 1983 September 25, 2009
Brielle. Borouah of	August 31. 1973	February 11. 1977	April 2, 1979	September 30, 1983 September 25, 2009
Colts Neck, Township of	April 12, 1974	September 17, 1976 February 11, 1977	April 15, 1982	September 25, 2009

FEDERAL EMERGENCY MANAGEMENT AGENCY

TABLE 13

MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)

INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISIONS DATE	FIRM EFFECTIVE DATE	FIRM REVISIONS DATE
January 14, 1972	February 21, 1975	March 5, 1976	Januarv 5, 1984 August 6, 2002 September 25, 2009
June 21, 1974	Februarv 6, 1976	September 16, 1981	September 25, 2009
June 21, 1974	None	March 16, 1981	September 25, 2009
July 6, 1973	August 20, 1976	October 16, 1979	September 25, 2009
March 15, 1974	February 13, 1976	November 26, 1982	September 25, 2009
February 15, 1974	July 16, 1976	April 4, 1983	September 25, 2009
December 28, 1973	April 30, 1976	December 1, 1982	September 25, 2009
December 15, 1970	None	September 3, 1971	July 1, 1974 June 30, 1976 September 25, 2009
January 25, 1974	October 17, 1975 April 16, 1976	March 1, 1982	September 25, 2009
March 22, 1974	August 27. 1976	Januarv 6. 1983	September 25. 2009
	Januarv 14, 1972 June 21, 1974 June 21, 1974 Julv 6, 1973 March 15, 1974 Februarv 15, 1974 December 28, 1973 December 15, 1970 Januarv 25, 1974	January 14, 1972 February 21, 1975 June 21, 1974 February 6, 1976 June 21, 1974 None July 6, 1973 August 20, 1976 March 15, 1974 February 13, 1976 February 15, 1974 July 16, 1976 December 28, 1973 April 30, 1976 December 15, 1970 None January 25, 1974 October 17, 1975 April 16, 1976 April 16, 1976	January 14, 1972 February 21, 1975 March 5, 1976 June 21, 1974 February 6, 1976 September 16, 1981 June 21, 1974 None March 16, 1981 July 6, 1973 August 20, 1976 October 16, 1979 March 15, 1974 February 13, 1976 November 26, 1982 February 15, 1974 July 16, 1976 April 4, 1983 December 28, 1973 April 30, 1976 December 1, 1982 December 15, 1970 None September 3, 1971 January 25, 1974 October 17, 1975 March 1, 1982

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)

COMMUNITY	INITIAL	FLOOD HAZARD BOUNDARY MAP	FIRM	FIRM
NAME	IDENTIFICATION	REVISIONS DATE	EFFECTIVE DATE	REVISIONS DATE
Interlaken, Borough of	March 15, 1974	None	January 2, 1981	September 25, 2009
Keansburg, Borough of	April 20, 1973	February 3, 1978	Mav 16, 1983	September 25, 2009
Keyport, Borough of	January 23. 1974	Februarv 6, 1976	July 2. 1979	April 18, 1983 Julv 15, 1992 September 25, 2009
Lake Como, Borough of	Februarv 22, 1974	None	November 28, 1980	November 2, 1995 September 25, 2009
Little Silver. Borough of	August 31, 1973	August 27, 1976	Februarv 1, 1978	December 15, 1982 September 25, 2009
Loch Arbour, Village of	November 30, 1973	April 16, 1976	March 15, 1979	September 15, 1983 September 25, 2009
Long Branch. City of	May 31, 1974	None	Mav 5. 1976	Januarv 13, 1978 January 5, 1984 September 25, 2009
Manalapan, Township of	July 20, 1973	None	September 15, 1977	September 25, 2009
Manasquan, Borough of	Mav 12. 1972	None	Mav 12. 1972	Julv 1. 1974 January 16, 1976 December 15, 1983 September 25, 2009

TABLE 13

FEDERAL EMERGENCY MANAGEMENT AGENCY

MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)

COMMUNITY	INITIAL	FLOOD HAZARD BOUNDARY MAP	FIRM	FIRM
NAME	IDENTIFICATION	REVISIONS DATE	EFFECTIVE DATE	REVISIONS DATE
Marlboro. Township of	December 21, 1973	None	June 15, 1978	April 9, 1982 September 25, 2009
Matawan, Borough of	March 1, 1974	None	September 30, 1981	September 25, 2009
Middletown. Township of	Julv 19. 1974	Julv 9. 1976	Februarv 15. 1984	Julv 15, 1992 September 25, 2009
Millstone, Township of	March 29, 1974	Mav 14, 1976	January 20, 1982	September 25, 2009
Monmouth Beach. Borough of	May 17, 1974	None	May 16, 1977	May 2, 1983 October 16, 1984 July 15, 1992 September 25, 2009
Neptune City, Borough of	June 28, 1974	August 11, 1978	August 11. 1978	September 25, 2009
Neptune. Township of	Julv 13, 1973	September 3, 1976	Februarv 16. 1977	March 1. 1984 September 25, 2009
Ocean, Township of	June 1, 1973	Mav 31, 1974 September 10, 1976	October 14. 1977	December 18, 1981 July 2, 2003 September 25, 2009
Oceanport, Borough of	May 11, 1973	None	February 16, 1977	September 25, 2009
Red Bank, Borough of	March 8, 1974	March 19, 1976	May 19, 1981	September 25, 2009

FEDERAL EMERGENCY MANAGEMENT AGENCY

TABLE 13

MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)

COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISIONS DATE	FIRM EFFECTIVE_DATE	FIRM REVISIONS DATE
Roosevelt, Borough of	September 25, 2009	None	September 25, 2009	September 25, 2009
Rumson. Borouah of	December 28. 1973	None	December 28, 1973	Julv 1, 1974 November 7, 1975 April 23, 1976 December 15, 1982 Julv 15, 1992 September 25, 2009
Sea Briaht. Borouah of	October 14. 1971	None	October 14. 1971	Julv 1, 1974 April 23, 1976 November 16, 1983 Julv 15, 1992 September 25, 2009
Sea Girt, Borough of	February 2, 1973	None	April 16. 1976	April 16, 1976 January 5, 1984 September 25, 2009
Shrewsburv, Borough of	June 7, 1974	October 24, 1975	August 1, 1979	September 25, 2009
Spring Lake, Borough of	May 25. 1973	March 5, 1976 April 9, 1976 Januarv 4, 1980 June 30, 1980	February 17, 1982	March 1, 1984 September 25, 2009

 TABLE
 FEDERAL EMERGENCY MANAGEMENT AGENCY

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 MONMOUTH COUNTY, NJ

 13
 (ALL JURISDICTIONS)

COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISIONS DATE	FIRM EFFECTIVE DATE	FIRM REVISIONS DATE
Spring Lake Heights, Borough of	May 3, 1974	March 5, 1976	December 15, 1981	September 25, 2009
Tinton Falls, Borough of	April 12, 1974	April 23, 1976 December 31, 1976	April 15, 1982	September 25, 2009
Union Beach, Borough of	December 28. 1973	Auaust 6. 1976	May 15. 1980	March 2. 1983 August 15. 1992 September 25, 2009
Upper Freehold, Township of	March 22, 1974	August 20, 1976 July 29, 1977	October 12, 1979	December 11, 1981
Wall, Township of	June 1, 1973	None	February 16, 1977	September 25, 2009
West Long Branch, Borough of	August 24, 1973	August 20, 1976	January 16, 1981	September 25, 2009
			·	
FEDERAL EMERGENCY MANAGEMENT AGENCY				
MONMOUTH COUNTY, NJ (ALL JURISDICTIONS)		COMMUNITY MAP HISTORY		

7.0 <u>OTHER STUDIES</u>

A FIS has been prepared for Ocean County, New Jersey (All Jurisdictions) (FEMA, September 29, 2006). FISs have been prepared for municipalities in Mercer County bordering Monmouth County: Township of East Windsor (FEMA, September 16, 1982) and the Township of Washington (U.S. Department of Housing and Urban Development, March 1978). FISs have also been prepared for municipalities in Middlesex County bordering Monmouth County: Township of Monroe (FEMA, November 6, 1981) and the Township of Old Bridge (FEMA, October 16, 1987). No FISs have been published for the municipalities in Burlington County bordering Monmouth County: the Township of Chesterfield and the Township of North Hanover.

Information pertaining to each jurisdiction within Monmouth County has been compiled into this FIS. Therefore, this FIS supersedes all previously printed FIS Reports, FHBMs, FBFMs, and FIRMs for all jurisdictions within Monmouth County.

8.0 LOCATION OF DATA

Information concerning the pertinent data used in the preparation of this FIS can be obtained by contacting FEMA, Federal Insurance and Mitigation Division, 26 Federal Plaza, Room 1351, New York, New York 10278.

9.0 BIBLIOGRAPHY AND REFERENCES

Abrams Aerial Survey. (January 1976). <u>Aerial Contour Maps</u>, Scale 1"=500', Contour Interval 2 Feet: Allenhurst, New Jersey.

Abrams Aerial Survey. (January 1976). <u>Aerial Contour Maps</u>, Scale 1:6,000, Contour Interval 2 Feet: Loch Arbour, New Jersey.

Abrams Aerial Survey. (January 1976). <u>Aerial Contour Maps</u>, Scale 1:6,000, Contour Interval 2 Feet: South Amboy, New Jersey.

Abrams Aerial Survey. (January 1976). <u>Aerial Contour Maps</u>, Scale 1"=500', Contour Interval 2 Feet: Keansburg, New Jersey.

Abrams Aerial Survey. (1976). <u>Aerial Contour Maps</u>, Scale 1:6,000, Contour Interval 2 Feet: Borough of Brielle, New Jersey. Lansing, Michigan.

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